DRAINAGE ANALYSIS

FOR

RiverWoods Phase II

Stone Quarry Drive Durham, NH

Tax Map 209 Lot 33

July 23, 2025 Revised Through October 8, 2025

Prepared For:

RiverWoods Durham

14 Stone Quarry Drive Durham, NH 03824

Prepared By:

Altus Engineering

133 Court Street Portsmouth, NH 03801 Phone: (603) 433-2335



ERIC D. WEINRY No.

Table of Contents

Section 1 Narrative

Project Description Site Overview Site Soils

Proposed Site Design Calculation Methods

Disclaimer

Drainage Analysis

Conclusions

Section 2 Aerial Photo

USGS Location Map

Section 3 Drainage Analysis, Pre-Development

Section 4 Drainage Analysis, Post-Development

Section 5 Precipitation Table

Section 6 NRCS Soils Report

Site Specific Soil Survey

Test Pit Logs

Section 7 BMP and Riprap Sizing Calculations

Section 8 Stormwater Operations and Maintenance Plan

Inspection Form

Stormwater Management Plan

Section 9 Watershed Plans

Pre-Development Watershed Plan

Pre-Development Soils Plan

Post-Development Watershed Plan

Post-Development Soils Plan



Section 1

Narrative



PROJECT DESCRIPTION

RiverWoods Durham is proposing to construct a second phase on an undeveloped parcel adjacent to the existing RiverWoods continuing care retirement community campus in Durham, New Hampshire. The 22.5-acre property is identified as Assessor's Map 209, Lot 33 and is located in the Office Research District. The project site is primarily woodland with sections forested wetland. Access is from NH 108 via the existing Stone Quarry Drive.

The proposal includes two new age-restricted multi-family housing buildings, a community center and a maintenance garage together with associated accessways and parking areas, utilities and stormwater infrastructure. These measures will include four bioretention ponds and a section of porous pavement. Pretreatment will be provided by catch basins with deep sumps and grease hoods. This proposed stormwater management system will reduce peak flows and treat runoff from the site's impervious areas prior to leaving the site.

Site Soils

Michael Cuomo, NH soils scientist completed a site-specific soil survey (SSSS) for the project site. This survey indicates that the subject property can be broken into hydrologic soils groups (HSG) B, C and D. Test pits indicate that majority of the soils underlying the development area are comprised of clay. Information on soils in offsite areas was taken from a high intensity soil survey (HISS) done for the adjacent RiverWoods campus in 2018 and NRCS maps.

Pre-Development (Existing Conditions)

Runoff generally flows through and off the site in a southerly direction to three points of analysis. POA #1 is an existing culvert under Stone Quarry Drive that captures flow from the western corner of the parcel. POA #2 is an offsite wetland to which the majority of the site and some offsite areas are directed. This offsite area was included as it represents the confluence of two wetland systems and an area affected by the relocation of Stone Quarry Drive. POA #3 is the southern property line where an existing stream captures the southeast portion of the lot. The sites hydrology is characterized by five existing subcatchments as delineated on the accompanying "Pre-Development Watershed Plan".

Post-Development (Proposed Conditions)

The post-development conditions were analyzed at the same discharge points as the predevelopment conditions as delineated on the accompanying "Post-Development Watershed Plan". Modifications to the delineated areas and associated ground cover were made to subcatchments to account for the proposed improvements to the property. As shown on the attached Post-Development Watershed Plan, the site was divided into forty post-development subcatchment areas. Site topography, existing features, proposed site improvements, proposed grading, drainage and erosion control measures are shown on the accompanying plan set. Recommended erosion control measures are based upon the December 2008 edition of the "New Hampshire Stormwater Manual Volumes 1 through 3" prepared by NHDES and Comprehensive Environmental, Inc. as amended.

CALCULATION METHODS

The drainage study was completed using the USDA SCS TR-20 Method within the HydroCAD Stormwater Modeling System. Reservoir routing was performed with the Dynamic Storage Indication method with automated calculation of tailwater conditions. A Type III 24-hour rainfall distribution was utilized in analyzing the data for the 2, 10 and 25 year - 24-hour storm events using rainfall data provided by the Northeast Regional Climate Center (NRCC). As the project site lies within a Coastal and Great Bay Community identified by NHDES Alteration of Terrain, all rainfall amounts were increased by 15% to account for potential future increases in rainfall due to climate change. A time span of 0 to 48 hours was analyzed at 0.01-hour increments. Design infiltration rates used in the analysis were taken from the corresponding test pit logs. Pond summaries for the 100-year storm event are also included in the proposed analysis.

Disclaimer

Altus Engineering, Inc. notes that stormwater modeling is limited in its capacity to precisely predict peak rates of runoff and flood elevations. Results should not be considered to represent actual storm events due to the number of variables and assumptions involved in the modeling effort. Surface roughness coefficients (n), entrance loss coefficients (ke), velocity factors (kv) and times of concentration (Tc) are based on subjective field observations and engineering judgment using available data. For design purposes, curve numbers (Cn) describe the average conditions. However, curve numbers will vary from storm to storm depending on the antecedent runoff conditions (ARC) including saturation and frozen ground. Also, higher water elevations than predicted by modeling could occur if drainage channels, closed drain systems or culverts are not maintained and/or become blocked by debris before and/or during a storm event as this will impact flow capacity of the structures. Structures should be re-evaluated if future changes occur within relevant drainage areas in order to assess any required design modifications.

Drainage Analysis

A complete summary of the drainage model is included in the appendix of this report. The following tables compare pre- and post-development peak rates and volumes at the Point of Analysis identified on the plans for the 1", 2, 10 and 25-year storm events:

Stormwater Modeling Summary
Peak Q (cfs) for Type III 24-Hour Storm Events

	1" Storm	2-Yr Storm	10-Yr Storm	25-Yr Storm
	(1.00 inch)	(3.61 inch)	(5.47 inch)	(6.93 inch)
POA #1 (South to Culvert)				
Pre	0.01	2.40	4.26	5.12
Post	0.01	2.05	3.78	4.78
Change	0.00	-0.35	-0.48	-0.34
POA #2 (South to Wetland)				
Pre	0.05	15.07	27.12	34.48
Post	0.10	13.65	23.20	30.13
Change	0.05	-1.42	-3.92	-4.35
POA #3 (South to Wetland)				
Pre	0.00	6.43	16.14	24.80
Post	0.04	5.84	15.73	24.21
Change	0.04	-0.59	-0.41	-0.59

As the above table demonstrates, the proposed peak rates of runoff at the points of analysis will match or be decreased from the existing conditions for all analyzed storm events with the exception of the 1" event where the increases have been determined to be insignificant due to the fact that they total no more than 0.05 cfs and because decreases are shown in all larger storms. A decrease in the 1" storm is also not a requirement of the Town of Durham's Site Plan Regulations.

The following table demonstrates the pre- and post- volumes of runoff. As shown in the attached calculations, the use of porous pavement allows the infiltration of the town's requisite groundwater recharge volume which exceeds that required by the NHDES.

Stormwater Modeling Summary Volume V (af) for Type III 24-Hour Storm Events

	1" Storm	2-Yr Storm	10-Yr Storm	25-Yr Storm
	(1.00 inch)	(3.61 inch)	(5.47 inch)	(6.93 inch)
POA #1 (South to Culvert)				
Pre	0.004	0.215	0.448	0.650
Post	0.008	0.214	0.430	0.614
Change	0.004	-0.001	-0.018	-0.036
POA #2 (South to Wetland)				
Pre	0.038	2.198	4.597	6.679
Post	0.062	2.221	4.580	6.621
Change	0.024	0.023	-0.017	-0.058
POA #3 (South to Wetland)				
Pre	0.001	0.691	1.589	2.399
Post	0.064	0.968	1.989	2.885
Change	0.063	0.277	0.400	0.486

POLLUTANT REMOVAL

Based on the New Hampshire Stormwater Manual (Volume 2), the following pollutant removal rates would be expected from the implementation of the proposed stormwater BMPs:

<u>BMP</u>	<u>Pollutant</u>	Removal Efficiency
Deep Sum	p Catch Basin -	
•	Total Suspended Solids (TSS)	15%
	Total Nitrogen (TN)	5%
	Total Phosphorus (TP)	5%
Bioretenti	on Pond -	
	Total Suspended Solids (TSS)	90%
	Total Nitrogen (TN)	65%
	Total Phosphorus (TP)	65%
Porous Pa	vement -	
	Total Suspended Solids (TSS)	90%
	Total Nitrogen (TN)	60%
	Total Phosphorus (TP)	65%

CONCLUSION

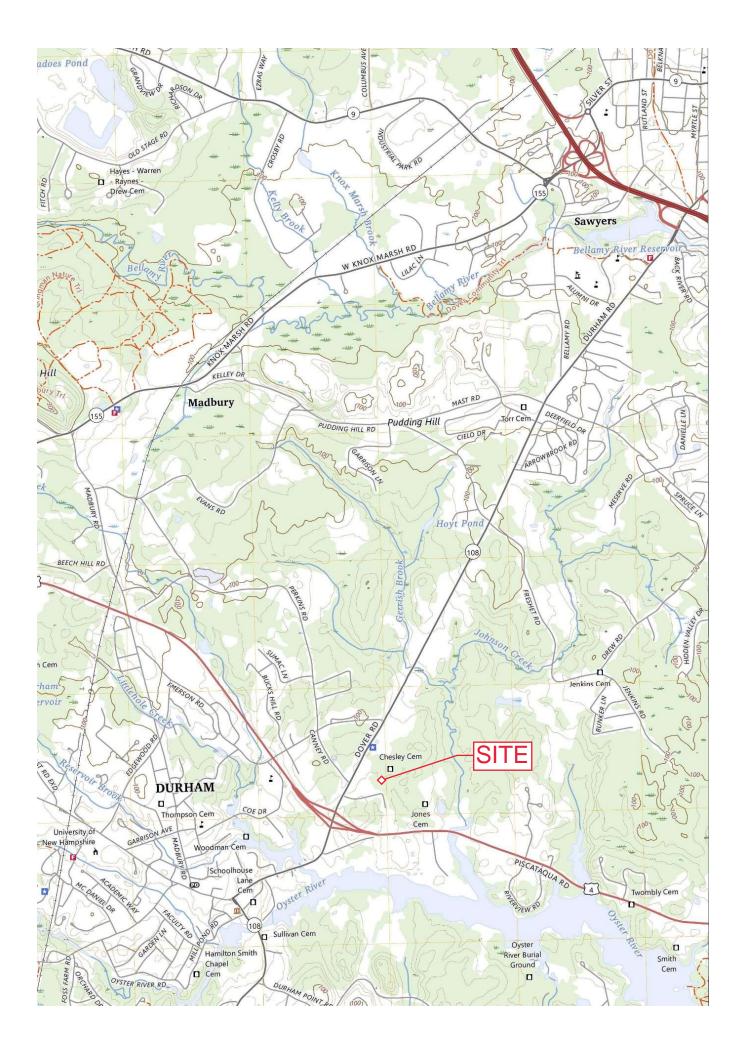
This proposed development of property located on Stone Quarry Drive in Durham, New Hampshire will have minimal adverse effect on abutting properties and infrastructure as a result of stormwater runoff or siltation. Post-construction peak rates of runoff from the site will be at or lower than the existing conditions for all analyzed storm events with the insignificant exceptions noted above. The new stormwater management system will also provide appropriate treatment to and infiltration of runoff from the proposed impervious surfaces. Appropriate steps will be taken to properly mitigate erosion and sedimentation through the use of temporary and permanent Best Management Practices for sediment and erosion control, including deep sump catch basins with grease hoods, bioretention ponds and porous pavement.

Section 2

Aerial Photo and USGS Map





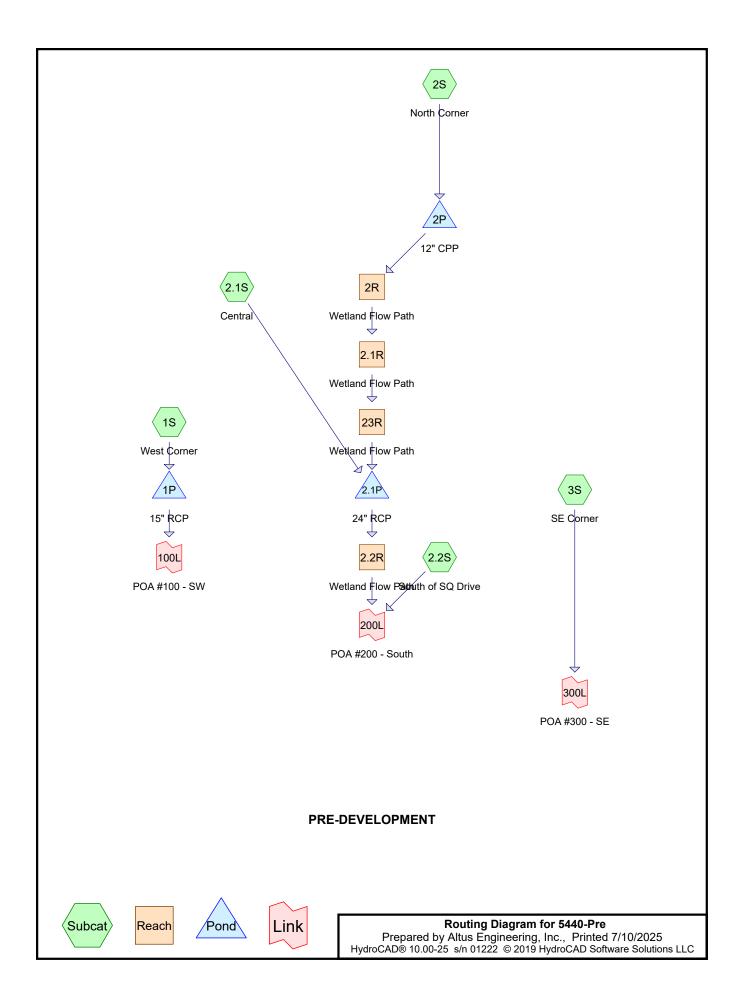


Section 3

Drainage Calculations

Pre-Development 2-Year, 24-Hour Summary 10-Year, 24-Hour Complete 25-Year, 24-Hour Summary





5440-Pre

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: West Corner Runoff Area=85,332 sf 8.89% Impervious Runoff Depth=0.02"

Flow Length=366' Tc=9.8 min CN=74 Runoff=0.01 cfs 0.004 af

Subcatchment 2.1S: Central Runoff Area=494,247 sf 8.83% Impervious Runoff Depth=0.02"

Flow Length=1,341' Tc=15.8 min CN=74 Runoff=0.04 cfs 0.022 af

Subcatchment 2.2S: South of SQ Drive Runoff Area=105,068 sf 10.77% Impervious Runoff Depth=0.05"

Flow Length=391' Tc=6.0 min CN=77 Runoff=0.03 cfs 0.010 af

Subcatchment 2S: North Corner Runoff Area = 284,221 sf 6.78% Impervious Runoff Depth = 0.01"

Tc=0.0 min CN=72 Runoff=0.01 cfs 0.007 af

Subcatchment 3S: SE Corner Runoff Area=373,920 sf 2.67% Impervious Runoff Depth=0.00"

Flow Length=1,149' Tc=15.9 min CN=68 Runoff=0.00 cfs 0.001 af

Reach 2.1R: Wetland Flow Path

Avg. Flow Depth=0.02' Max Vel=0.23 fps Inflow=0.01 cfs 0.007 af

n=0.040 L=344.0' S=0.0075 '/' Capacity=54.59 cfs Outflow=0.01 cfs 0.007 af

Reach 2.2R: Wetland Flow Path

Avg. Flow Depth=0.03' Max Vel=0.74 fps Inflow=0.04 cfs 0.028 af

n=0.040 L=263.0' S=0.0502'/' Capacity=141.18 cfs Outflow=0.04 cfs 0.028 af

Reach 2R: Wetland Flow Path

Avg. Flow Depth=0.01' Max Vel=0.57 fps Inflow=0.01 cfs 0.007 af

n=0.040 L=151.0' S=0.0446'/' Capacity=133.09 cfs Outflow=0.01 cfs 0.007 af

Reach 23R: Wetland Flow Path Avg. Flow Depth=0.01' Max Vel=0.36 fps Inflow=0.01 cfs 0.007 af

n=0.040 L=131.0' S=0.0185 '/' Capacity=85.86 cfs Outflow=0.01 cfs 0.007 af

Pond 1P: 15" RCP Peak Elev=52.31' Storage=1 cf Inflow=0.01 cfs 0.004 af

15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=0.01 cfs 0.004 af

Pond 2.1P: 24" RCP Peak Elev=47.65' Storage=8 cf Inflow=0.04 cfs 0.028 af

24.0" Round Culvert n=0.012 L=78.0' S=0.0135 '/' Outflow=0.04 cfs 0.028 af

Pond 2P: 12" CPP Peak Elev=59.70' Storage=30 cf Inflow=0.01 cfs 0.007 af

Outflow=0.01 cfs 0.007 af

Link 100L: POA #100 - SW Inflow=0.01 cfs 0.004 af

Primary=0.01 cfs 0.004 af

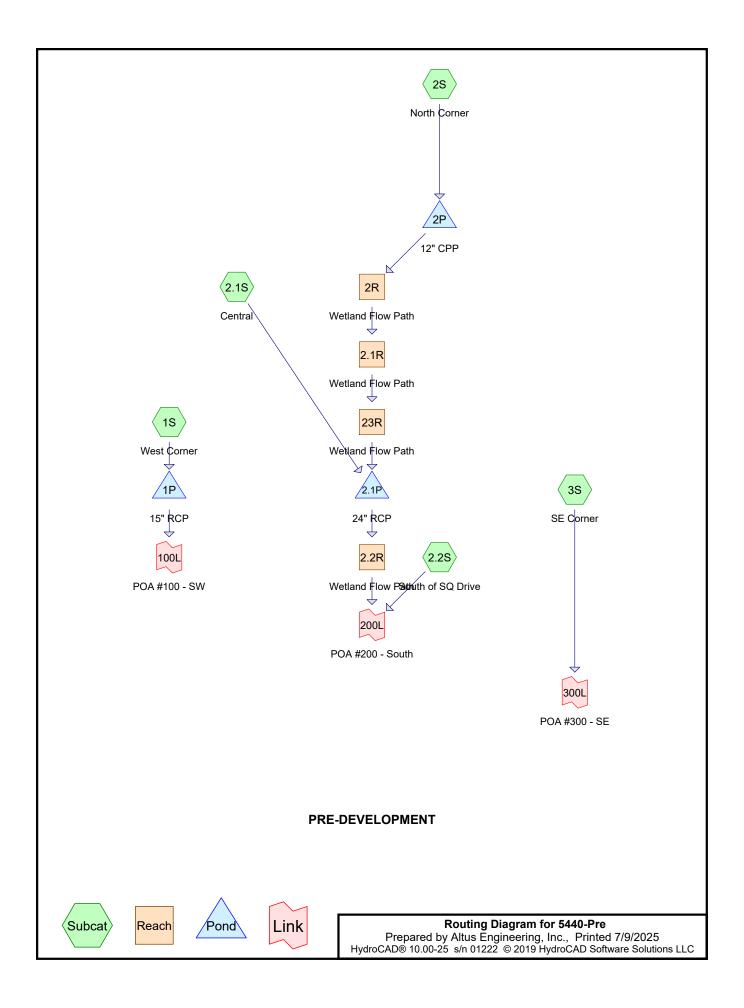
Link 200L: POA #200 - South Inflow=0.05 cfs 0.038 af

Primary=0.05 cfs 0.038 af

Link 300L: POA #300 - SE Inflow=0.00 cfs 0.001 af

Primary=0.00 cfs 0.001 af

Total Runoff Area = 30.826 ac Runoff Volume = 0.042 af Average Runoff Depth = 0.02" 93.16% Pervious = 28.718 ac 6.84% Impervious = 2.108 ac



Printed 7/9/2025

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: West Corner Runoff Area=85,332 sf 8.89% Impervious Runoff Depth=1.32"

Flow Length=366' Tc=9.8 min CN=74 Runoff=2.56 cfs 0.215 af

Subcatchment 2.1S: Central Runoff Area=494,247 sf 8.83% Impervious Runoff Depth=1.32"

Flow Length=1,341' Tc=15.8 min CN=74 Runoff=12.48 cfs 1.245 af

Subcatchment 2.2S: South of SQ Drive Runoff Area=105,068 sf 10.77% Impervious Runoff Depth=1.51" Flow Length=391' Tc=6.0 min CN=77 Runoff=4.22 cfs 0.304 af

Subcatchment 2S: North Corner Runoff Area = 284,221 sf 6.78% Impervious Runoff Depth = 1.19"

Tc=0.0 min CN=72 Runoff=10.62 cfs 0.649 af

Subcatchment 3S: SE Corner Runoff Area=373,920 sf 2.67% Impervious Runoff Depth=0.97"

Flow Length=1,149' Tc=15.9 min CN=68 Runoff=6.43 cfs 0.691 af

Reach 2.1R: Wetland Flow Path

Avg. Flow Depth=0.45' Max Vel=1.48 fps Inflow=2.22 cfs 0.649 af

n=0.040 L=344.0' S=0.0075'/' Capacity=54.59 cfs Outflow=2.22 cfs 0.649 af

Reach 2.2R: Wetland Flow Path Avg. Flow Depth=0.68' Max Vel=4.79 fps Inflow=13.20 cfs 1.894 af

n=0.040 L=263.0' S=0.0502 '/' Capacity=141.18 cfs Outflow=13.19 cfs 1.894 af

Reach 2R: Wetland Flow Path Avg. Flow Depth=0.28' Max Vel=2.78 fps Inflow=2.23 cfs 0.649 af

n=0.040 L=151.0' S=0.0446 '/' Capacity=133.09 cfs Outflow=2.22 cfs 0.649 af

Reach 23R: Wetland Flow Path Avg. Flow Depth=0.35' Max Vel=2.04 fps Inflow=2.22 cfs 0.649 af

n=0.040 L=131.0' S=0.0185 '/' Capacity=85.86 cfs Outflow=2.22 cfs 0.649 af

Pond 1P: 15" RCP Peak Elev=53.20' Storage=137 cf Inflow=2.56 cfs 0.215 af

15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=2.40 cfs 0.215 af

Pond 2.1P: 24" RCP Peak Elev=49.33' Storage=2,463 cf Inflow=14.47 cfs 1.894 af

24.0" Round Culvert n=0.012 L=78.0' S=0.0135 '/' Outflow=13.20 cfs 1.894 af

Pond 2P: 12" CPP Peak Elev=60.71' Storage=8,446 cf Inflow=10.62 cfs 0.649 af

Outflow=2.23 cfs 0.649 af

Link 100L: POA #100 - SW Inflow=2.40 cfs 0.215 af

Primary=2.40 cfs 0.215 af

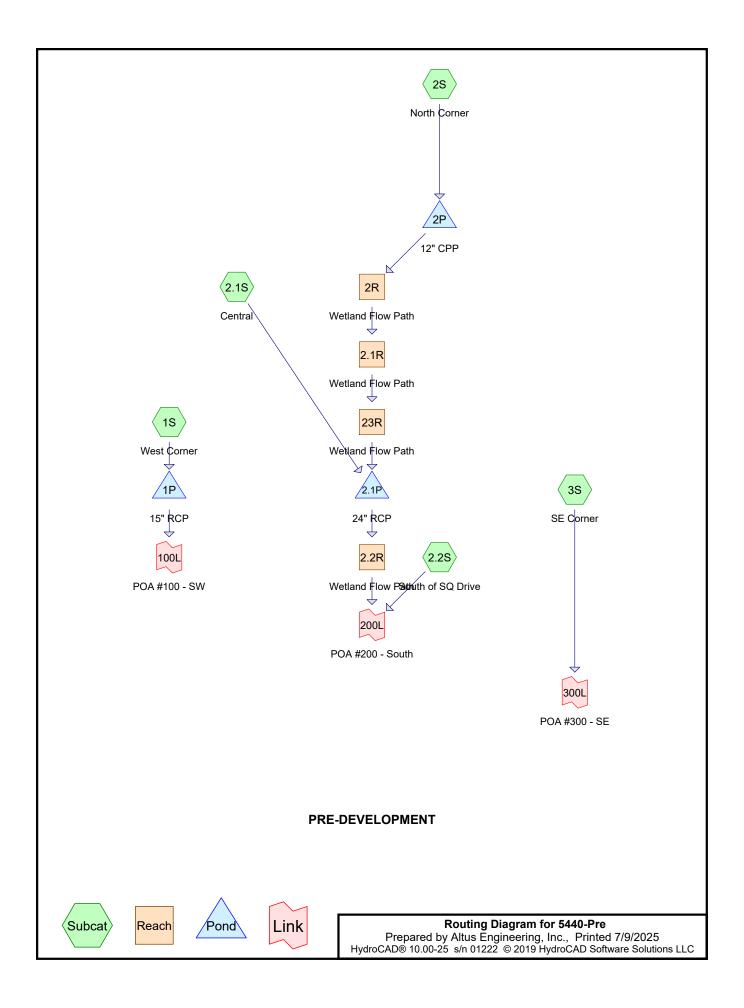
Link 200L: POA #200 - South Inflow=15.07 cfs 2.198 af

Primary=15.07 cfs 2.198 af

Link 300L: POA #300 - SE Inflow=6.43 cfs 0.691 af

Primary=6.43 cfs 0.691 af

Total Runoff Area = 30.826 ac Runoff Volume = 3.103 af Average Runoff Depth = 1.21" 93.16% Pervious = 28.718 ac 6.84% Impervious = 2.108 ac



Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.039	61	>75% Grass cover, Good, HSG B (2.1S, 3S)
3.140	74	>75% Grass cover, Good, HSG C (1S, 2.1S, 2S, 3S)
0.087	80	>75% Grass cover, Good, HSG D (2.1S, 2S)
0.243	65	Brush, Good, HSG C (2.1S)
0.039	98	Paved parking, HSG B (2.1S, 3S)
1.406	98	Paved parking, HSG C (2.1S, 2.2S, 2S, 3S)
0.227	98	Paved parking, HSG D (2.1S, 2.2S, 2S)
0.174	98	Paved roads w/curbs & sewers, HSG C (1S)
0.261	98	Roofs, HSG C (2.1S, 2S)
2.232	55	Woods, Good, HSG B (1S, 2.1S, 2S, 3S)
19.669	70	Woods, Good, HSG C (1S, 2.1S, 2.2S, 2S, 3S)
3.308	77	Woods, Good, HSG D (2.1S, 2.2S, 2S)
30.826	72	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
2.310	HSG B	1S, 2.1S, 2S, 3S
24.894	HSG C	1S, 2.1S, 2.2S, 2S, 3S
3.622	HSG D	2.1S, 2.2S, 2S
0.000	Other	
30.826		TOTAL AREA

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: West Corner Runoff Area=85,332 sf 8.89% Impervious Runoff Depth=2.74"

Flow Length=366' Tc=9.8 min CN=74 Runoff=5.53 cfs 0.448 af

Subcatchment 2.1S: Central Runoff Area=494,247 sf 8.83% Impervious Runoff Depth=2.74"

Flow Length=1,341' Tc=15.8 min CN=74 Runoff=26.96 cfs 2.595 af

Subcatchment 2.2S: South of SQ Drive Runoff Area=105,068 sf 10.77% Impervious Runoff Depth=3.02"

Flow Length=391' Tc=6.0 min CN=77 Runoff=8.55 cfs 0.607 af

Subcatchment 2S: North Corner Runoff Area=284,221 sf 6.78% Impervious Runoff Depth=2.57"

Tc=0.0 min CN=72 Runoff=23.92 cfs 1.395 af

Subcatchment 3S: SE Corner Runoff Area=373,920 sf 2.67% Impervious Runoff Depth=2.22"

Flow Length=1,149' Tc=15.9 min CN=68 Runoff=16.14 cfs 1.589 af

Reach 2.1R: Wetland Flow Path

Avg. Flow Depth=1.19' Max Vel=2.53 fps Inflow=18.63 cfs 1.395 af

n=0.040 L=344.0' S=0.0075 '/' Capacity=54.59 cfs Outflow=16.73 cfs 1.395 af

Reach 2.2R: Wetland Flow Path Avg. Flow Depth=0.91' Max Vel=5.61 fps Inflow=24.09 cfs 3.990 af

n=0.040 L=263.0' S=0.0502 '/' Capacity=141.18 cfs Outflow=24.09 cfs 3.990 af

Reach 2R: Wetland Flow Path Avg. Flow Depth=0.83' Max Vel=5.02 fps Inflow=18.70 cfs 1.395 af

n=0.040 L=151.0' S=0.0446 '/' Capacity=133.09 cfs Outflow=18.63 cfs 1.395 af

Reach 23R: Wetland Flow Path Avg. Flow Depth=0.96' Max Vel=3.53 fps Inflow=16.73 cfs 1.395 af

n=0.040 L=131.0' S=0.0185'/' Capacity=85.86 cfs Outflow=16.62 cfs 1.395 af

Pond 1P: 15" RCP Peak Elev=53.65' Storage=1,011 cf Inflow=5.53 cfs 0.448 af

15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=4.26 cfs 0.448 af

Pond 2.1P: 24" RCP Peak Elev=51.11' Storage=18,912 cf Inflow=38.13 cfs 3.990 af

24.0" Round Culvert n=0.012 L=78.0' S=0.0135 '/' Outflow=24.09 cfs 3.990 af

Pond 2P: 12" CPP Peak Elev=60.93' Storage=11,719 cf Inflow=23.92 cfs 1.395 af

Outflow=18.70 cfs 1.395 af

Link 100L: POA #100 - SW Inflow=4.26 cfs 0.448 af

Primary=4.26 cfs 0.448 af

Link 200L: POA #200 - South Inflow=27.12 cfs 4.597 af

Primary=27.12 cfs 4.597 af

Link 300L: POA #300 - SE Inflow=16.14 cfs 1.589 af

Primary=16.14 cfs 1.589 af

Total Runoff Area = 30.826 ac Runoff Volume = 6.634 af Average Runoff Depth = 2.58" 93.16% Pervious = 28.718 ac 6.84% Impervious = 2.108 ac

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

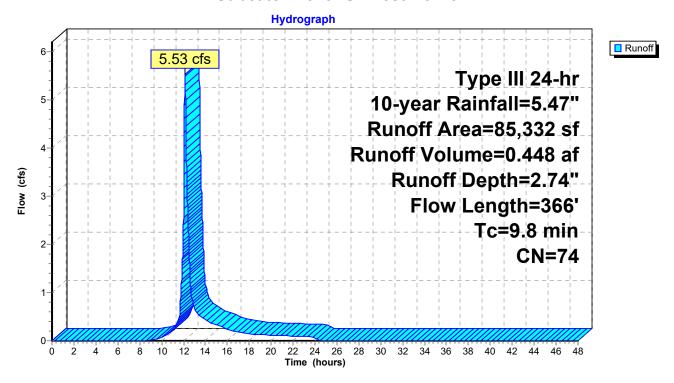
Summary for Subcatchment 1S: West Corner

Runoff = 5.53 cfs @ 12.14 hrs, Volume= 0.448 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN [Description			
		7,590	98 F	Paved roads w/curbs & sewers, HSG C			
		37,391	74 >	75% Gras	s cover, Go	ood, HSG C	
		40,320	70 V	Noods, Go	od, HSG C		
		31	55 V	Noods, Go	od, HSG B		
		85,332	74 \	Veighted A	verage		
		77,742			vious Area		
	7,590 8.89% Impervious Area			3.89% Impe	ervious Area	a	
	Tc	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	4.5	35	0.1071	0.13		Sheet Flow,	
						Woods: Light underbrush n= 0.400 P2= 3.61"	
	1.6	129	0.0388	1.38		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	0.6	55	0.0909	1.51		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	3.1	147	0.0254	0.80		Shallow Concentrated Flow,	
_						Woodland Kv= 5.0 fps	
	9.8	366	Total				

Subcatchment 1S: West Corner



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

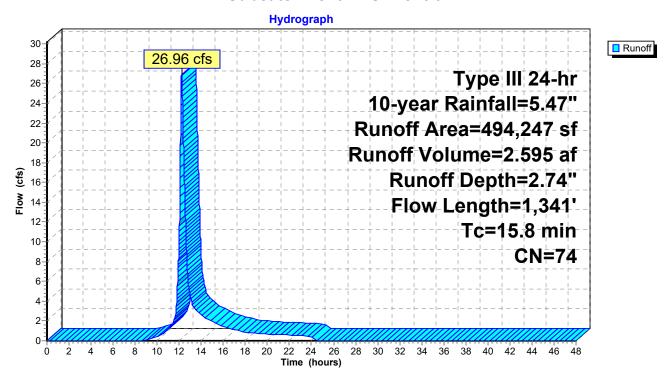
Summary for Subcatchment 2.1S: Central

Runoff = 26.96 cfs @ 12.22 hrs, Volume= 2.595 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN	Description			
		4,537	98	Paved parking, HSG D			
		33,589	98	Paved park			
		5,448	98	Roofs, HSC			
		[′] 49		Paved park			
		3,520	80	>75% Ġras			
		69,745		Woods, Go		,	
		10,593	65	Brush, Goo			
		71,371	74	>75% Gras		ood, HSG C	
		80,230		Woods, Go			
		530		>75% Gras	•	ood, HSG B	
		14,635		Woods, Go	,		
	4	94,247	74	Weighted A	verage		
	4	50,624		91.17% Per			
		43,623		8.83% Impe	ervious Area	a	
				•			
	Тс	Length	Slope		Capacity	Description	
(m	nin)	(feet)	(ft/ft) (ft/sec)	(cfs)		
(0.5	35	0.0200	1.18		Sheet Flow,	
						Smooth surfaces n= 0.011 P2= 3.61"	
4	4.7	255	0.016	5 0.90		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
2	2.0	132	0.050°	1 1.12		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
(0.4	67	0.0313	3 2.85		Shallow Concentrated Flow,	
						Unpaved Kv= 16.1 fps	
•	1.0	89	0.046	5 1.51		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
4	4.3	289	0.0500	1.12		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
2	2.9	474	0.0106	3 2.72	13.61	Trap/Vee/Rect Channel Flow,	
						Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'	
						n= 0.040 Winding stream, pools & shoals	
1:	5.8	1,341	Total				

Subcatchment 2.1S: Central



Summary for Subcatchment 2.2S: South of SQ Drive

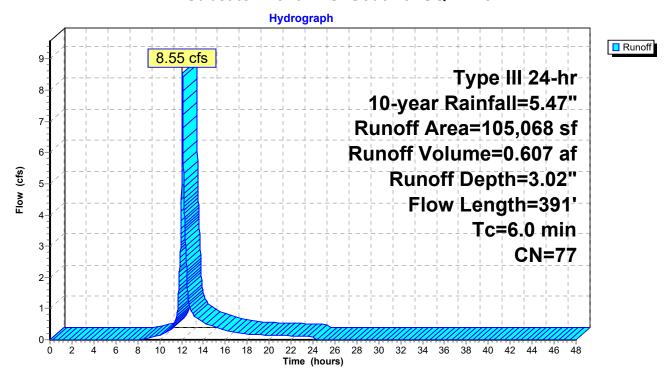
Runoff = 8.55 cfs @ 12.09 hrs, Volume= 0.607 af, Depth= 3.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	А	rea (sf)	CN [Description		
_		5,341	98 F	Paved park	ing, HSG D	
		5,978			ing, HSG C	
		53,584			od, HSG D	
		40,165		•	od, HSG C	
	1	05,068	77 V	Veighted A	verage	
	93,749 89.23% Pervious Area				vious Area	
	11,319 10.77% Impervious Are			0.77% Imp	ervious Are	ea
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	0.3	19	0.0200	1.05		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 3.61"
	1.4	118	0.0816	1.43		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.7	254	0.0502	5.93	29.63	Trap/Vee/Rect Channel Flow,
						Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'
_						n= 0.040 Winding stream, pools & shoals

2.4 391 Total, Increased to minimum Tc = 6.0 min

Subcatchment 2.2S: South of SQ Drive



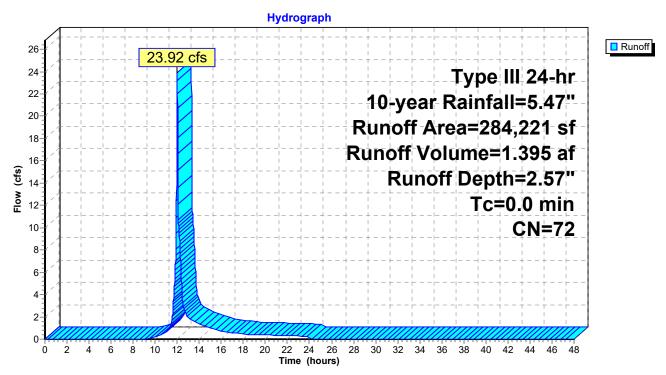
Summary for Subcatchment 2S: North Corner

Runoff = 23.92 cfs @ 12.00 hrs, Volume= 1.395 af, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

Area (sf)	CN	Description
27	98	Paved parking, HSG D
13,324	98	Paved parking, HSG C
5,932	98	Roofs, HSG C
254	80	>75% Grass cover, Good, HSG D
20,767	77	Woods, Good, HSG D
21,632	74	>75% Grass cover, Good, HSG C
199,815	70	Woods, Good, HSG C
22,470	55	Woods, Good, HSG B
284,221	72	Weighted Average
264,938		93.22% Pervious Area
19,283		6.78% Impervious Area

Subcatchment 2S: North Corner



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

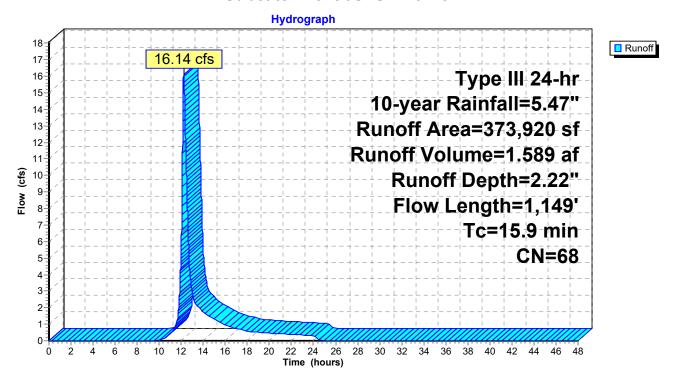
Summary for Subcatchment 3S: SE Corner

Runoff = 16.14 cfs @ 12.23 hrs, Volume= 1.589 af, Depth= 2.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN [Description		
	8,365	98 F	Paved park	ing, HSG C	
	1,632	98 F	Paved park	ing, HSG B	
	6,383	74 >	75% Gras	s cover, Go	od, HSG C
2	296,269	70 V	Voods, Go	od, HSG C	
	1,172			s cover, Go	od, HSG B
	60,099	55 V	Voods, Go	od, HSG B	
3	373,920	68 V	Veighted A	verage	
3	363,923	ç	7.33% Per	vious Area	
	9,997	2	2.67% Impe	ervious Area	a e e e e e e e e e e e e e e e e e e e
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.5	90	0.1412	0.18		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.61"
4.7	331	0.0554	1.18		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.9	331	0.0554	6.22	31.12	Trap/Vee/Rect Channel Flow,
					Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'
4.0	007	0.0400	0.00	05.00	n= 0.040 Winding stream, pools & shoals
1.8	397	0.0160	3.63	25.39	Trap/Vee/Rect Channel Flow,
					Bot.W=4.00' D=1.00' Z= 3.0 '/' Top.W=10.00'
					n= 0.040 Winding stream, pools & shoals
15.9	1,149	Total			

Subcatchment 3S: SE Corner



Summary for Reach 2.1R: Wetland Flow Path

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 18.63 cfs @ 12.05 hrs, Volume= 1.395 af

Outflow = 16.73 cfs @ 12.09 hrs, Volume= 1.395 af, Atten= 10%, Lag= 2.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.53 fps, Min. Travel Time= 2.3 min Avg. Velocity = 0.72 fps, Avg. Travel Time= 7.9 min

Peak Storage= 2,278 cf @ 12.09 hrs
Average Depth at Peak Storage= 1.19'

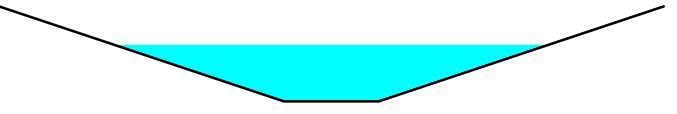
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 54.59 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

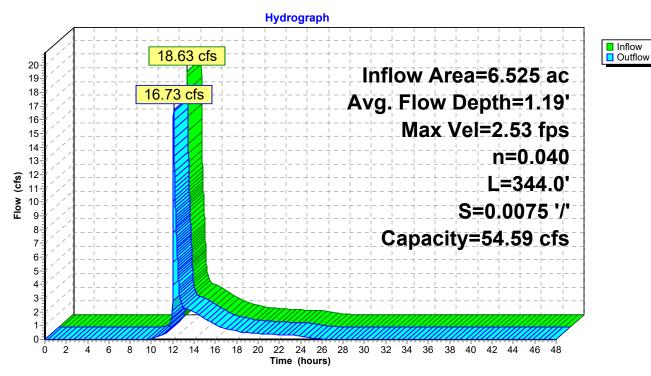
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 344.0' Slope= 0.0075 '/'

Inlet Invert= 52.58', Outlet Invert= 50.00'



Reach 2.1R: Wetland Flow Path



Summary for Reach 2.2R: Wetland Flow Path

Inflow Area = 17.871 ac, 8.08% Impervious, Inflow Depth = 2.68" for 10-year event

Inflow = 24.09 cfs @ 12.43 hrs, Volume= 3.990 af

Outflow = 24.09 cfs @ 12.44 hrs, Volume= 3.990 af, Atten= 0%, Lag= 0.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 5.61 fps, Min. Travel Time= 0.8 min Avg. Velocity = 1.93 fps, Avg. Travel Time= 2.3 min

Peak Storage= 1,129 cf @ 12.44 hrs Average Depth at Peak Storage= 0.91'

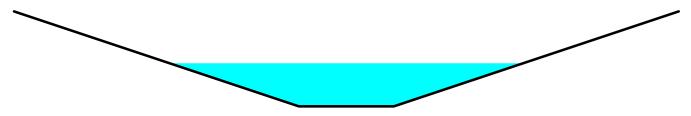
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 141.18 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

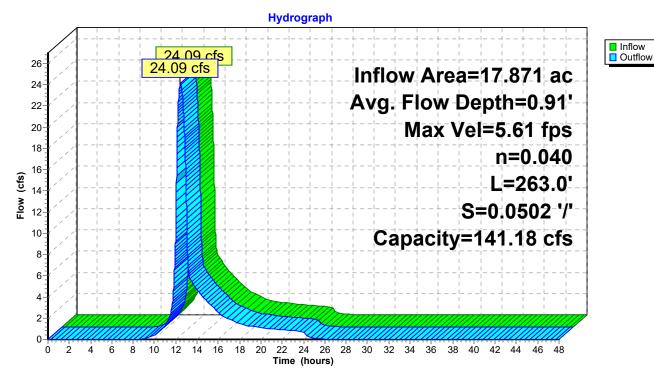
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 263.0' Slope= 0.0502 '/'

Inlet Invert= 46.52', Outlet Invert= 33.33'



Reach 2.2R: Wetland Flow Path



Summary for Reach 2R: Wetland Flow Path

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 18.70 cfs @ 12.05 hrs, Volume= 1.395 af

Outflow = 18.63 cfs @ 12.05 hrs, Volume= 1.395 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 5.02 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.45 fps, Avg. Travel Time= 1.7 min

Peak Storage= 560 cf @ 12.05 hrs Average Depth at Peak Storage= 0.83'

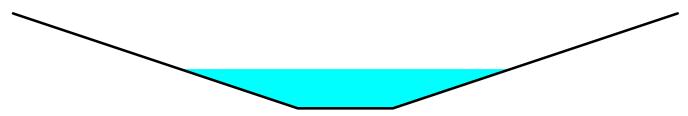
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 133.09 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

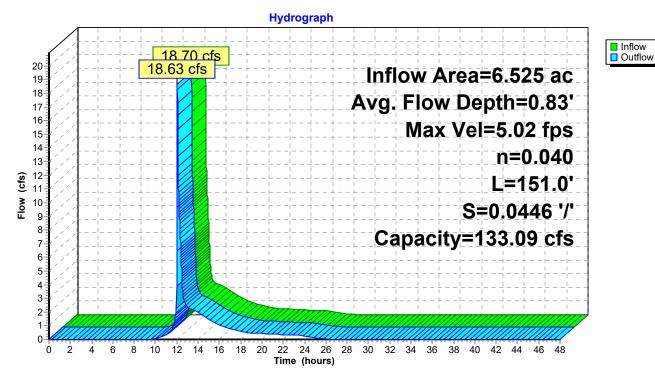
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 151.0' Slope= 0.0446 '/'

Inlet Invert= 59.31', Outlet Invert= 52.58'



Reach 2R: Wetland Flow Path



Summary for Reach 23R: Wetland Flow Path

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 16.73 cfs @ 12.09 hrs, Volume= 1.395 af

Outflow = 16.62 cfs @ 12.10 hrs, Volume= 1.395 af, Atten= 1%, Lag= 0.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.53 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.03 fps, Avg. Travel Time= 2.1 min

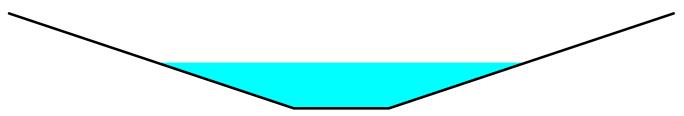
Peak Storage= 617 cf @ 12.10 hrs Average Depth at Peak Storage= 0.96' Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 85.86 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

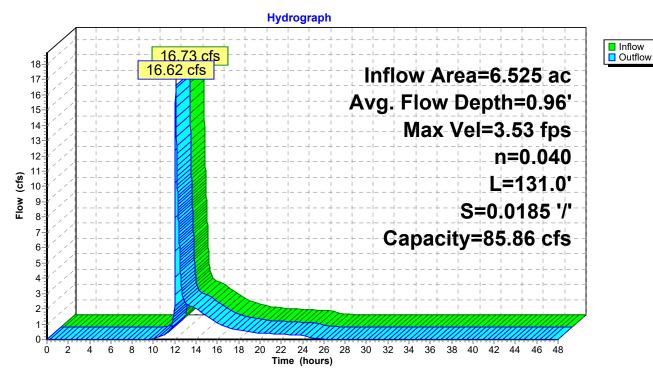
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 131.0' Slope= 0.0185 '/'

Inlet Invert= 50.00', Outlet Invert= 47.57'



Reach 23R: Wetland Flow Path



Prepared by Altus Engineering, Inc.

Printed 7/9/2025 HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 1P: 15" RCP

Inflow Area = 1.959 ac, 8.89% Impervious, Inflow Depth = 2.74" for 10-year event

5.53 cfs @ 12.14 hrs, Volume= Inflow 0.448 af

4.26 cfs @ 12.23 hrs, Volume= Outflow = 0.448 af, Atten= 23%, Lag= 5.5 min

4.26 cfs @ 12.23 hrs, Volume= Primary 0.448 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 53.65' @ 12.23 hrs Surf.Area= 2,904 sf Storage= 1,011 cf

Plug-Flow detention time= 1.4 min calculated for 0.448 af (100% of inflow)

Center-of-Mass det. time= 1.3 min (837.9 - 836.6)

Volume	Invert	Avail.Storage	Storage Description
#1	52.26'	9,237 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

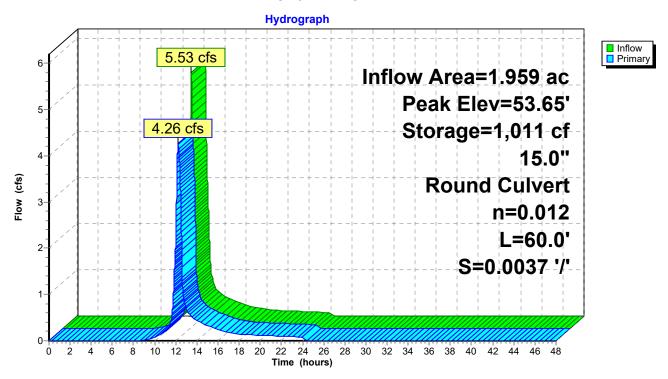
Elevation	Surt.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
52.26	10	0	0
53.00	85	35	35
54.00	4,401	2,243	2,278
55.00	9,516	6,959	9,237

Device	Routing	Invert	Outlet Devices
#1	Primary	52.26'	15.0" Round Culvert

L= 60.0' RCP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 52.26' / 52.04' S= 0.0037 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=4.26 cfs @ 12.23 hrs HW=53.65' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.26 cfs @ 3.89 fps)

Pond 1P: 15" RCP



Prepared by Altus Engineering, Inc.

Printed 7/9/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 2.1P: 24" RCP

Inflow Area = 17.871 ac, 8.08% Impervious, Inflow Depth = 2.68" for 10-year event

Inflow = 38.13 cfs @ 12.19 hrs, Volume= 3.990 af

Outflow = 24.09 cfs @ 12.43 hrs, Volume= 3.990 af, Atten= 37%, Lag= 14.6 min

Primary = 24.09 cfs @ 12.43 hrs, Volume= 3.990 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 51.11' @ 12.43 hrs Surf.Area= 16,584 sf Storage= 18,912 cf

Plug-Flow detention time= 4.8 min calculated for 3.990 af (100% of inflow) Center-of-Mass det. time= 4.7 min (861.2 - 856.5)

Volume	Invert	Avail.Storage	Storage Description
#1	47.57'	80,071 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
47.57	100	0	0
48.00	164	57	57
49.00	2,506	1,335	1,392
50.00	7,196	4,851	6,243
51.00	14,807	11,002	17,244
52.00	31,534	23,171	40,415
53.00	47,779	39,657	80,071

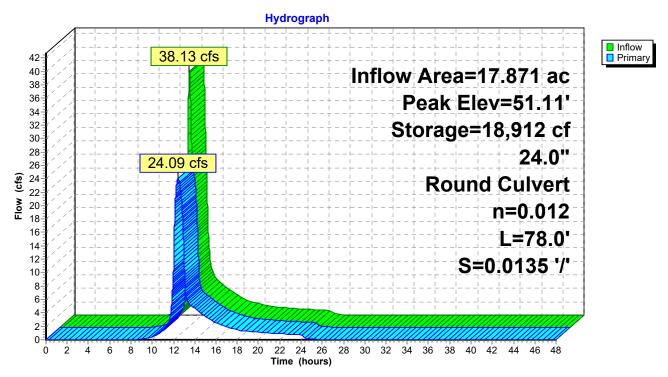
Device Routing Invert Outlet Devices

#1 Primary 47.57' 24.0" Round Culvert

L= 78.0' RCP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 47.57' / 46.52' S= 0.0135 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

Primary OutFlow Max=24.09 cfs @ 12.43 hrs HW=51.11' TW=47.43' (Dynamic Tailwater) 1=Culvert (Inlet Controls 24.09 cfs @ 7.67 fps)

Pond 2.1P: 24" RCP



Prepared by Altus Engineering, Inc.

Printed 7/9/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 2P: 12" CPP

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 23.92 cfs @ 12.00 hrs, Volume= 1.395 af

Outflow = 18.70 cfs @ 12.05 hrs, Volume= 1.395 af, Atten= 22%, Lag= 2.6 min

Primary = 18.70 cfs @ 12.05 hrs, Volume= 1.395 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 60.93' @ 12.05 hrs Surf.Area= 15,490 sf Storage= 11,719 cf

Plug-Flow detention time= 44.2 min calculated for 1.395 af (100% of inflow) Center-of-Mass det. time= 44.1 min (876.6 - 832.4)

VolumeInvertAvail.StorageStorage Description#159.65'35,764 cfCustom Stage Data (Prismatic)Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
59.65	100	0	0
60.00	6,996	1,242	1,242
61.00	16,111	11,554	12,795
62.00	29,827	22,969	35,764

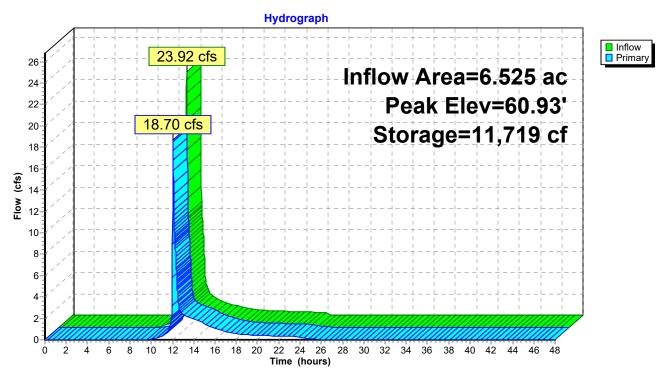
Device	Routing	Invert	Outlet Devices
#1	Primary	59.65'	12.0" Round Culvert
			L= 9.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 59.65' / 59.31' S= 0.0378 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
#2	Primary	60.75'	87.0' long x 4.0' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66
			2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Primary OutFlow Max=18.66 cfs @ 12.05 hrs HW=60.93' TW=60.13' (Dynamic Tailwater)

1=Culvert (Inlet Controls 2.64 cfs @ 3.36 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 16.02 cfs @ 1.01 fps)





Summary for Link 100L: POA #100 - SW

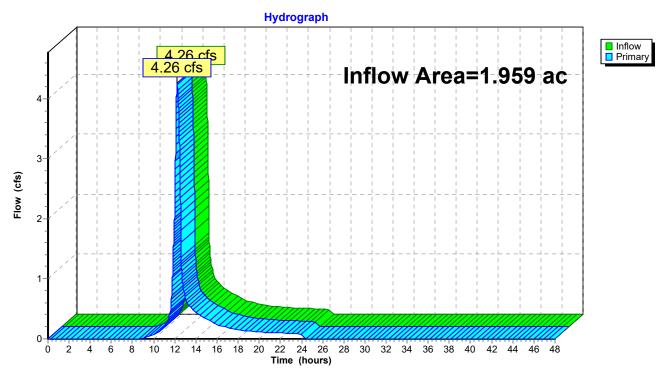
Inflow Area = 1.959 ac, 8.89% Impervious, Inflow Depth = 2.74" for 10-year event

Inflow = 4.26 cfs @ 12.23 hrs, Volume= 0.448 af

Primary = 4.26 cfs @ 12.23 hrs, Volume= 0.448 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 100L: POA #100 - SW



Summary for Link 200L: POA #200 - South

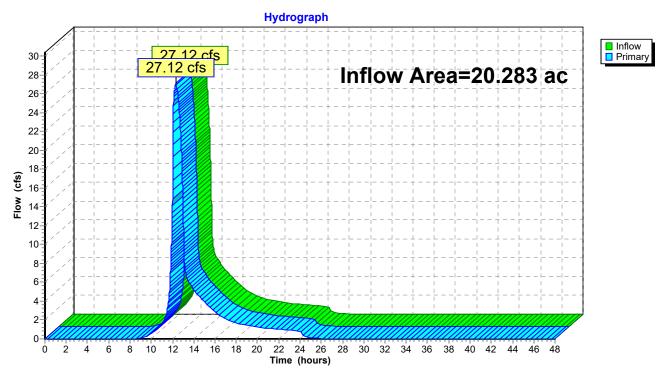
Inflow Area = 20.283 ac, 8.40% Impervious, Inflow Depth = 2.72" for 10-year event

Inflow = 27.12 cfs @ 12.34 hrs, Volume= 4.597 af

Primary = 27.12 cfs @ 12.34 hrs, Volume= 4.597 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 200L: POA #200 - South



Summary for Link 300L: POA #300 - SE

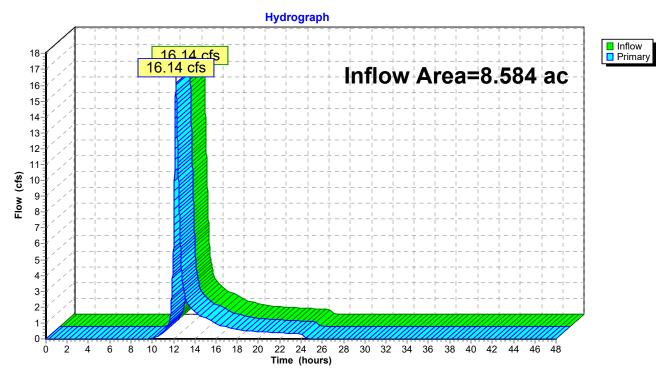
Inflow Area = 8.584 ac, 2.67% Impervious, Inflow Depth = 2.22" for 10-year event

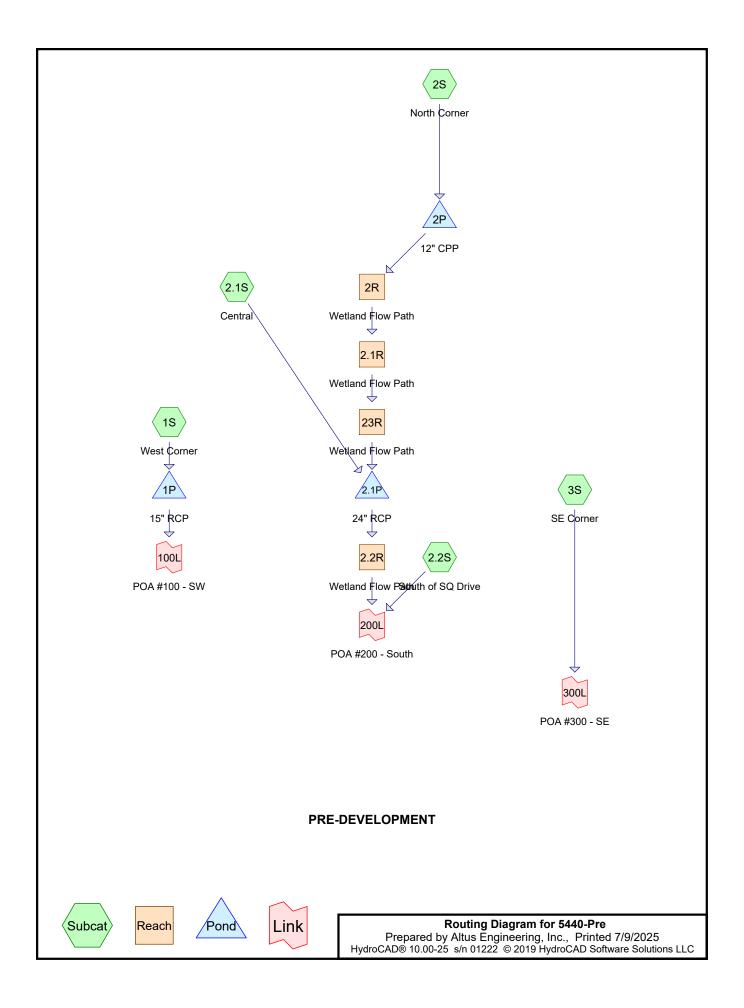
Inflow = 16.14 cfs @ 12.23 hrs, Volume= 1.589 af

Primary = 16.14 cfs @ 12.23 hrs, Volume= 1.589 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 300L: POA #300 - SE





Printed 7/9/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: West Corner Runoff Area=85,332 sf 8.89% Impervious Runoff Depth=3.98"

Flow Length=366' Tc=9.8 min CN=74 Runoff=8.04 cfs 0.650 af

Subcatchment 2.1S: Central Runoff Area=494,247 sf 8.83% Impervious Runoff Depth=3.98"

Flow Length=1,341' Tc=15.8 min CN=74 Runoff=39.25 cfs 3.764 af

Subcatchment 2.2S: South of SQ Drive Runoff Area=105,068 sf 10.77% Impervious Runoff Depth=4.30"

Flow Length=391' Tc=6.0 min CN=77 Runoff=12.13 cfs 0.865 af

Subcatchment 2S: North Corner Runoff Area=284,221 sf 6.78% Impervious Runoff Depth=3.77"

Tc=0.0 min CN=72 Runoff=35.33 cfs 2.050 af

Subcatchment 3S: SE Corner Runoff Area=373,920 sf 2.67% Impervious Runoff Depth=3.35"

Flow Length=1,149' Tc=15.9 min CN=68 Runoff=24.80 cfs 2.399 af

Reach 2.1R: Wetland Flow Path

Avg. Flow Depth=1.53' Max Vel=2.91 fps Inflow=31.17 cfs 2.050 af

n=0.040 L=344.0' S=0.0075 '/' Capacity=54.59 cfs Outflow=29.25 cfs 2.050 af

Reach 2.2R: Wetland Flow Path Avg. Flow Depth=0.98' Max Vel=5.85 fps Inflow=28.17 cfs 5.814 af

n=0.040 L=263.0' S=0.0502 '/' Capacity=141.18 cfs Outflow=28.17 cfs 5.814 af

Reach 2R: Wetland Flow Path Avg. Flow Depth=1.05' Max Vel=5.74 fps Inflow=31.29 cfs 2.050 af

n=0.040 L=151.0' S=0.0446 '/' Capacity=133.09 cfs Outflow=31.17 cfs 2.050 af

Reach 23R: Wetland Flow Path Avg. Flow Depth=1.25' Max Vel=4.08 fps Inflow=29.25 cfs 2.050 af

n=0.040 L=131.0' S=0.0185'/' Capacity=85.86 cfs Outflow=29.14 cfs 2.050 af

Pond 1P: 15" RCP Peak Elev=54.02' Storage=2,363 cf Inflow=8.04 cfs 0.650 af

15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=5.12 cfs 0.650 af

Pond 2.1P: 24" RCP Peak Elev=52.04' Storage=41,604 cf Inflow=55.24 cfs 5.814 af

24.0" Round Culvert n=0.012 L=78.0' S=0.0135 '/' Outflow=28.17 cfs 5.814 af

Pond 2P: 12" CPP Peak Elev=61.01' Storage=13,034 cf Inflow=35.33 cfs 2.050 af

Outflow=31.29 cfs 2.050 af

Link 100L: POA #100 - SW Inflow=5.12 cfs 0.650 af

Primary=5.12 cfs 0.650 af

Link 200L: POA #200 - South Inflow=34.48 cfs 6.679 af

Primary=34.48 cfs 6.679 af

Link 300L: POA #300 - SE Inflow=24.80 cfs 2.399 af

Primary=24.80 cfs 2.399 af

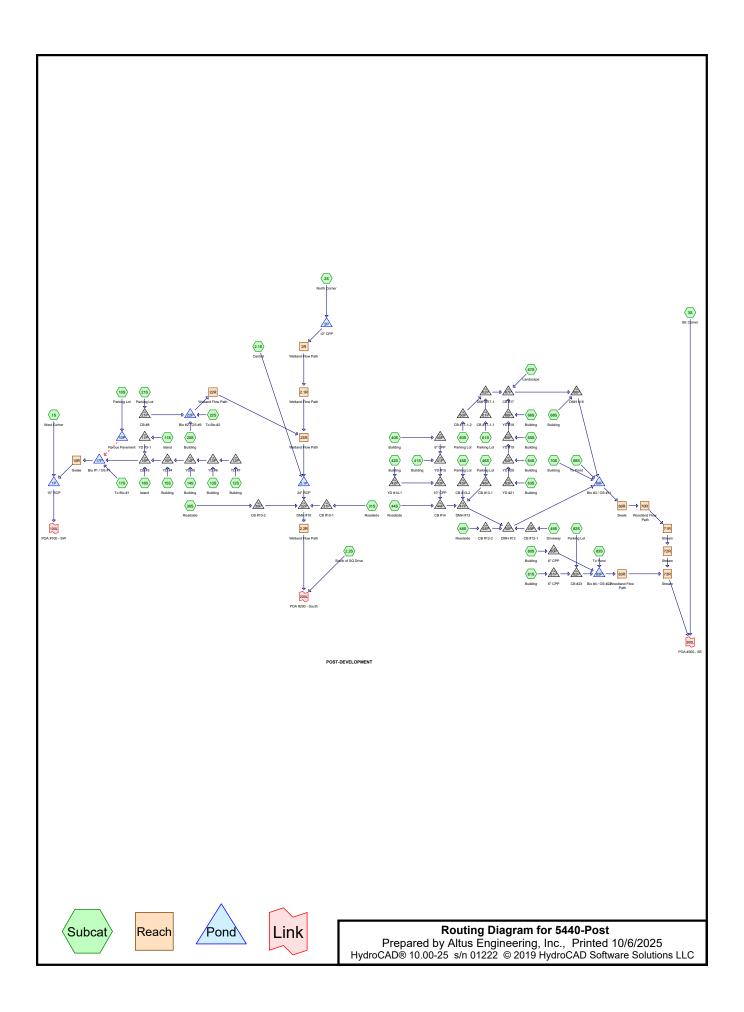
Total Runoff Area = 30.826 ac Runoff Volume = 9.728 af Average Runoff Depth = 3.79" 93.16% Pervious = 28.718 ac 6.84% Impervious = 2.108 ac

Section 4

Drainage Calculations

Post-Development
2-Year, 24-Hour Summary
10-Year, 24-Hour Complete
25-Year, 24-Hour Summary
100-Year, 24-Hour Pond Reports





Tc=6.0 min CN=74 Runoff=0.00 cfs 0.000 af

5440-Post

Prepared by Altus Engineering, Inc. HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Runoff Area=66,034 sf 9.68% Impervious Runoff Depth=0.03" Subcatchment 1S: West Corner Flow Length=366' Tc=9.8 min CN=75 Runoff=0.01 cfs 0.004 af Runoff Area=444,574 sf 8.10% Impervious Runoff Depth=0.02" Subcatchment 2.1S: Central Flow Length=1,341' Tc=15.8 min CN=74 Runoff=0.03 cfs 0.020 af Runoff Area=102,088 sf 10.13% Impervious Runoff Depth=0.05" Subcatchment 2.2S: South of SQ Drive Flow Length=391' Tc=6.0 min CN=77 Runoff=0.03 cfs 0.009 af Runoff Area=284,221 sf 6.78% Impervious Runoff Depth=0.01" Subcatchment 2S: North Corner Tc=0.0 min CN=72 Runoff=0.01 cfs 0.007 af Runoff Area=306,834 sf 2.19% Impervious Runoff Depth=0.00" Subcatchment 3S: SE Corner Flow Length=1,149' Tc=15.9 min CN=68 Runoff=0.00 cfs 0.000 af Runoff Area=10,182 sf 98.98% Impervious Runoff Depth>0.79" Subcatchment 10S: Parking Lot Tc=790.0 min CN=98 Runoff=0.01 cfs 0.015 af Runoff Area=588 sf 65.82% Impervious Runoff Depth=0.32" Subcatchment 11S: Island Tc=6.0 min CN=90 Runoff=0.00 cfs 0.000 af Runoff Area=1,878 sf 65.12% Impervious Runoff Depth=0.32" Subcatchment 12S: Building Tc=6.0 min CN=90 Runoff=0.02 cfs 0.001 af Runoff Area=1,349 sf 73.83% Impervious Runoff Depth=0.40" Subcatchment 13S: Building Tc=6.0 min CN=92 Runoff=0.01 cfs 0.001 af Runoff Area=1,021 sf 77.86% Impervious Runoff Depth=0.45" Subcatchment 14S: Building Tc=6.0 min CN=93 Runoff=0.01 cfs 0.001 af Runoff Area=1,376 sf 43.24% Impervious Runoff Depth=0.15" Subcatchment 15S: Building Tc=6.0 min CN=84 Runoff=0.00 cfs 0.000 af Runoff Area=928 sf 26.51% Impervious Runoff Depth=0.08" Subcatchment 16S: Island Tc=6.0 min CN=80 Runoff=0.00 cfs 0.000 af Subcatchment 17S: To Bio #1 Runoff Area=2,893 sf 0.28% Impervious Runoff Depth=0.02" Tc=6.0 min CN=74 Runoff=0.00 cfs 0.000 af Runoff Area=9,168 sf 100.00% Impervious Runoff Depth=0.79" Subcatchment 20S: Building Tc=6.0 min CN=98 Runoff=0.19 cfs 0.014 af Runoff Area=2,610 sf 100.00% Impervious Runoff Depth=0.79" **Subcatchment 21S: Parking Lot** Tc=6.0 min CN=98 Runoff=0.05 cfs 0.004 af Subcatchment 22S: To Bio #2 Runoff Area=3,961 sf 0.00% Impervious Runoff Depth=0.02"

5440-Post

Subcatchment 30S: Roadside	Runoff Area=8,230 sf 53.10% Impervious Runoff Depth=0.22" Flow Length=204' Tc=6.0 min CN=87 Runoff=0.04 cfs 0.004 af
Subcatchment 31S: Roadside	Runoff Area=7,135 sf 71.11% Impervious Runoff Depth=0.36" Flow Length=143' Tc=6.0 min CN=91 Runoff=0.07 cfs 0.005 af
Subcatchment 40S: Building	Runoff Area=3,066 sf 100.00% Impervious Runoff Depth=0.79" Tc=6.0 min CN=98 Runoff=0.06 cfs 0.005 af
Subcatchment 41S: Building	Runoff Area=2,125 sf 90.78% Impervious Runoff Depth=0.63" Tc=6.0 min CN=96 Runoff=0.04 cfs 0.003 af
Subcatchment 42S: Building	Runoff Area=3,298 sf 74.41% Impervious Runoff Depth=0.40" Tc=6.0 min CN=92 Runoff=0.04 cfs 0.003 af
Subcatchment 44S: Roadside	Runoff Area=4,663 sf 40.92% Impervious Runoff Depth=0.15" Tc=6.0 min CN=84 Runoff=0.01 cfs 0.001 af
Subcatchment 45S: Parking Lot	Runoff Area=3,860 sf 71.71% Impervious Runoff Depth=0.36" Tc=6.0 min CN=91 Runoff=0.04 cfs 0.003 af
Subcatchment 46S: Parking Lot	Runoff Area=4,489 sf 73.25% Impervious Runoff Depth=0.40" Tc=6.0 min CN=92 Runoff=0.05 cfs 0.003 af
Subcatchment 48S: Roadside	Runoff Area=3,415 sf 55.67% Impervious Runoff Depth=0.25" Tc=6.0 min CN=88 Runoff=0.02 cfs 0.002 af
Subcatchment 49S: Driveway	Runoff Area=3,657 sf 83.78% Impervious Runoff Depth=0.45" Tc=6.0 min CN=93 Runoff=0.04 cfs 0.003 af
Subcatchment 60S: Parking Lot	Runoff Area=9,169 sf 78.55% Impervious Runoff Depth=0.45" Flow Length=116' Tc=6.0 min CN=93 Runoff=0.11 cfs 0.008 af
Subcatchment 61S: Parking Lot	Runoff Area=7,068 sf 91.98% Impervious Runoff Depth=0.63" Flow Length=111' Tc=6.0 min CN=96 Runoff=0.12 cfs 0.009 af
Subcatchment 63S: Building	Runoff Area=1,459 sf 65.52% Impervious Runoff Depth=0.32" Tc=6.0 min CN=90 Runoff=0.01 cfs 0.001 af
Subcatchment 64S: Building	Runoff Area=1,507 sf 81.22% Impervious Runoff Depth=0.45" Tc=6.0 min CN=93 Runoff=0.02 cfs 0.001 af
Subcatchment 65S: Building	Runoff Area=982 sf 80.86% Impervious Runoff Depth=0.45" Tc=6.0 min CN=93 Runoff=0.01 cfs 0.001 af
Subcatchment 66S: Building	Runoff Area=1,478 sf 44.79% Impervious Runoff Depth=0.11" Tc=6.0 min CN=82 Runoff=0.00 cfs 0.000 af
Subcatchment 67S: Landscape	Runoff Area=1,430 sf 23.15% Impervious Runoff Depth=0.00" Tc=6.0 min CN=70 Runoff=0.00 cfs 0.000 af
Subcatchment 68S: Building	Runoff Area=1,370 sf 100.00% Impervious Runoff Depth=0.79" Tc=6.0 min CN=98 Runoff=0.03 cfs 0.002 af

5440-Post Prepared by Altus

Reach 73R: Stream

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Runoff Area=13,261 sf 1.69% Impervious Runoff Depth=0.00" Subcatchment 69S: To Pond Tc=6.0 min CN=68 Runoff=0.00 cfs 0.000 af Subcatchment 70S: Building Runoff Area=6,906 sf 100.00% Impervious Runoff Depth=0.79" Tc=6.0 min CN=98 Runoff=0.14 cfs 0.010 af Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=0.79" Subcatchment 80S: Building Tc=6.0 min CN=98 Runoff=0.02 cfs 0.002 af Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=0.79" Subcatchment81S: Building Tc=6.0 min CN=98 Runoff=0.02 cfs 0.002 af Runoff Area=9,597 sf 90.39% Impervious Runoff Depth=0.56" Subcatchment 82S: Parking Lot Flow Length=214' Tc=6.0 min CN=95 Runoff=0.15 cfs 0.010 af Runoff Area=2,507 sf 0.00% Impervious Runoff Depth=0.02" Subcatchment 83S: To Pond Tc=6.0 min CN=74 Runoff=0.00 cfs 0.000 af Avg. Flow Depth=0.02' Max Vel=0.23 fps Inflow=0.01 cfs 0.007 af Reach 2.1R: Wetland Flow Path n=0.040 L=344.0' S=0.0075 '/' Capacity=54.59 cfs Outflow=0.01 cfs 0.007 af Avg. Flow Depth=0.05' Max Vel=1.04 fps Inflow=0.11 cfs 0.052 af Reach 2.2R: Wetland Flow Path n=0.040 L=263.0' S=0.0502'/' Capacity=141.18 cfs Outflow=0.10 cfs 0.052 af Avg. Flow Depth=0.01' Max Vel=0.57 fps Inflow=0.01 cfs 0.007 af Reach 2R: Wetland Flow Path n=0.040 L=151.0' S=0.0446'/' Capacity=133.09 cfs Outflow=0.01 cfs 0.007 af Avg. Flow Depth=0.01' Max Vel=0.14 fps Inflow=0.00 cfs 0.004 af Reach 10R: Swale n=0.035 L=44.0' S=0.0050 '/' Capacity=16.22 cfs Outflow=0.00 cfs 0.004 af Reach 22R: Wetland Flow Path Avg. Flow Depth=0.00' Max Vel=0.42 fps Inflow=0.01 cfs 0.018 af n=0.040 L=211.0' S=0.0245'/' Capacity=392.58 cfs Outflow=0.01 cfs 0.018 af Avg. Flow Depth=0.02' Max Vel=0.43 fps Inflow=0.02 cfs 0.024 af Reach 23R: Wetland Flow Path n=0.040 L=131.0' S=0.0185 '/' Capacity=85.86 cfs Outflow=0.02 cfs 0.024 af Avg. Flow Depth=0.03' Max Vel=0.25 fps Inflow=0.03 cfs 0.050 af Reach 69R: Swale n=0.069 L=17.0' S=0.0147'/' Capacity=14.11 cfs Outflow=0.03 cfs 0.050 af Avg. Flow Depth=0.02' Max Vel=0.12 fps Inflow=0.03 cfs 0.050 af Reach 70R: Woodland Flow Path n=0.400 L=57.0' S=0.1404 '/' Capacity=21.20 cfs Outflow=0.03 cfs 0.049 af Avg. Flow Depth=0.02' Max Vel=0.67 fps Inflow=0.03 cfs 0.049 af Reach 71R: Stream n=0.040 L=80.0' S=0.0554'/' Capacity=31.12 cfs Outflow=0.03 cfs 0.049 af Reach 72R: Stream Avg. Flow Depth=0.02' Max Vel=0.35 fps Inflow=0.03 cfs 0.049 af n=0.040 L=88.0' S=0.0160 '/' Capacity=25.40 cfs Outflow=0.03 cfs 0.049 af

Avg. Flow Depth=0.03' Max Vel=0.41 fps Inflow=0.04 cfs 0.063 af

n=0.040 L=309.0' S=0.0160 '/' Capacity=25.38 cfs Outflow=0.04 cfs 0.063 af

Avg. Flow Depth=0.02' Max Vel=0.08 fps Inflow=0.01 cfs 0.014 af Reach 83R: Woodland Flow Path n=0.400 L=122.0' S=0.0950 '/' Capacity=17.44 cfs Outflow=0.01 cfs 0.014 af **Pond 1P: 15" RCP** Peak Elev=52.32' Storage=1 cf Inflow=0.01 cfs 0.008 af 15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=0.01 cfs 0.008 af Peak Elev=47.66' Storage=9 cf Inflow=0.05 cfs 0.044 af Pond 2.1P: 24" RCP 24.0" Round Culvert n=0.012 L=17.0' S=0.0135 '/' Outflow=0.05 cfs 0.044 af **Pond 2P: 12" CPP** Peak Elev=59.70' Storage=30 cf Inflow=0.01 cfs 0.007 af Outflow=0.01 cfs 0.007 af Peak Elev=56.05' Storage=5 cf Inflow=0.01 cfs 0.015 af **Pond 10P: Porous Pavement** Discarded=0.01 cfs 0.015 af Secondary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.015 af Peak Elev=57.21' Inflow=0.00 cfs 0.000 af Pond 11P: YD #3-1 6.0" Round Culvert n=0.012 L=36.0' S=0.0050 '/' Outflow=0.00 cfs 0.000 af Pond 12P: YD #7 Peak Elev=57.84' Inflow=0.02 cfs 0.001 af 8.0" Round Culvert n=0.012 L=52.0' S=0.0050 '/' Outflow=0.02 cfs 0.001 af Pond 13P: YD #6 Peak Elev=57.43' Inflow=0.03 cfs 0.002 af 10.0" Round Culvert n=0.012 L=29.0' S=0.0052 '/' Outflow=0.03 cfs 0.002 af Peak Elev=57.20' Inflow=0.04 cfs 0.003 af Pond 14P: YD #5 10.0" Round Culvert n=0.012 L=20.0' S=0.0050 '/' Outflow=0.04 cfs 0.003 af Pond 15P: YD #4 Peak Elev=56.93' Inflow=0.05 cfs 0.003 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.05 cfs 0.003 af Pond 16P: CB #3 Peak Elev=56.61' Inflow=0.05 cfs 0.004 af 12.0" Round Culvert n=0.012 L=45.0' S=0.0049 '/' Outflow=0.05 cfs 0.004 af Pond 17P: Bio #1 / OS #1 Peak Elev=56.42' Storage=126 cf Inflow=0.05 cfs 0.004 af Outflow=0.00 cfs 0.004 af Peak Elev=58.16' Inflow=0.05 cfs 0.004 af Pond 21P: CB #8 12.0" Round Culvert n=0.012 L=22.0' S=0.0050 '/' Outflow=0.05 cfs 0.004 af Peak Elev=58.16' Storage=546 cf Inflow=0.24 cfs 0.018 af Pond 22P: Bio #2 / OS #9 Outflow=0.01 cfs 0.018 af Pond 30P: CB #10-2 Peak Elev=52.75' Inflow=0.04 cfs 0.004 af 12.0" Round Culvert n=0.012 L=78.0' S=0.0397 '/' Outflow=0.04 cfs 0.004 af Pond 31P: CB #10-1 Peak Elev=49.73' Inflow=0.07 cfs 0.005 af 12.0" Round Culvert n=0.012 L=2.0' S=0.0250 '/' Outflow=0.07 cfs 0.005 af Peak Elev=47.42' Inflow=0.11 cfs 0.052 af Pond 32P: DMH #10 24.0" Round Culvert n=0.012 L=57.0' S=0.0135 '/' Outflow=0.11 cfs 0.052 af Pond 40P: 6" CPP Peak Elev=53.73' Inflow=0.06 cfs 0.005 af

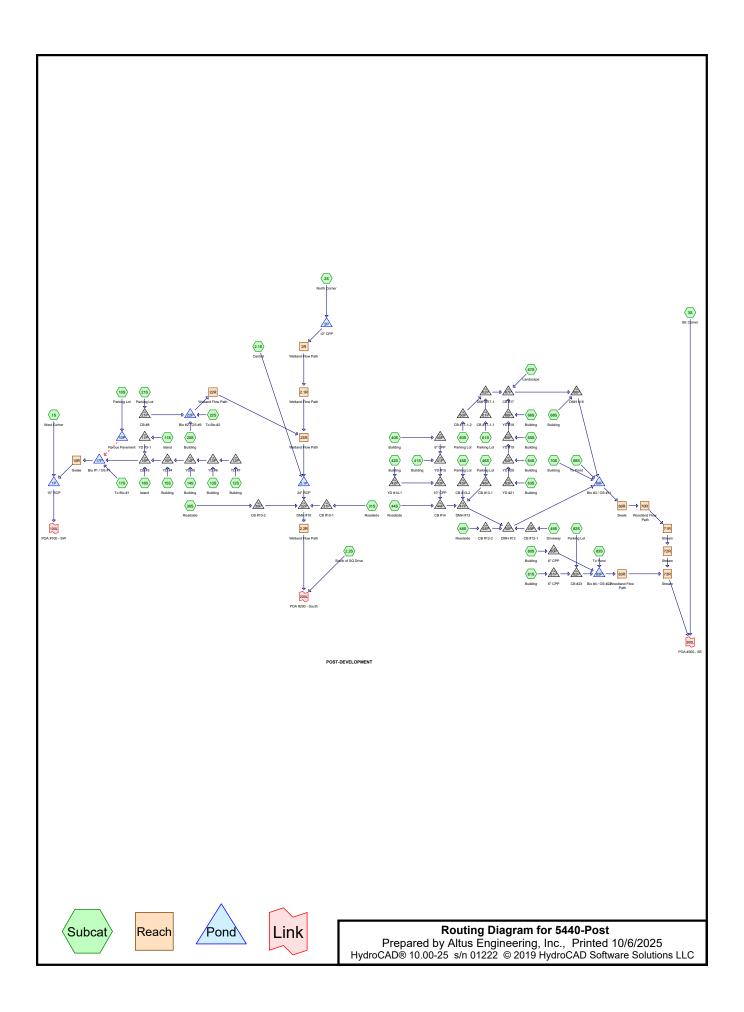
6.0" Round Culvert n=0.012 L=174.0' S=0.0150 '/' Outflow=0.06 cfs 0.005 af

5440-Post

Pond 41P: YD #15	Peak Elev=50.97' Inflow=0.10 cfs 0.007 af 8.0" Round Culvert n=0.012 L=47.0' S=0.0149 '/' Outflow=0.10 cfs 0.007 af
Pond 42P: YD #14-1	Peak Elev=54.06' Inflow=0.04 cfs 0.003 af 6.0" Round Culvert n=0.012 L=158.0' S=0.0175 '/' Outflow=0.04 cfs 0.003 af
Pond 43P: 10" CPP	Peak Elev=50.30' Inflow=0.13 cfs 0.010 af 8.0" Round Culvert n=0.012 L=33.0' S=0.0148 '/' Outflow=0.13 cfs 0.010 af
Pond 44P: CB #14	Peak Elev=49.27' Inflow=0.15 cfs 0.011 af 12.0" Round Culvert n=0.012 L=31.0' S=0.0048 '/' Outflow=0.15 cfs 0.011 af
Pond 45P: CB #13-2	Peak Elev=49.34' Inflow=0.04 cfs 0.003 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0164 '/' Outflow=0.04 cfs 0.003 af
Pond 46P: CB #13-1	Peak Elev=49.10' Inflow=0.05 cfs 0.003 af 12.0" Round Culvert n=0.012 L=12.0' S=0.0050 '/' Outflow=0.05 cfs 0.003 af
Pond 47P: DMH #13	Peak Elev=49.05' Inflow=0.23 cfs 0.017 af 15.0" Round Culvert n=0.012 L=67.0' S=0.0040 '/' Outflow=0.23 cfs 0.017 af
Pond 48P: CB #12-2	Peak Elev=48.78' Inflow=0.02 cfs 0.002 af 12.0" Round Culvert n=0.012 L=6.0' S=0.0050 '/' Outflow=0.02 cfs 0.002 af
Pond 49P: CB #12-1	Peak Elev=49.02' Inflow=0.04 cfs 0.003 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.04 cfs 0.003 af
Pond 50P: DMH #12	Peak Elev=48.78' Inflow=0.29 cfs 0.022 af 15.0" Round Culvert n=0.012 L=103.0' S=0.0040 '/' Outflow=0.29 cfs 0.022 af
Pond 60P: CB #17-1-2	Peak Elev=50.60' Inflow=0.11 cfs 0.008 af 12.0" Round Culvert n=0.012 L=21.0' S=0.0052'/' Outflow=0.11 cfs 0.008 af
Pond 61P: CB #17-1-1	Peak Elev=50.57' Inflow=0.12 cfs 0.009 af 12.0" Round Culvert n=0.012 L=3.0' S=0.0367 '/' Outflow=0.12 cfs 0.009 af
Pond 62P: DMH #17-1	Peak Elev=50.46' Inflow=0.23 cfs 0.016 af 12.0" Round Culvert n=0.012 L=25.0' S=0.0052'/' Outflow=0.23 cfs 0.016 af
Pond 63P: YD #21	Peak Elev=51.76' Inflow=0.01 cfs 0.001 af 12.0" Round Culvert n=0.012 L=49.0' S=0.0049 '/' Outflow=0.01 cfs 0.001 af
Pond 64P: YD #20	Peak Elev=51.38' Inflow=0.03 cfs 0.002 af 10.0" Round Culvert n=0.012 L=35.0' S=0.0049 '/' Outflow=0.03 cfs 0.002 af
Pond 65P: YD #19	Peak Elev=51.12' Inflow=0.04 cfs 0.003 af 10.0" Round Culvert n=0.012 L=42.0' S=0.0050 '/' Outflow=0.04 cfs 0.003 af
Pond 66P: YD #18	Peak Elev=50.72' Inflow=0.04 cfs 0.003 af 12.0" Round Culvert n=0.012 L=11.0' S=0.0509 '/' Outflow=0.04 cfs 0.003 af

Pond 67P: CB #17	Peak Elev=50.25' Inflow=0.27 cfs 0.020 af 12.0" Round Culvert n=0.012 L=74.0' S=0.0050 '/' Outflow=0.27 cfs 0.020 af
Pond 68P: DMH #16	Peak Elev=49.76' Inflow=0.30 cfs 0.022 af 12.0" Round Culvert n=0.012 L=90.0' S=0.0110 '/' Outflow=0.30 cfs 0.022 af
Pond 69P: Bio #3 / OS #11	Peak Elev=48.78' Storage=1,650 cf Inflow=0.74 cfs 0.054 af Outflow=0.03 cfs 0.050 af
Pond 80P: 6" CPP	Peak Elev=49.48' Inflow=0.02 cfs 0.002 af 6.0" Round Culvert n=0.012 L=111.0' S=0.0350 '/' Outflow=0.02 cfs 0.002 af
Pond 81P: 6" CPP	Peak Elev=49.42' Inflow=0.02 cfs 0.002 af 6.0" Round Culvert n=0.012 L=85.0' S=0.0351 '/' Outflow=0.02 cfs 0.002 af
Pond 82P: CB #23	Peak Elev=46.28' Inflow=0.17 cfs 0.012 af 12.0" Round Culvert n=0.012 L=18.0' S=0.0194 '/' Outflow=0.17 cfs 0.012 af
Pond 83P: Bio #4 / OS #22	Peak Elev=46.28' Storage=358 cf Inflow=0.19 cfs 0.014 af Outflow=0.01 cfs 0.014 af
Link 100L: POA #100 - SW	Inflow=0.01 cfs 0.008 af Primary=0.01 cfs 0.008 af
Link 200L: POA #200 - South	Inflow=0.10 cfs 0.062 af Primary=0.10 cfs 0.062 af
Link 300L: POA #300 - SE	Inflow=0.04 cfs 0.064 af Primary=0.04 cfs 0.064 af

Total Runoff Area = 30.826 ac Runoff Volume = 0.154 af Average Runoff Depth = 0.06" 87.20% Pervious = 26.879 ac 12.80% Impervious = 3.947 ac



Tc=6.0 min CN=74 Runoff=0.14 cfs 0.010 af

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Runoff Area=66,034 sf 9.68% Impervious Runoff Depth=1.38" Subcatchment 1S: West Corner Flow Length=366' Tc=9.8 min CN=75 Runoff=2.10 cfs 0.174 af Runoff Area=444,574 sf 8.10% Impervious Runoff Depth=1.32" Subcatchment 2.1S: Central Flow Length=1,341' Tc=15.8 min CN=74 Runoff=11.22 cfs 1.120 af Runoff Area=102,088 sf 10.13% Impervious Runoff Depth=1.51" Subcatchment 2.2S: South of SQ Drive Flow Length=391' Tc=6.0 min CN=77 Runoff=4.10 cfs 0.295 af Runoff Area=284,221 sf 6.78% Impervious Runoff Depth=1.19" Subcatchment 2S: North Corner Tc=0.0 min CN=72 Runoff=10.62 cfs 0.649 af Runoff Area=306,834 sf 2.19% Impervious Runoff Depth=0.97" Subcatchment 3S: SE Corner Flow Length=1,149' Tc=15.9 min CN=68 Runoff=5.28 cfs 0.567 af Runoff Area=10,182 sf 98.98% Impervious Runoff Depth>3.36" Subcatchment 10S: Parking Lot Tc=790.0 min CN=98 Runoff=0.06 cfs 0.065 af Runoff Area=588 sf 65.82% Impervious Runoff Depth=2.55" Subcatchment 11S: Island Tc=6.0 min CN=90 Runoff=0.04 cfs 0.003 af Runoff Area=1,878 sf 65.12% Impervious Runoff Depth=2.55" Subcatchment 12S: Building Tc=6.0 min CN=90 Runoff=0.13 cfs 0.009 af Runoff Area=1,349 sf 73.83% Impervious Runoff Depth=2.74" Subcatchment 13S: Building Tc=6.0 min CN=92 Runoff=0.10 cfs 0.007 af Runoff Area=1,021 sf 77.86% Impervious Runoff Depth=2.84" Subcatchment 14S: Building Tc=6.0 min CN=93 Runoff=0.08 cfs 0.006 af Runoff Area=1,376 sf 43.24% Impervious Runoff Depth=2.03" Subcatchment 15S: Building Tc=6.0 min CN=84 Runoff=0.08 cfs 0.005 af Runoff Area=928 sf 26.51% Impervious Runoff Depth=1.72" Subcatchment 16S: Island Tc=6.0 min CN=80 Runoff=0.04 cfs 0.003 af Subcatchment 17S: To Bio #1 Runoff Area=2,893 sf 0.28% Impervious Runoff Depth=1.32" Tc=6.0 min CN=74 Runoff=0.10 cfs 0.007 af Runoff Area=9,168 sf 100.00% Impervious Runoff Depth=3.38" Subcatchment 20S: Building Tc=6.0 min CN=98 Runoff=0.74 cfs 0.059 af Runoff Area=2,610 sf 100.00% Impervious Runoff Depth=3.38" **Subcatchment 21S: Parking Lot** Tc=6.0 min CN=98 Runoff=0.21 cfs 0.017 af Subcatchment 22S: To Bio #2 Runoff Area=3,961 sf 0.00% Impervious Runoff Depth=1.32"

5440-Post

Subcatchment30S: Roadside	Runoff Area=8,230 sf 53.10% Impervious Runoff Depth=2.28" Flow Length=204' Tc=6.0 min CN=87 Runoff=0.50 cfs 0.036 af
Subcatchment31S: Roadside	Runoff Area=7,135 sf 71.11% Impervious Runoff Depth=2.65" Flow Length=143' Tc=6.0 min CN=91 Runoff=0.50 cfs 0.036 af
Subcatchment40S: Building	Runoff Area=3,066 sf 100.00% Impervious Runoff Depth=3.38" Tc=6.0 min CN=98 Runoff=0.25 cfs 0.020 af
Subcatchment41S: Building	Runoff Area=2,125 sf 90.78% Impervious Runoff Depth=3.15" Tc=6.0 min CN=96 Runoff=0.17 cfs 0.013 af
Subcatchment42S: Building	Runoff Area=3,298 sf 74.41% Impervious Runoff Depth=2.74" Tc=6.0 min CN=92 Runoff=0.24 cfs 0.017 af
Subcatchment44S: Roadside	Runoff Area=4,663 sf 40.92% Impervious Runoff Depth=2.03" Tc=6.0 min CN=84 Runoff=0.26 cfs 0.018 af
Subcatchment45S: Parking Lot	Runoff Area=3,860 sf 71.71% Impervious Runoff Depth=2.65" Tc=6.0 min CN=91 Runoff=0.27 cfs 0.020 af
Subcatchment46S: Parking Lot	Runoff Area=4,489 sf 73.25% Impervious Runoff Depth=2.74" Tc=6.0 min CN=92 Runoff=0.32 cfs 0.024 af
Subcatchment48S: Roadside	Runoff Area=3,415 sf 55.67% Impervious Runoff Depth=2.37" Tc=6.0 min CN=88 Runoff=0.22 cfs 0.015 af
Subcatchment49S: Driveway	Runoff Area=3,657 sf 83.78% Impervious Runoff Depth=2.84" Tc=6.0 min CN=93 Runoff=0.27 cfs 0.020 af
Subcatchment 60S: Parking Lot	Runoff Area=9,169 sf 78.55% Impervious Runoff Depth=2.84" Flow Length=116' Tc=6.0 min CN=93 Runoff=0.67 cfs 0.050 af
Subcatchment 61S: Parking Lot	Runoff Area=7,068 sf 91.98% Impervious Runoff Depth=3.15" Flow Length=111' Tc=6.0 min CN=96 Runoff=0.55 cfs 0.043 af
Subcatchment 63S: Building	Runoff Area=1,459 sf 65.52% Impervious Runoff Depth=2.55" Tc=6.0 min CN=90 Runoff=0.10 cfs 0.007 af
Subcatchment 64S: Building	Runoff Area=1,507 sf 81.22% Impervious Runoff Depth=2.84" Tc=6.0 min CN=93 Runoff=0.11 cfs 0.008 af
Subcatchment65S: Building	Runoff Area=982 sf 80.86% Impervious Runoff Depth=2.84" Tc=6.0 min CN=93 Runoff=0.07 cfs 0.005 af
Subcatchment66S: Building	Runoff Area=1,478 sf 44.79% Impervious Runoff Depth=1.87" Tc=6.0 min CN=82 Runoff=0.07 cfs 0.005 af
Subcatchment 67S: Landscape	Runoff Area=1,430 sf 23.15% Impervious Runoff Depth=1.08" Tc=6.0 min CN=70 Runoff=0.04 cfs 0.003 af
Subcatchment 68S: Building	Runoff Area=1,370 sf 100.00% Impervious Runoff Depth=3.38" Tc=6.0 min CN=98 Runoff=0.11 cfs 0.009 af

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Runoff Area=13,261 sf 1.69% Impervious Runoff Depth=0.97" Subcatchment 69S: To Pond

Tc=6.0 min CN=68 Runoff=0.31 cfs 0.025 af

Subcatchment 70S: Building Runoff Area=6,906 sf 100.00% Impervious Runoff Depth=3.38"

Tc=6.0 min CN=98 Runoff=0.56 cfs 0.045 af

Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=3.38" Subcatchment 80S: Building

Tc=6.0 min CN=98 Runoff=0.10 cfs 0.008 af

Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=3.38" Subcatchment81S: Building

Tc=6.0 min CN=98 Runoff=0.10 cfs 0.008 af

Runoff Area=9,597 sf 90.39% Impervious Runoff Depth=3.05" Subcatchment 82S: Parking Lot

Flow Length=214' Tc=6.0 min CN=95 Runoff=0.74 cfs 0.056 af

Runoff Area=2,507 sf 0.00% Impervious Runoff Depth=1.32" Subcatchment 83S: To Pond

Tc=6.0 min CN=74 Runoff=0.09 cfs 0.006 af

Avg. Flow Depth=0.45' Max Vel=1.48 fps Inflow=2.22 cfs 0.649 af Reach 2.1R: Wetland Flow Path

n=0.040 L=344.0' S=0.0075 '/' Capacity=54.59 cfs Outflow=2.22 cfs 0.649 af

Avg. Flow Depth=0.65' Max Vel=4.66 fps Inflow=11.91 cfs 1.925 af Reach 2.2R: Wetland Flow Path

n=0.040 L=263.0' S=0.0502 '/' Capacity=141.18 cfs Outflow=11.91 cfs 1.925 af

Avg. Flow Depth=0.28' Max Vel=2.78 fps Inflow=2.23 cfs 0.649 af Reach 2R: Wetland Flow Path

n=0.040 L=151.0' S=0.0446'/' Capacity=133.09 cfs Outflow=2.22 cfs 0.649 af

Avg. Flow Depth=0.05' Max Vel=0.38 fps Inflow=0.07 cfs 0.040 af Reach 10R: Swale

n=0.035 L=44.0' S=0.0050 '/' Capacity=16.22 cfs Outflow=0.07 cfs 0.040 af

Reach 22R: Wetland Flow Path Avg. Flow Depth=0.04' Max Vel=0.67 fps Inflow=0.29 cfs 0.085 af

n=0.040 L=211.0' S=0.0245'/' Capacity=392.58 cfs Outflow=0.29 cfs 0.085 af

Avg. Flow Depth=0.38' Max Vel=2.11 fps Inflow=2.50 cfs 0.734 af Reach 23R: Wetland Flow Path

n=0.040 L=131.0' S=0.0185 '/' Capacity=85.86 cfs Outflow=2.50 cfs 0.734 af

Avg. Flow Depth=0.23' Max Vel=0.88 fps Inflow=0.95 cfs 0.324 af Reach 69R: Swale

n=0.069 L=17.0' S=0.0147'/' Capacity=14.11 cfs Outflow=0.95 cfs 0.324 af

Avg. Flow Depth=0.19' Max Vel=0.42 fps Inflow=0.95 cfs 0.324 af Reach 70R: Woodland Flow Path

n=0.400 L=57.0' S=0.1404 '/' Capacity=21.20 cfs Outflow=0.95 cfs 0.324 af

Avg. Flow Depth=0.16' Max Vel=2.30 fps Inflow=0.95 cfs 0.324 af Reach 71R: Stream

n=0.040 L=80.0' S=0.0554 '/' Capacity=31.12 cfs Outflow=0.95 cfs 0.324 af

Reach 72R: Stream Avg. Flow Depth=0.16' Max Vel=1.30 fps Inflow=0.95 cfs 0.324 af

n=0.040 L=88.0' S=0.0160 '/' Capacity=25.40 cfs Outflow=0.95 cfs 0.324 af

Avg. Flow Depth=0.19' Max Vel=1.43 fps Inflow=1.27 cfs 0.401 af Reach 73R: Stream

n=0.040 L=309.0' S=0.0160 '/' Capacity=25.38 cfs Outflow=1.26 cfs 0.401 af

Reach 83R: Woodland Flow	Path Avg. Flow Depth=0.11' Max Vel=0.25 fps Inflow=0.33 cfs 0.077 af n=0.400 L=122.0' S=0.0950 '/' Capacity=17.44 cfs Outflow=0.32 cfs 0.077 af
Pond 1P: 15" RCP	Peak Elev=53.11' Storage=73 cf Inflow=2.12 cfs 0.214 af 15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=2.05 cfs 0.214 af
Pond 2.1P: 24" RCP	Peak Elev=49.51' Storage=3,293 cf Inflow=13.44 cfs 1.853 af 24.0" Round Culvert n=0.012 L=17.0' S=0.0135 '/' Outflow=11.55 cfs 1.853 af
Pond 2P: 12" CPP	Peak Elev=60.71' Storage=8,446 cf Inflow=10.62 cfs 0.649 af Outflow=2.23 cfs 0.649 af
Pond 10P: Porous Pavement Disca	t Peak Elev=56.57' Storage=573 cf Inflow=0.06 cfs 0.065 af arded=0.03 cfs 0.065 af Secondary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.065 af
Pond 11P: YD #3-1	Peak Elev=57.49' Inflow=0.04 cfs 0.003 af 6.0" Round Culvert n=0.012 L=36.0' S=0.0050 '/' Outflow=0.04 cfs 0.003 af
Pond 12P: YD #7	Peak Elev=57.99' Inflow=0.13 cfs 0.009 af 8.0" Round Culvert n=0.012 L=52.0' S=0.0050 '/' Outflow=0.13 cfs 0.009 af
Pond 13P: YD #6	Peak Elev=57.62' Inflow=0.22 cfs 0.016 af 10.0" Round Culvert n=0.012 L=29.0' S=0.0052 '/' Outflow=0.22 cfs 0.016 af
Pond 14P: YD #5	Peak Elev=57.49' Inflow=0.30 cfs 0.022 af 10.0" Round Culvert n=0.012 L=20.0' S=0.0050 '/' Outflow=0.30 cfs 0.022 af
Pond 15P: YD #4	Peak Elev=57.49' Inflow=0.37 cfs 0.027 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.37 cfs 0.027 af
Pond 16P: CB #3	Peak Elev=57.49' Inflow=0.46 cfs 0.033 af 12.0" Round Culvert n=0.012 L=45.0' S=0.0049 '/' Outflow=0.46 cfs 0.033 af
Pond 17P: Bio #1 / OS #1	Peak Elev=57.49' Storage=933 cf Inflow=0.56 cfs 0.040 af Outflow=0.07 cfs 0.040 af
Pond 21P: CB #8	Peak Elev=59.00' Inflow=0.21 cfs 0.017 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0050 '/' Outflow=0.21 cfs 0.017 af
Pond 22P: Bio #2 / OS #9	Peak Elev=59.00' Storage=1,828 cf Inflow=1.08 cfs 0.086 af Outflow=0.29 cfs 0.085 af
Pond 30P: CB #10-2	Peak Elev=53.00' Inflow=0.50 cfs 0.036 af 12.0" Round Culvert n=0.012 L=78.0' S=0.0397 '/' Outflow=0.50 cfs 0.036 af
Pond 31P: CB #10-1	Peak Elev=50.00' Inflow=0.50 cfs 0.036 af 12.0" Round Culvert n=0.012 L=2.0' S=0.0250 '/' Outflow=0.50 cfs 0.036 af
Pond 32P: DMH #10	Peak Elev=48.92' Inflow=11.91 cfs 1.925 af 24.0" Round Culvert n=0.012 L=57.0' S=0.0135 '/' Outflow=11.91 cfs 1.925 af
Pond 40P: 6" CPP	Peak Elev=53.89' Inflow=0.25 cfs 0.020 af 6.0" Round Culvert n=0.012 L=174.0' S=0.0150 '/' Outflow=0.25 cfs 0.020 af

Pond 41P: YD #15	Peak Elev=51.17' Inflow=0.41 cfs 0.033 af 8.0" Round Culvert n=0.012 L=47.0' S=0.0149 '/' Outflow=0.41 cfs 0.033 af
Pond 42P: YD #14-1	Peak Elev=54.26' Inflow=0.24 cfs 0.017 af 6.0" Round Culvert n=0.012 L=158.0' S=0.0175 '/' Outflow=0.24 cfs 0.017 af
Pond 43P: 10" CPP	Peak Elev=50.86' Inflow=0.65 cfs 0.050 af 8.0" Round Culvert n=0.012 L=33.0' S=0.0148 '/' Outflow=0.65 cfs 0.050 af
Pond 44P: CB #14	Peak Elev=50.86' Inflow=0.90 cfs 0.068 af 12.0" Round Culvert n=0.012 L=31.0' S=0.0048'/ Outflow=0.90 cfs 0.068 af
Pond 45P: CB #13-2	Peak Elev=50.85' Inflow=0.27 cfs 0.020 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0164 '/' Outflow=0.27 cfs 0.019 af
Pond 46P: CB #13-1	Peak Elev=50.85' Inflow=0.32 cfs 0.024 af 12.0" Round Culvert n=0.012 L=12.0' S=0.0050 '/' Outflow=0.32 cfs 0.024 af
Pond 47P: DMH #13	Peak Elev=50.85' Inflow=1.49 cfs 0.111 af 15.0" Round Culvert n=0.012 L=67.0' S=0.0040'/' Outflow=1.49 cfs 0.111 af
Pond 48P: CB #12-2	Peak Elev=50.85' Inflow=0.22 cfs 0.015 af 12.0" Round Culvert n=0.012 L=6.0' S=0.0050 '/' Outflow=0.22 cfs 0.015 af
Pond 49P: CB #12-1	Peak Elev=50.85' Inflow=0.27 cfs 0.020 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.27 cfs 0.020 af
Pond 50P: DMH #12	Peak Elev=50.85' Inflow=1.98 cfs 0.146 af 15.0" Round Culvert n=0.012 L=103.0' S=0.0040 '/' Outflow=1.98 cfs 0.146 af
Pond 60P: CB #17-1-2	Peak Elev=51.11' Inflow=0.67 cfs 0.050 af 12.0" Round Culvert n=0.012 L=21.0' S=0.0052'/' Outflow=0.67 cfs 0.050 af
Pond 61P: CB #17-1-1	Peak Elev=51.08' Inflow=0.55 cfs 0.043 af 12.0" Round Culvert n=0.012 L=3.0' S=0.0367 '/' Outflow=0.55 cfs 0.043 af
Pond 62P: DMH #17-1	Peak Elev=51.04' Inflow=1.23 cfs 0.092 af 12.0" Round Culvert n=0.012 L=25.0' S=0.0052'/' Outflow=1.23 cfs 0.092 af
Pond 63P: YD #21	Peak Elev=51.88' Inflow=0.10 cfs 0.007 af 12.0" Round Culvert n=0.012 L=49.0' S=0.0049 '/' Outflow=0.10 cfs 0.007 af
Pond 64P: YD #20	Peak Elev=51.56' Inflow=0.21 cfs 0.015 af 10.0" Round Culvert n=0.012 L=35.0' S=0.0049 '/' Outflow=0.21 cfs 0.015 af
Pond 65P: YD #19	Peak Elev=51.32' Inflow=0.28 cfs 0.021 af 10.0" Round Culvert n=0.012 L=42.0' S=0.0050 '/' Outflow=0.28 cfs 0.021 af
Pond 66P: YD #18	Peak Elev=51.00' Inflow=0.36 cfs 0.026 af 12.0" Round Culvert n=0.012 L=11.0' S=0.0509 '/' Outflow=0.36 cfs 0.026 af

544	ΛI	\Box	cŧ
244	U - I	20	SI

Type III 24-hr 2-year Rainfall=3.61"

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Printed 10/6/2025

Pond 67P: CB #17	Peak Elev=50.89' Inflows	=1.62 cfs 0.121 af
	12.0" Round Culvert n=0.012 L=74.0' S=0.0050 '/' Outflow=	=1.62 cfs 0.121 af

Pond 68P: DMH #16 Peak Elev=50.86' Inflow=1.73 cfs 0.130 af 12.0" Round Culvert n=0.012 L=90.0' S=0.0110 '/' Outflow=1.73 cfs 0.130 af

Pond 69P: Bio #3 / OS #11 Peak Elev=50.85' Storage=7,761 cf Inflow=4.58 cfs 0.346 af

Outflow=0.95 cfs 0.324 af

Pond 80P: 6" CPP Peak Elev=49.58' Inflow=0.10 cfs 0.008 af

6.0" Round Culvert n=0.012 L=111.0' S=0.0350 '/' Outflow=0.10 cfs 0.008 af

Pond 81P: 6" CPP Peak Elev=49.52' Inflow=0.10 cfs 0.008 af

6.0" Round Culvert n=0.012 L=85.0' S=0.0351 '/' Outflow=0.10 cfs 0.008 af

Pond 82P: CB #23 Peak Elev=47.69' Inflow=0.84 cfs 0.064 af

12.0" Round Culvert n=0.012 L=18.0' S=0.0194 '/' Outflow=0.84 cfs 0.064 af

Pond 83P: Bio #4 / OS #22 Peak Elev=47.68' Storage=1,472 cf Inflow=1.02 cfs 0.078 af

Outflow=0.33 cfs 0.077 af

Link 100L: POA #100 - SW Inflow=2.05 cfs 0.214 af

Primary=2.05 cfs 0.214 af

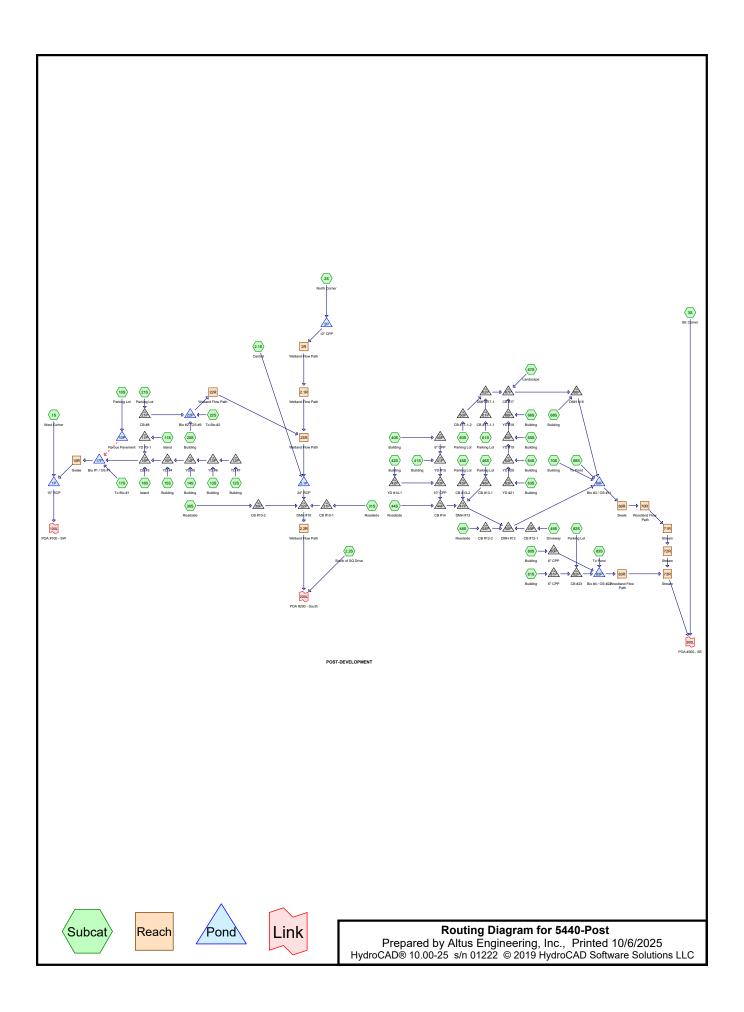
Link 200L: POA #200 - South Inflow=13.65 cfs 2.221 af

Primary=13.65 cfs 2.221 af

Link 300L: POA #300 - SEInflow=5.84 cfs 0.968 af

Primary=5.84 cfs 0.968 af

Total Runoff Area = 30.826 ac Runoff Volume = 3.493 af Average Runoff Depth = 1.36" 87.20% Pervious = 26.879 ac 12.80% Impervious = 3.947 ac



Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.269	61	>75% Grass cover, Good, HSG B (2.1S, 3S, 48S, 49S, 61S, 66S, 67S, 69S, 82S)
4.037	74	>75% Grass cover, Good, HSG C (1S, 2.1S, 2.2S, 2S, 3S, 10S, 11S, 12S, 13S,
		14S, 15S, 16S, 17S, 22S, 30S, 31S, 41S, 42S, 44S, 45S, 46S, 48S, 49S, 60S, 61S,
		63S, 64S, 65S, 66S, 69S, 82S, 83S)
0.107	80	>75% Grass cover, Good, HSG D (2.1S, 2.2S, 2S, 48S, 49S)
0.195	65	Brush, Good, HSG C (2.1S)
0.126	98	Paved parking, HSG B (2.1S, 3S, 49S, 61S, 67S, 69S, 82S)
2.508	98	Paved parking, HSG C (1S, 2.1S, 2.2S, 2S, 3S, 10S, 11S, 12S, 15S, 16S, 17S, 21S,
		30S, 31S, 42S, 44S, 45S, 46S, 48S, 49S, 60S, 61S, 66S, 82S)
0.241	98	Paved parking, HSG D (2.1S, 2.2S, 2S, 3S, 48S, 49S)
0.035	98	Roofs, HSG B (66S, 68S, 70S)
1.037	98	Roofs, HSG C (2.1S, 2S, 11S, 12S, 13S, 14S, 15S, 20S, 21S, 40S, 41S, 42S, 48S,
		63S, 64S, 65S, 66S, 68S, 70S, 80S, 81S)
1.880	55	Woods, Good, HSG B (1S, 2.1S, 2S, 3S)
17.117	70	Woods, Good, HSG C (1S, 2.1S, 2.2S, 2S, 3S, 60S)
3.274	77	Woods, Good, HSG D (2.1S, 2.2S, 2S)
30.826	74	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
2.310	HSG B	1S, 2.1S, 2S, 3S, 48S, 49S, 61S, 66S, 67S, 68S, 69S, 70S, 82S
24.894	HSG C	1S, 2.1S, 2.2S, 2S, 3S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 20S, 21S, 22S,
		30S, 31S, 40S, 41S, 42S, 44S, 45S, 46S, 48S, 49S, 60S, 61S, 63S, 64S, 65S,
		66S, 68S, 69S, 70S, 80S, 81S, 82S, 83S
3.622	HSG D	2.1S, 2.2S, 2S, 3S, 48S, 49S
0.000	Other	
30.826		TOTAL AREA

Runoff Area=3,961 sf 0.00% Impervious Runoff Depth=2.74"

Tc=6.0 min CN=74 Runoff=0.29 cfs 0.021 af

Subcatchment 22S: To Bio #2

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Runoff Area=66,034 sf 9.68% Impervious Runoff Depth=2.84" Subcatchment 1S: West Corner Flow Length=366' Tc=9.8 min CN=75 Runoff=4.43 cfs 0.358 af Runoff Area=444,574 sf 8.10% Impervious Runoff Depth=2.74" Subcatchment 2.1S: Central Flow Length=1,341' Tc=15.8 min CN=74 Runoff=24.25 cfs 2.334 af Runoff Area=102,088 sf 10.13% Impervious Runoff Depth=3.02" Subcatchment 2.2S: South of SQ Drive Flow Length=391' Tc=6.0 min CN=77 Runoff=8.31 cfs 0.590 af Runoff Area=284,221 sf 6.78% Impervious Runoff Depth=2.57" Subcatchment 2S: North Corner Tc=0.0 min CN=72 Runoff=23.92 cfs 1.395 af Runoff Area=306,834 sf 2.19% Impervious Runoff Depth=2.22" Subcatchment 3S: SE Corner Flow Length=1,149' Tc=15.9 min CN=68 Runoff=13.24 cfs 1.304 af Runoff Area=10,182 sf 98.98% Impervious Runoff Depth>5.20" Subcatchment 10S: Parking Lot Tc=790.0 min CN=98 Runoff=0.09 cfs 0.101 af Runoff Area=588 sf 65.82% Impervious Runoff Depth=4.33" Subcatchment 11S: Island Tc=6.0 min CN=90 Runoff=0.07 cfs 0.005 af Runoff Area=1,878 sf 65.12% Impervious Runoff Depth=4.33" Subcatchment 12S: Building Tc=6.0 min CN=90 Runoff=0.21 cfs 0.016 af Runoff Area=1,349 sf 73.83% Impervious Runoff Depth=4.55" Subcatchment 13S: Building Tc=6.0 min CN=92 Runoff=0.16 cfs 0.012 af Runoff Area=1,021 sf 77.86% Impervious Runoff Depth=4.66" Subcatchment 14S: Building Tc=6.0 min CN=93 Runoff=0.12 cfs 0.009 af Runoff Area=1,376 sf 43.24% Impervious Runoff Depth=3.70" Subcatchment 15S: Building Tc=6.0 min CN=84 Runoff=0.14 cfs 0.010 af Runoff Area=928 sf 26.51% Impervious Runoff Depth=3.31" Subcatchment 16S: Island Tc=6.0 min CN=80 Runoff=0.08 cfs 0.006 af Subcatchment 17S: To Bio #1 Runoff Area=2,893 sf 0.28% Impervious Runoff Depth=2.74" Tc=6.0 min CN=74 Runoff=0.21 cfs 0.015 af Runoff Area=9,168 sf 100.00% Impervious Runoff Depth=5.23" Subcatchment 20S: Building Tc=6.0 min CN=98 Runoff=1.13 cfs 0.092 af Runoff Area=2,610 sf 100.00% Impervious Runoff Depth=5.23" **Subcatchment 21S: Parking Lot** Tc=6.0 min CN=98 Runoff=0.32 cfs 0.026 af

5440-Post

Subcatchment30S: Roadside	Runoff Area=8,230 sf 53.10% Impervious Runoff Depth=4.01" Flow Length=204' Tc=6.0 min CN=87 Runoff=0.87 cfs 0.063 af
Subcatchment31S: Roadside	Runoff Area=7,135 sf 71.11% Impervious Runoff Depth=4.44" Flow Length=143' Tc=6.0 min CN=91 Runoff=0.81 cfs 0.061 af
Subcatchment40S: Building	Runoff Area=3,066 sf 100.00% Impervious Runoff Depth=5.23" Tc=6.0 min CN=98 Runoff=0.38 cfs 0.031 af
Subcatchment41S: Building	Runoff Area=2,125 sf 90.78% Impervious Runoff Depth=5.00" Tc=6.0 min CN=96 Runoff=0.26 cfs 0.020 af
Subcatchment42S: Building	Runoff Area=3,298 sf 74.41% Impervious Runoff Depth=4.55" Tc=6.0 min CN=92 Runoff=0.38 cfs 0.029 af
Subcatchment44S: Roadside	Runoff Area=4,663 sf 40.92% Impervious Runoff Depth=3.70" Tc=6.0 min CN=84 Runoff=0.46 cfs 0.033 af
Subcatchment45S: Parking Lot	Runoff Area=3,860 sf 71.71% Impervious Runoff Depth=4.44" Tc=6.0 min CN=91 Runoff=0.44 cfs 0.033 af
Subcatchment46S: Parking Lot	Runoff Area=4,489 sf 73.25% Impervious Runoff Depth=4.55" Tc=6.0 min CN=92 Runoff=0.52 cfs 0.039 af
Subcatchment48S: Roadside	Runoff Area=3,415 sf 55.67% Impervious Runoff Depth=4.12" Tc=6.0 min CN=88 Runoff=0.37 cfs 0.027 af
Subcatchment49S: Driveway	Runoff Area=3,657 sf 83.78% Impervious Runoff Depth=4.66" Tc=6.0 min CN=93 Runoff=0.43 cfs 0.033 af
Subcatchment 60S: Parking Lot	Runoff Area=9,169 sf 78.55% Impervious Runoff Depth=4.66" Flow Length=116' Tc=6.0 min CN=93 Runoff=1.07 cfs 0.082 af
Subcatchment 61S: Parking Lot	Runoff Area=7,068 sf 91.98% Impervious Runoff Depth=5.00" Flow Length=111' Tc=6.0 min CN=96 Runoff=0.86 cfs 0.068 af
Subcatchment 63S: Building	Runoff Area=1,459 sf 65.52% Impervious Runoff Depth=4.33" Tc=6.0 min CN=90 Runoff=0.16 cfs 0.012 af
Subcatchment 64S: Building	Runoff Area=1,507 sf 81.22% Impervious Runoff Depth=4.66" Tc=6.0 min CN=93 Runoff=0.18 cfs 0.013 af
Subcatchment65S: Building	Runoff Area=982 sf 80.86% Impervious Runoff Depth=4.66" Tc=6.0 min CN=93 Runoff=0.12 cfs 0.009 af
Subcatchment66S: Building	Runoff Area=1,478 sf 44.79% Impervious Runoff Depth=3.50" Tc=6.0 min CN=82 Runoff=0.14 cfs 0.010 af
Subcatchment 67S: Landscape	Runoff Area=1,430 sf 23.15% Impervious Runoff Depth=2.39" Tc=6.0 min CN=70 Runoff=0.09 cfs 0.007 af
Subcatchment68S: Building	Runoff Area=1,370 sf 100.00% Impervious Runoff Depth=5.23" Tc=6.0 min CN=98 Runoff=0.17 cfs 0.014 af

Runoff Area=13,261 sf 1.69% Impervious Runoff Depth=2.22" Subcatchment 69S: To Pond

Tc=6.0 min CN=68 Runoff=0.78 cfs 0.056 af

Subcatchment 70S: Building Runoff Area=6,906 sf 100.00% Impervious Runoff Depth=5.23"

Tc=6.0 min CN=98 Runoff=0.85 cfs 0.069 af

Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=5.23" Subcatchment 80S: Building

Tc=6.0 min CN=98 Runoff=0.15 cfs 0.012 af

Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=5.23" Subcatchment81S: Building

Tc=6.0 min CN=98 Runoff=0.15 cfs 0.012 af

Runoff Area=9,597 sf 90.39% Impervious Runoff Depth=4.89" Subcatchment 82S: Parking Lot

Flow Length=214' Tc=6.0 min CN=95 Runoff=1.15 cfs 0.090 af

Runoff Area=2,507 sf 0.00% Impervious Runoff Depth=2.74" Subcatchment 83S: To Pond

Tc=6.0 min CN=74 Runoff=0.18 cfs 0.013 af

Avg. Flow Depth=1.19' Max Vel=2.53 fps Inflow=18.63 cfs 1.395 af Reach 2.1R: Wetland Flow Path n=0.040 L=344.0' S=0.0075 '/' Capacity=54.59 cfs Outflow=16.73 cfs 1.395 af

Avg. Flow Depth=0.81' Max Vel=5.28 fps Inflow=19.28 cfs 3.990 af Reach 2.2R: Wetland Flow Path

n=0.040 L=263.0' S=0.0502'/' Capacity=141.18 cfs Outflow=19.04 cfs 3.990 af

Avg. Flow Depth=0.83' Max Vel=5.02 fps Inflow=18.70 cfs 1.395 af Reach 2R: Wetland Flow Path

n=0.040 L=151.0' S=0.0446 '/' Capacity=133.09 cfs Outflow=18.63 cfs 1.395 af

Avg. Flow Depth=0.11' Max Vel=0.64 fps Inflow=0.30 cfs 0.072 af Reach 10R: Swale

n=0.035 L=44.0' S=0.0050 '/' Capacity=16.22 cfs Outflow=0.30 cfs 0.072 af

Avg. Flow Depth=0.05' Max Vel=0.80 fps Inflow=0.44 cfs 0.137 af Reach 22R: Wetland Flow Path

n=0.040 L=211.0' S=0.0245'/' Capacity=392.58 cfs Outflow=0.44 cfs 0.137 af

Avg. Flow Depth=0.97' Max Vel=3.54 fps Inflow=17.02 cfs 1.532 af Reach 23R: Wetland Flow Path

n=0.040 L=131.0' S=0.0185'/' Capacity=85.86 cfs Outflow=16.91 cfs 1.532 af

Avg. Flow Depth=0.40' Max Vel=1.22 fps Inflow=2.52 cfs 0.560 af Reach 69R: Swale

n=0.069 L=17.0' S=0.0147'/' Capacity=14.11 cfs Outflow=2.52 cfs 0.560 af

Avg. Flow Depth=0.33' Max Vel=0.57 fps Inflow=2.52 cfs 0.560 af Reach 70R: Woodland Flow Path

n=0.400 L=57.0' S=0.1404 '/' Capacity=21.20 cfs Outflow=2.52 cfs 0.560 af

Reach 71R: Stream Avg. Flow Depth=0.28' Max Vel=3.12 fps Inflow=2.52 cfs 0.560 af

n=0.040 L=80.0' S=0.0554 '/' Capacity=31.12 cfs Outflow=2.52 cfs 0.560 af

Reach 72R: Stream Avg. Flow Depth=0.29' Max Vel=1.81 fps Inflow=2.52 cfs 0.560 af

n=0.040 L=88.0' S=0.0160 '/' Capacity=25.40 cfs Outflow=2.52 cfs 0.559 af

Avg. Flow Depth=0.32' Max Vel=1.92 fps Inflow=3.01 cfs 0.686 af Reach 73R: Stream

n=0.040 L=309.0' S=0.0160 '/' Capacity=25.38 cfs Outflow=3.00 cfs 0.685 af

Reach 83R: Woodland Flow	Path Avg. Flow Depth=0.15' Max Vel=0.29 fps Inflow=0.50 cfs 0.127 af n=0.400 L=122.0' S=0.0950 '/' Capacity=17.44 cfs Outflow=0.49 cfs 0.126 af
Pond 1P: 15" RCP	Peak Elev=53.53' Storage=682 cf Inflow=4.64 cfs 0.430 af 15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=3.78 cfs 0.430 af
Pond 2.1P: 24" RCP	Peak Elev=51.40' Storage=24,417 cf Inflow=35.88 cfs 3.867 af 24.0" Round Culvert n=0.012 L=17.0' S=0.0135 '/' Outflow=18.94 cfs 3.867 af
Pond 2P: 12" CPP	Peak Elev=60.93' Storage=11,719 cf Inflow=23.92 cfs 1.395 af Outflow=18.70 cfs 1.395 af
Pond 10P: Porous Pavement Disca	Peak Elev=57.53' Storage=1,632 cf Inflow=0.09 cfs 0.101 af arded=0.04 cfs 0.097 af Secondary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.097 af
Pond 11P: YD #3-1	Peak Elev=57.89' Inflow=0.07 cfs 0.005 af 6.0" Round Culvert n=0.012 L=36.0' S=0.0050 '/' Outflow=0.07 cfs 0.005 af
Pond 12P: YD #7	Peak Elev=58.07' Inflow=0.21 cfs 0.016 af 8.0" Round Culvert n=0.012 L=52.0' S=0.0050 '/' Outflow=0.21 cfs 0.016 af
Pond 13P: YD #6	Peak Elev=57.90' Inflow=0.37 cfs 0.027 af 10.0" Round Culvert n=0.012 L=29.0' S=0.0052 '/' Outflow=0.37 cfs 0.027 af
Pond 14P: YD #5	Peak Elev=57.90' Inflow=0.49 cfs 0.036 af 10.0" Round Culvert n=0.012 L=20.0' S=0.0050 '/' Outflow=0.49 cfs 0.036 af
Pond 15P: YD #4	Peak Elev=57.89' Inflow=0.62 cfs 0.046 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.62 cfs 0.046 af
Pond 16P: CB #3	Peak Elev=57.89' Inflow=0.77 cfs 0.057 af 12.0" Round Culvert n=0.012 L=45.0' S=0.0049 '/' Outflow=0.77 cfs 0.057 af
Pond 17P: Bio #1 / OS #1	Peak Elev=57.89' Storage=1,367 cf Inflow=0.98 cfs 0.072 af Outflow=0.30 cfs 0.072 af
Pond 21P: CB #8	Peak Elev=59.46' Inflow=0.32 cfs 0.026 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0050 '/' Outflow=0.32 cfs 0.026 af
Pond 22P: Bio #2 / OS #9	Peak Elev=59.46' Storage=2,731 cf Inflow=1.74 cfs 0.139 af Outflow=0.44 cfs 0.137 af
Pond 30P: CB #10-2	Peak Elev=53.13' Inflow=0.87 cfs 0.063 af 12.0" Round Culvert n=0.012 L=78.0' S=0.0397 '/' Outflow=0.87 cfs 0.063 af
Pond 31P: CB #10-1	Peak Elev=50.14' Inflow=0.81 cfs 0.061 af 12.0" Round Culvert n=0.012 L=2.0' S=0.0250 '/' Outflow=0.81 cfs 0.061 af
Pond 32P: DMH #10	Peak Elev=49.91' Inflow=19.28 cfs 3.990 af 24.0" Round Culvert n=0.012 L=57.0' S=0.0135 '/' Outflow=19.28 cfs 3.990 af
Pond 40P: 6" CPP	Peak Elev=53.99' Inflow=0.38 cfs 0.031 af 6.0" Round Culvert n=0.012 L=174.0' S=0.0150 '/' Outflow=0.38 cfs 0.031 af

Pond 41P: YD #15	Peak Elev=52.30' Inflow=0.63 cfs 0.051 af 8.0" Round Culvert n=0.012 L=47.0' S=0.0149 '/' Outflow=0.63 cfs 0.051 af
Pond 42P: YD #14-1	Peak Elev=54.36' Inflow=0.38 cfs 0.029 af 6.0" Round Culvert n=0.012 L=158.0' S=0.0175 '/' Outflow=0.38 cfs 0.029 af
Pond 43P: 10" CPP	Peak Elev=52.16' Inflow=1.02 cfs 0.080 af 8.0" Round Culvert n=0.012 L=33.0' S=0.0148 '/' Outflow=1.02 cfs 0.080 af
Pond 44P: CB #14	Peak Elev=51.85' Inflow=1.48 cfs 0.113 af 12.0" Round Culvert n=0.012 L=31.0' S=0.0048 '/' Outflow=1.48 cfs 0.113 af
Pond 45P: CB #13-2	Peak Elev=51.76' Inflow=0.44 cfs 0.033 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0164 '/' Outflow=0.44 cfs 0.033 af
Pond 46P: CB #13-1	Peak Elev=51.76' Inflow=0.52 cfs 0.039 af 12.0" Round Culvert n=0.012 L=12.0' S=0.0050 '/' Outflow=0.52 cfs 0.039 af
Pond 47P: DMH #13	Peak Elev=51.76' Inflow=2.43 cfs 0.185 af 15.0" Round Culvert n=0.012 L=67.0' S=0.0040 '/' Outflow=2.43 cfs 0.185 af
Pond 48P: CB #12-2	Peak Elev=51.74' Inflow=0.37 cfs 0.027 af 12.0" Round Culvert n=0.012 L=6.0' S=0.0050 '/' Outflow=0.37 cfs 0.027 af
Pond 49P: CB #12-1	Peak Elev=51.74' Inflow=0.43 cfs 0.033 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.43 cfs 0.033 af
Pond 50P: DMH #12	Peak Elev=51.73' Inflow=3.23 cfs 0.244 af 15.0" Round Culvert n=0.012 L=103.0' S=0.0040 '/' Outflow=3.23 cfs 0.244 af
Pond 60P: CB #17-1-2	Peak Elev=52.78' Inflow=1.07 cfs 0.082 af 12.0" Round Culvert n=0.012 L=21.0' S=0.0052 '/' Outflow=1.07 cfs 0.082 af
Pond 61P: CB #17-1-1	Peak Elev=52.76' Inflow=0.86 cfs 0.068 af 12.0" Round Culvert n=0.012 L=3.0' S=0.0367 '/' Outflow=0.86 cfs 0.068 af
Pond 62P: DMH #17-1	Peak Elev=52.71' Inflow=1.93 cfs 0.149 af 12.0" Round Culvert n=0.012 L=25.0' S=0.0052'/' Outflow=1.93 cfs 0.149 af
Pond 63P: YD #21	Peak Elev=52.54' Inflow=0.16 cfs 0.012 af 12.0" Round Culvert n=0.012 L=49.0' S=0.0049 '/' Outflow=0.16 cfs 0.012 af
Pond 64P: YD #20	Peak Elev=52.54' Inflow=0.34 cfs 0.026 af 10.0" Round Culvert n=0.012 L=35.0' S=0.0049 '/' Outflow=0.34 cfs 0.026 af
Pond 65P: YD #19	Peak Elev=52.53' Inflow=0.45 cfs 0.034 af 10.0" Round Culvert n=0.012 L=42.0' S=0.0050 '/' Outflow=0.45 cfs 0.034 af
Pond 66P: YD #18	Peak Elev=52.50' Inflow=0.59 cfs 0.044 af 12.0" Round Culvert n=0.012 L=11.0' S=0.0509 '/' Outflow=0.59 cfs 0.044 af

544	Λ	D.	4
544	w-	r)ST

Type III 24-hr 10-year Rainfall=5.47"

Prepared by Altus Engineering, Inc.

Printed 10/6/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Pond 67P: CB #17	Peak Elev=52.48'	Inflow=2.62 cfs 0.200 af
------------------	------------------	--------------------------

12.0" Round Culvert n=0.012 L=74.0' S=0.0050 '/' Outflow=2.62 cfs 0.200 af

Pond 68P: DMH #16 Peak Elev=51.94' Inflow=2.78 cfs 0.214 af

12.0" Round Culvert n=0.012 L=90.0' S=0.0110 '/' Outflow=2.78 cfs 0.214 af

Pond 69P: Bio #3 / OS #11 Peak Elev=51.69' Storage=11,307 cf Inflow=7.64 cfs 0.583 af

Outflow=2.52 cfs 0.560 af

Pond 80P: 6" CPP Peak Elev=49.62' Inflow=0.15 cfs 0.012 af

6.0" Round Culvert n=0.012 L=111.0' S=0.0350 '/' Outflow=0.15 cfs 0.012 af

Pond 81P: 6" CPP Peak Elev=49.56' Inflow=0.15 cfs 0.012 af

6.0" Round Culvert $\,$ n=0.012 L=85.0' S=0.0351 '/' Outflow=0.15 cfs 0.012 af

Pond 82P: CB #23 Peak Elev=48.31' Inflow=1.30 cfs 0.102 af

12.0" Round Culvert n=0.012 L=18.0' S=0.0194 '/' Outflow=1.30 cfs 0.102 af

Pond 83P: Bio #4 / OS #22 Peak Elev=48.30' Storage=2,220 cf Inflow=1.63 cfs 0.127 af

Outflow=0.50 cfs 0.127 af

Link 100L: POA #100 - SW Inflow=3.78 cfs 0.430 af

Primary=3.78 cfs 0.430 af

Link 200L: POA #200 - South Inflow=23.20 cfs 4.580 af

Primary=23.20 cfs 4.580 af

Link 300L: POA #300 - SE Inflow=15.73 cfs 1.989 af

Primary=15.73 cfs 1.989 af

Total Runoff Area = 30.826 ac Runoff Volume = 7.127 af Average Runoff Depth = 2.77" 87.20% Pervious = 26.879 ac 12.80% Impervious = 3.947 ac Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

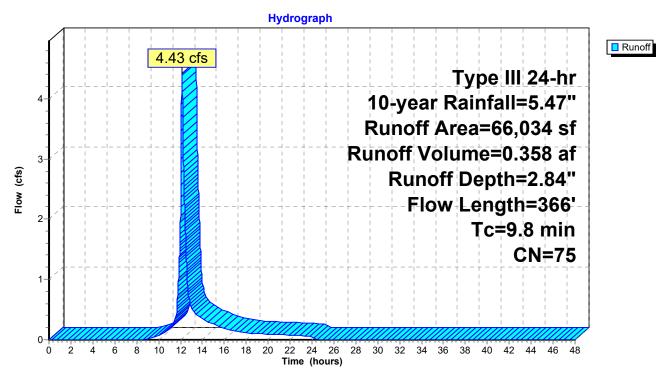
Summary for Subcatchment 1S: West Corner

Runoff = 4.43 cfs @ 12.14 hrs, Volume= 0.358 af, Depth= 2.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	А	rea (sf)	CN	Description				
		6,392	98	Paved parking, HSG C				
		35,003	74	>75% Ġras	s cover, Go	ood, HSG C		
		24,608	70	Woods, Go	od, HSG C			
		31	55	Woods, Go	od, HSG B			
		66,034	75	Weighted A	verage			
		59,642			rvious Area			
		6,392	!	9.68% Impe	ervious Area	a		
				-				
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	4.5	35	0.1071	0.13		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 3.61"		
	1.6	129	0.0388	1.38		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.6	55	0.0909	1.51		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	3.1	147	0.0254	0.80		Shallow Concentrated Flow,		
_						Woodland Kv= 5.0 fps		
	9.8	366	Total					

Subcatchment 1S: West Corner



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

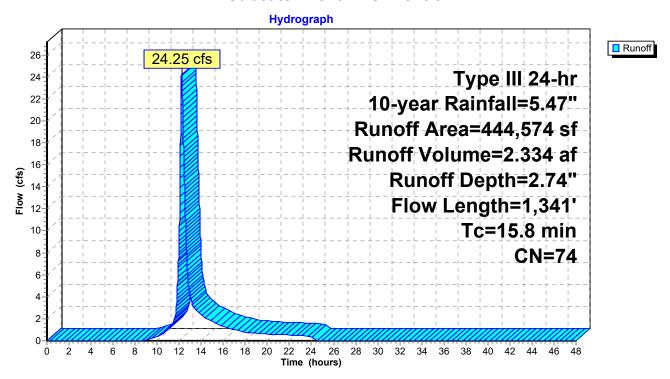
Summary for Subcatchment 2.1S: Central

Runoff = 24.25 cfs @ 12.22 hrs, Volume= 2.334 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN [Description				
	4,537	98 Paved parking, HSG D					
	25,978						
	5,448		1 07				
	49		•	ing, HSG B			
	3,520				od, HSG D		
	69,745			od, HSG D	, -		
	8,483		Brush, Goo	•			
	65,701				od, HSG C		
	48,205			od, HSG C	,		
	530			•	od, HSG B		
	12,378			od, HSG B	*		
4	44,574	74 V	Veighted A	verage			
4	08,562	ç	1.90% Per	vious Area			
	36,012	3	3.10% Impe	ervious Area	a e e e e e e e e e e e e e e e e e e e		
_				_			
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
0.5	35	0.0200	1.18		Sheet Flow,		
					Smooth surfaces n= 0.011 P2= 3.61"		
4.7	255	0.0165	0.90		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
2.0	132	0.0501	1.12		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.4	67	0.0313	2.85		Shallow Concentrated Flow,		
					Unpaved Kv= 16.1 fps		
1.0	89	0.0465	1.51		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
4.3	289	0.0500	1.12		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
2.9	474	0.0106	2.72	13.61	Trap/Vee/Rect Channel Flow,		
					Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'		
					n= 0.040 Winding stream, pools & shoals		
15.8	1,341	Total					

Subcatchment 2.1S: Central



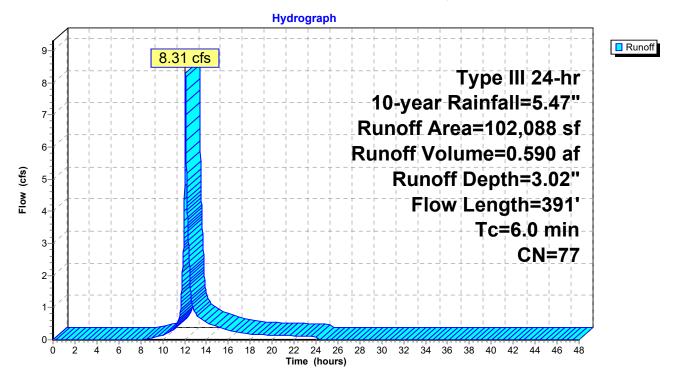
Summary for Subcatchment 2.2S: South of SQ Drive

Runoff = 8.31 cfs @ 12.09 hrs, Volume= 0.590 af, Depth= 3.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN E	Description				
	4,776	98 F	Paved parking, HSG D				
	5,561			ing, HSG C			
	639	80 >	·75% Gras	s cover, Go	ood, HSG D		
	52,111	77 V	Voods, Go	od, HSG D			
	1,323			,	ood, HSG C		
	37,678	70 V	Voods, Go	od, HSG C			
1	02,088		Veighted A				
	91,751	8	9.87% Per	vious Area			
	10,337	1	0.13% Imp	pervious Ar	ea		
_		01			B		
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
0.3	19	0.0200	1.05		Sheet Flow,		
					Smooth surfaces n= 0.011 P2= 3.61"		
1.4	118	0.0816	1.43		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.7	254	0.0502	5.93	29.63	Trap/Vee/Rect Channel Flow,		
					Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'		
					n= 0.040 Winding stream, pools & shoals		
2.4	391	Total, Increased to minimum Tc = 6.0 min					

Subcatchment 2.2S: South of SQ Drive



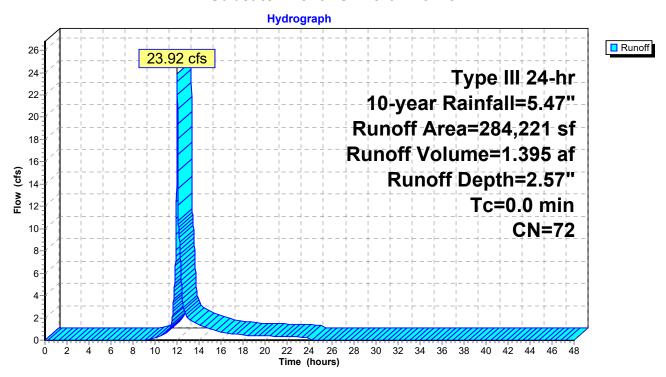
Summary for Subcatchment 2S: North Corner

Runoff = 23.92 cfs @ 12.00 hrs, Volume= 1.395 af, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

Area (sf)	CN	Description				
27	98	Paved parking, HSG D				
13,324	98	Paved parking, HSG C				
5,932	98	Roofs, HSG C				
254	80	>75% Grass cover, Good, HSG D				
20,767	77	Woods, Good, HSG D				
21,632	74	>75% Grass cover, Good, HSG C				
199,815	70	Woods, Good, HSG C				
22,470	55	Woods, Good, HSG B				
284,221	72	Weighted Average				
264,938		93.22% Pervious Area				
19,283		6.78% Impervious Area				

Subcatchment 2S: North Corner



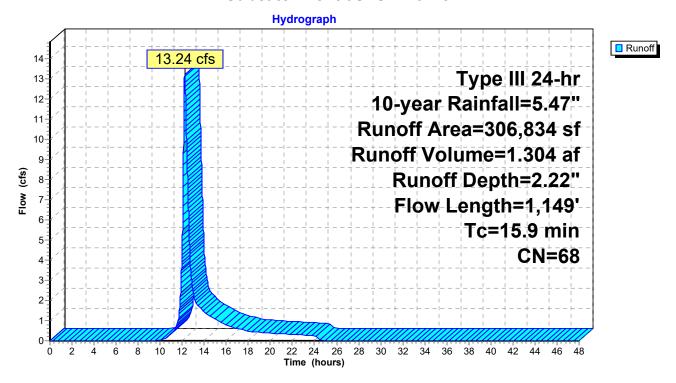
Summary for Subcatchment 3S: SE Corner

Runoff = 13.24 cfs @ 12.23 hrs, Volume= 1.304 af, Depth= 2.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN E	Description					
	223	98 F	B Paved parking, HSG D					
	5,324		Paved parking, HSG C					
	1,176	98 F	Paved park	ing, HSG B				
	16,453	74 >	75% Gras	s cover, Go	ood, HSG C			
2	234,823	70 V	Voods, Go	od, HSG C				
	1,816				ood, HSG B			
	47,019	55 V	Voods, Go	od, HSG B				
	806,834	5,834 68 Weighted Average						
3	300,111	-	-	vious Area				
	6,723	2	2.19% Impervious Area					
_								
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.5	90	0.1412	0.18		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.61"			
4.7	331	0.0554	1.18		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.9	331	0.0554	6.22	31.12	•			
					Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'			
4.0	007	0.0400	0.00	05.00	n= 0.040 Winding stream, pools & shoals			
1.8	397	0.0160	3.63	25.39	•			
					Bot.W=4.00' D=1.00' Z= 3.0 '/' Top.W=10.00'			
					n= 0.040 Winding stream, pools & shoals			
15.9	1,149	Total						

Subcatchment 3S: SE Corner



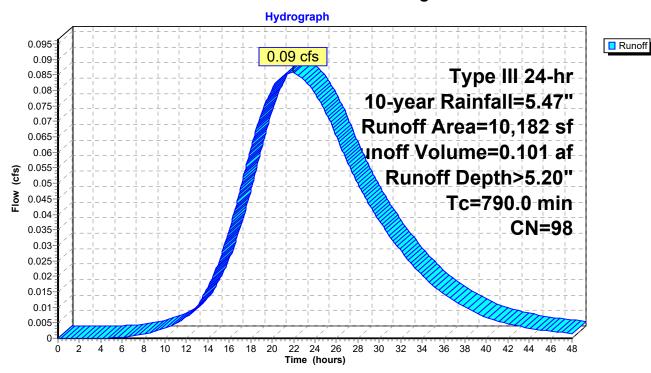
Summary for Subcatchment 10S: Parking Lot

Runoff = 0.09 cfs @ 21.94 hrs, Volume= 0.101 af, Depth> 5.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	10,078	98	Paved parking, HSG C						
	104	74	>75% Gras	s cover, Go	ood, HSG C				
	10,182	98	8 Weighted Average						
	104		1.02% Pervious Area						
	10,078		98.98% Imp	ervious Ar	rea				
Тс	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
790.0					Direct Entry,				

Subcatchment 10S: Parking Lot



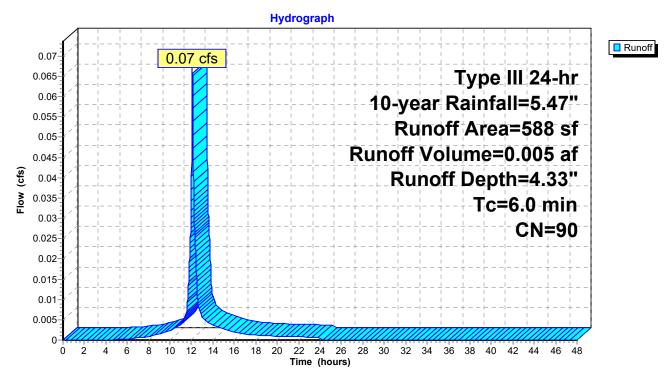
Summary for Subcatchment 11S: Island

Runoff = 0.07 cfs @ 12.08 hrs, Volume= 0.005 af, Depth= 4.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description							
	321	98	Roofs, HSG C							
	66	98	Paved parking, HSG C							
	201	74	>75% Ġras	75% Grass cover, Good, HSG C						
	588	90	Veighted Average							
	201		34.18% Per	vious Area						
	387		65.82% lmp	ervious Ar	ea					
Tc	Length	Slope	 Velocity 	Capacity	Description					
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
6.0					Direct Entry					

Subcatchment 11S: Island



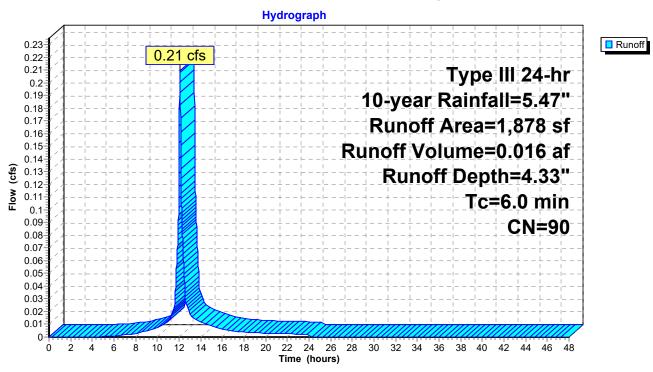
Summary for Subcatchment 12S: Building

Runoff = 0.21 cfs @ 12.08 hrs, Volume= 0.016 af, Depth= 4.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

Ar	ea (sf)	CN	Description							
	1,171	98	Roofs, HSG C							
	52	98	Paved parking, HSG C							
	655	74	>75% Ġras:	75% Grass cover, Good, HSG C						
•	1,878	90	Veighted Average							
	655		34.88% Per	vious Area						
	1,223		65.12% Imp	ervious Are	ea					
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry.					

Subcatchment 12S: Building



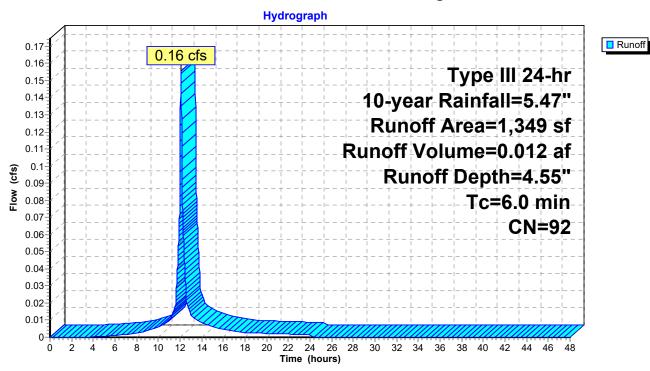
Summary for Subcatchment 13S: Building

Runoff = 0.16 cfs @ 12.08 hrs, Volume= 0.012 af, Depth= 4.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

_	Α	rea (sf)	CN	Description							
_		996	98	Roofs, HSG C							
_		353	74	>75% Gras	75% Grass cover, Good, HSG C						
		1,349	92	Weighted A	/eighted Average						
		353		26.17% Per	rvious Area						
		996		73.83% Imp	pervious Ar	ea					
	То	Longth	Clan	. Valacity	Canacity	Description					
	Tc	Length	Slope	,	Capacity	Description					
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
_	6.0					Direct Entry					

Subcatchment 13S: Building



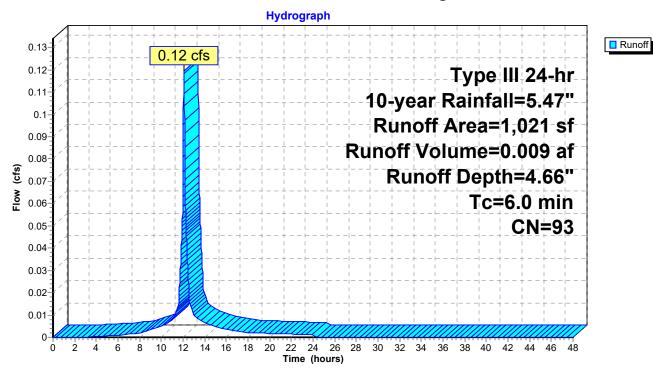
Summary for Subcatchment 14S: Building

Runoff = 0.12 cfs @ 12.08 hrs, Volume= 0.009 af, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Area (sf)	CN	Description						
	795	98	Roofs, HSG C						
	226	74	>75% Grass cover, Good, HSG C						
	1,021	93	Weighted A	Veighted Average					
	226		22.14% Pervious Area						
	795		77.86% lmp	pervious Ar	ea				
_									
Tc	9	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry				

Subcatchment 14S: Building



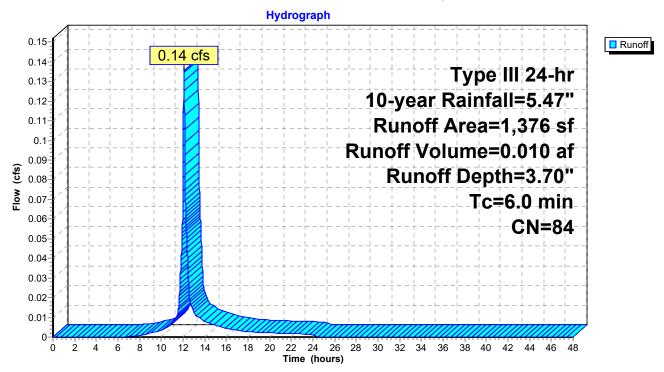
Summary for Subcatchment 15S: Building

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 0.010 af, Depth= 3.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description							
	484	98	Roofs, HSG C							
	111	98	Paved parking, HSG C							
	781	74	>75% Gras	75% Grass cover, Good, HSG C						
	1,376	84	Weighted A	Veighted Average						
	781		56.76% Per	vious Area						
	595		43.24% Imp	ervious Are	ea					
_				_						
Tc	Length	Slope	,	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry.					

Subcatchment 15S: Building



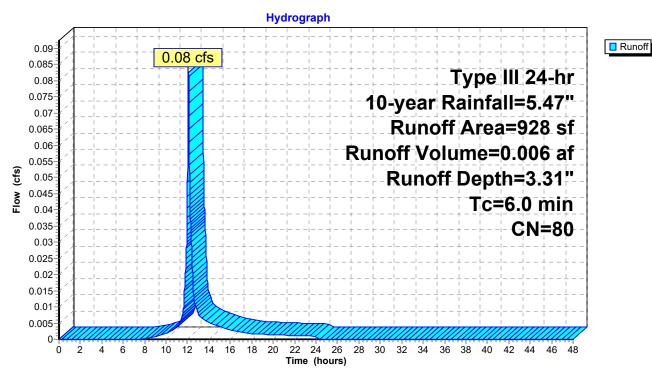
Summary for Subcatchment 16S: Island

Runoff = 0.08 cfs @ 12.09 hrs, Volume= 0.006 af, Depth= 3.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description							
	246	98	Paved parking, HSG C							
	682	74	>75% Grass cover, Good, HSG C							
	928	80	Weighted A	/eighted Average						
	682		73.49% Pervious Area							
	246		26.51% Impervious Area							
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft	t) (ft/sec) (cfs)							
6.0					Direct Entry,					

Subcatchment 16S: Island



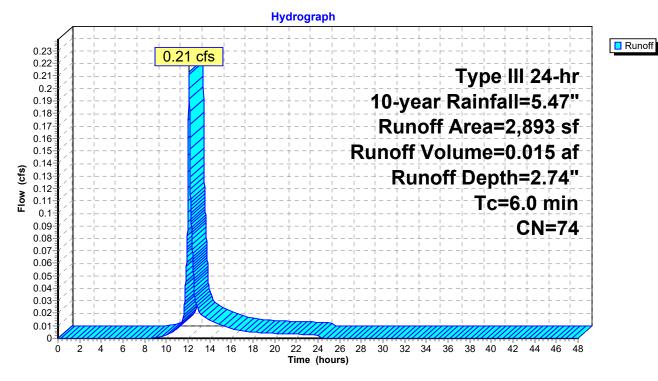
Summary for Subcatchment 17S: To Bio #1

Runoff = 0.21 cfs @ 12.09 hrs, Volume= 0.015 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Area (sf)	CN	Description							
	8	98	Paved parking, HSG C							
	2,885	74	>75% Gras	75% Grass cover, Good, HSG C						
	2,893	74	Weighted A	/eighted Average						
	2,885		99.72% Per	vious Area						
	8		0.28% Impe	ervious Area	a					
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)		(ft/ft	,	(cfs)	Becomplien					
6.0		(13,11	, , , , , , , , ,	()	Direct Entry					

Subcatchment 17S: To Bio #1



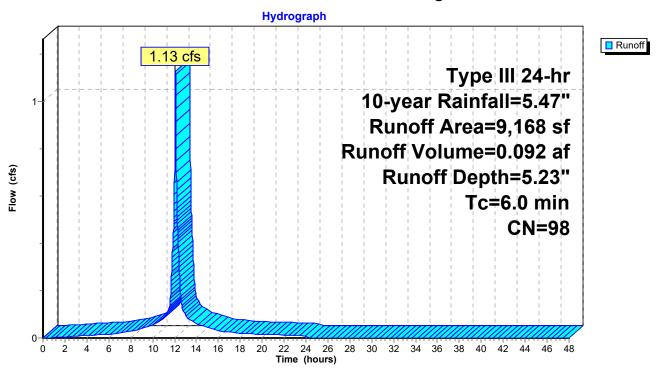
Summary for Subcatchment 20S: Building

Runoff = 1.13 cfs @ 12.08 hrs, Volume= 0.092 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN [Description					
	9,168	98 F	Roofs, HSG C					
	9,168	,	100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0	,	, ,	,	, ,	Direct Entry,			

Subcatchment 20S: Building



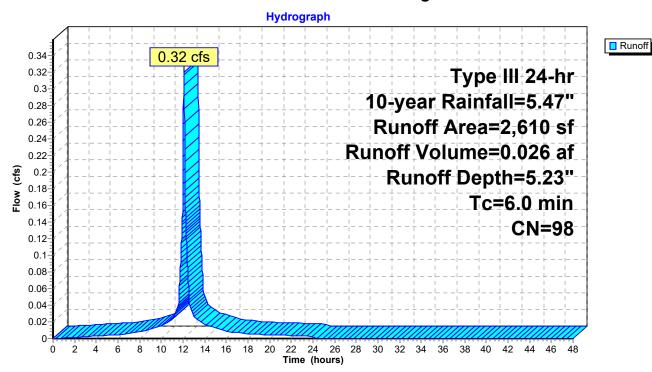
Summary for Subcatchment 21S: Parking Lot

Runoff = 0.32 cfs @ 12.08 hrs, Volume= 0.026 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

 Α	rea (sf)	CN	Description							
	331	98	Roofs, HSG C							
	2,279	98	Paved park	Paved parking, HSG C						
	2,610	98	Weighted A	Veighted Average						
	2,610		100.00% Impervious Area							
Tc	Length	Slop	,	Capacity	Description					
 (min)	(feet)	(ft/f	t) (ft/sec)	(cfs)						
6.0					Direct Entry,					

Subcatchment 21S: Parking Lot



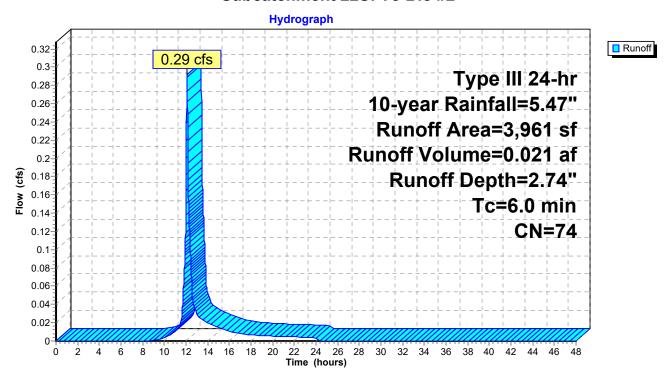
Summary for Subcatchment 22S: To Bio #2

Runoff = 0.29 cfs @ 12.09 hrs, Volume= 0.021 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Ar	ea (sf)	CN	Description						
		3,961	74	>75% Grass cover, Good, HSG C						
		3,961		100.00% Pervious Area						
(m	Tc iin)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	•				
	6.0					Direct Entry,				

Subcatchment 22S: To Bio #2



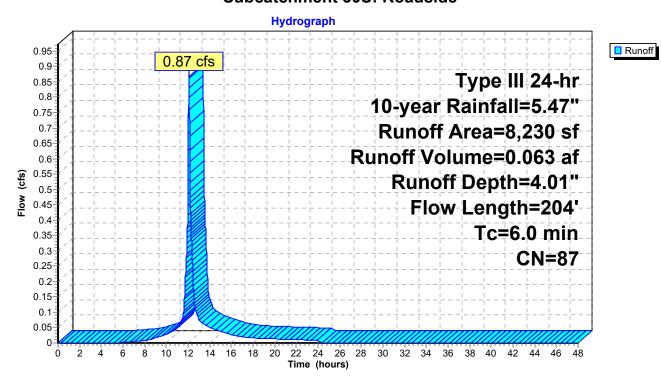
Summary for Subcatchment 30S: Roadside

Runoff = 0.87 cfs @ 12.09 hrs, Volume= 0.063 af, Depth= 4.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN D	escription						
		4,370	98 P	98 Paved parking, HSG C						
		3,860	74 >	75% Grass	s cover, Go	ood, HSG C				
		8,230	87 V	87 Weighted Average						
		3,860	4	6.90% Per	vious Area					
		4,370	5	3.10% Imp	ervious Are	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	0.3	19	0.0200	1.05		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 3.61"				
	2.9	185	0.0235	1.07		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	3.2	204	Total, I	ncreased t	o minimum	Tc = 6.0 min				

Subcatchment 30S: Roadside



Summary for Subcatchment 31S: Roadside

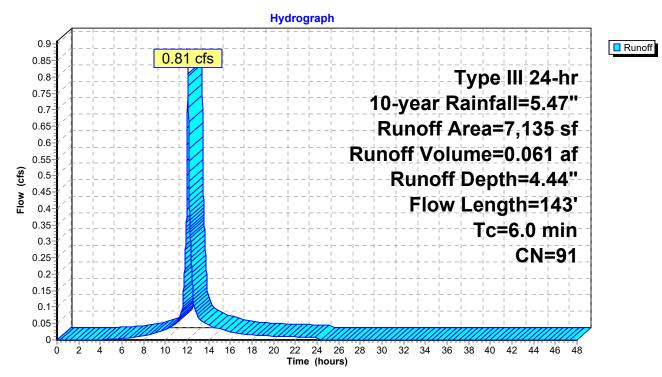
Runoff = 0.81 cfs @ 12.08 hrs, Volume= 0.061 af, Depth= 4.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

		(5)	ON -								
_	A	rea (sf)	CN E	CN Description							
		5,074	98 F	98 Paved parking, HSG C							
		2,061		1							
_		7,135	91 V	Veighted A	verage						
		2,061			vious Area						
		5,074	7	1.11% Imp	ervious Ar	ea					
		-,		· · · · · · · · · · · · · · · · · · ·							
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•					
_	0.3	17	0.0200	1.03	` '	Sheet Flow,					
						Smooth surfaces n= 0.011 P2= 3.61"					
	3.0	126	0.0100	0.70		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
_	2.2	4.40	T-4-1 1	Tatal In an analytic main income Tage C.O. main							

3.3 143 Total, Increased to minimum Tc = 6.0 min

Subcatchment 31S: Roadside



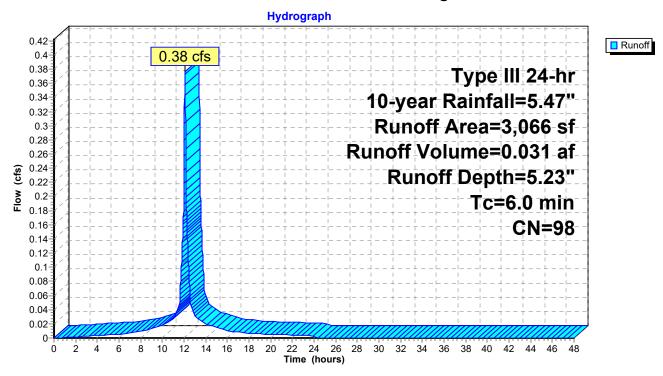
Summary for Subcatchment 40S: Building

Runoff = 0.38 cfs @ 12.08 hrs, Volume= 0.031 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN I	Description						
	3,066	98 F	Roofs, HSG C						
	3,066	•	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment 40S: Building



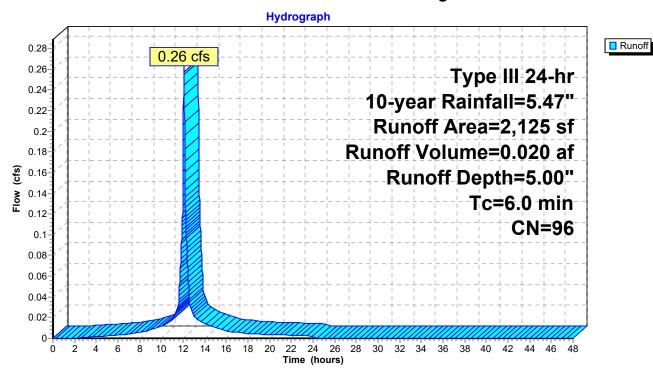
Summary for Subcatchment 41S: Building

Runoff = 0.26 cfs @ 12.08 hrs, Volume= 0.020 af, Depth= 5.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	1,929	98	Roofs, HSG C						
	196	74	>75% Grass cover, Good, HSG C						
	2,125	96	Weighted Average						
	196		9.22% Pervious Area						
	1,929		90.78% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 41S: Building



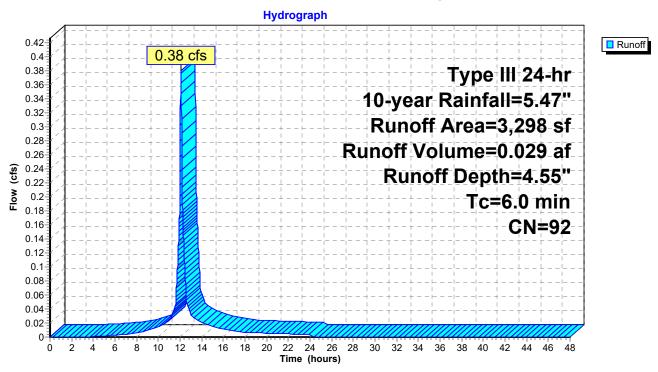
Summary for Subcatchment 42S: Building

Runoff = 0.38 cfs @ 12.08 hrs, Volume= 0.029 af, Depth= 4.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	292	98	Paved parking, HSG C						
	2,162	98	Roofs, HSG C						
	844	74	>75% Grass	s cover, Go	ood, HSG C				
	3,298	92	Weighted Average						
	844		25.59% Pervious Area						
	2,454		74.41% Impervious Area						
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry.				

Subcatchment 42S: Building



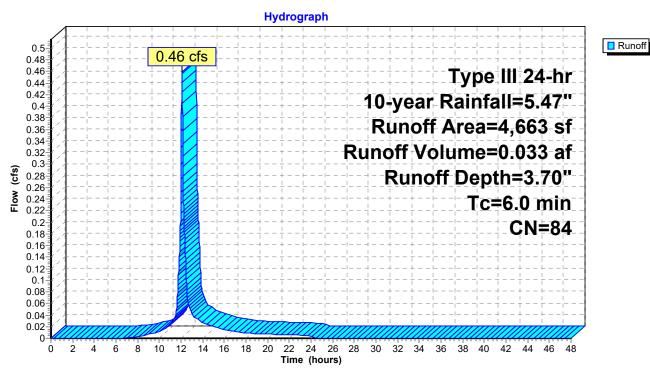
Summary for Subcatchment 44S: Roadside

Runoff = 0.46 cfs @ 12.09 hrs, Volume= 0.033 af, Depth= 3.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN	Description						
		1,908	98	Paved parking, HSG C						
		2,755	74	>75% Gras	>75% Grass cover, Good, HSG C					
		4,663	84	Weighted Average						
		2,755		59.08% Pervious Area						
		1,908		40.92% Impervious Area						
	Т-	ما المسمد م	Clan	. \/alaaitu	Conneitu	Description				
	Tc	Length	Slope	,	Capacity	Description				
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
•	6.0			•		Direct Entry				

Subcatchment 44S: Roadside



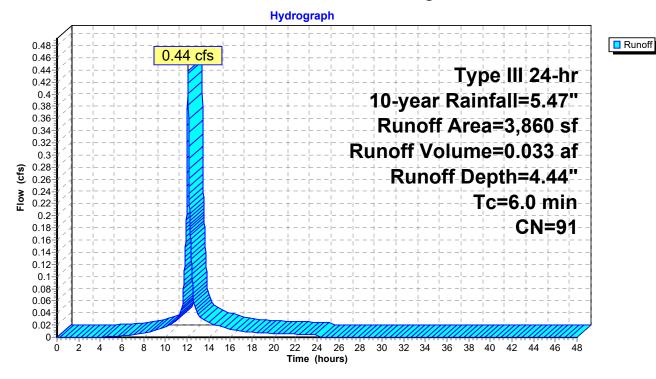
Summary for Subcatchment 45S: Parking Lot

Runoff = 0.44 cfs @ 12.08 hrs, Volume= 0.033 af, Depth= 4.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	2,768	98	Paved parking, HSG C						
	1,092	74	>75% Grass cover, Good, HSG C						
	3,860	91	Weighted Average						
	1,092		28.29% Pervious Area						
	2,768		71.71% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 45S: Parking Lot



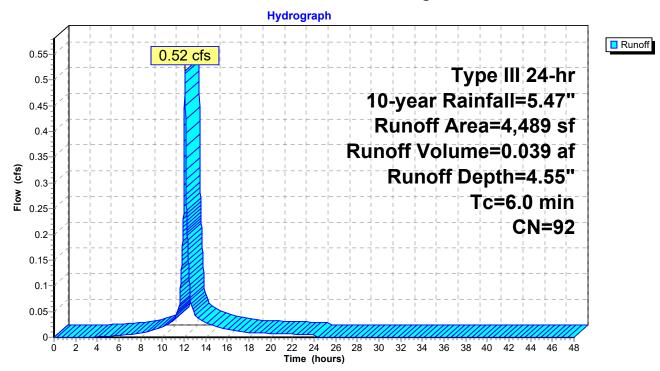
Summary for Subcatchment 46S: Parking Lot

Runoff = 0.52 cfs @ 12.08 hrs, Volume= 0.039 af, Depth= 4.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	3,288	98	Paved parking, HSG C						
	1,201	74	>75% Grass cover, Good, HSG C						
	4,489	92	Weighted Average						
	1,201		26.75% Pervious Area						
	3,288		73.25% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 46S: Parking Lot



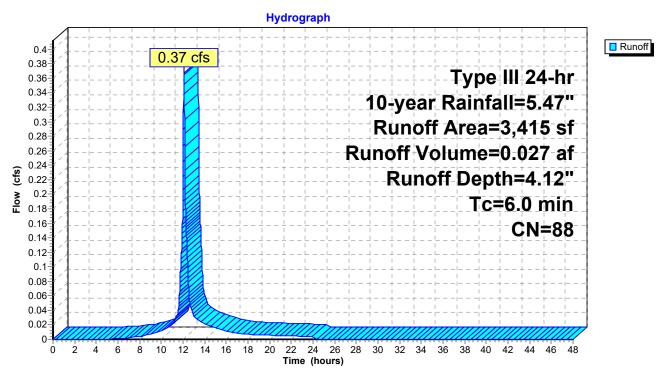
Summary for Subcatchment 48S: Roadside

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 0.027 af, Depth= 4.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description							
	245	98	Paved parking, HSG D							
	1,051	98	Paved park	ng, HSG C	,					
	605	98	Roofs, HSG	i Č						
	229	80	>75% Grass	>75% Grass cover, Good, HSG D						
	1,268	74	>75% Grass	>75% Grass cover, Good, HSG C						
	17	61	>75% Grass cover, Good, HSG B							
	3,415	88	88 Weighted Average							
	1,514		44.33% Per	vious Area						
	1,901		55.67% Imp	ervious Ar	ea					
Тс	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry,					

Subcatchment 48S: Roadside



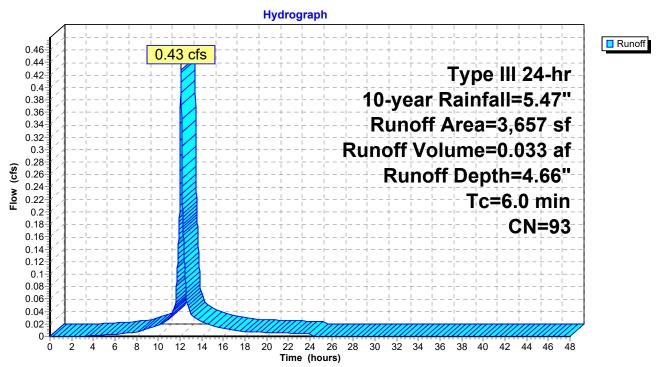
Summary for Subcatchment 49S: Driveway

Runoff = 0.43 cfs @ 12.08 hrs, Volume= 0.033 af, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	700	98	Paved park	ing, HSG D)				
	1,047	98	Paved park	ing, HSG C	,				
	1,317	98	Paved park	ing, HSG B	}				
	4	80	>75% Grass	s cover, Go	ood, HSG D				
	174	74	>75% Grass	>75% Grass cover, Good, HSG C					
	415	61	>75% Grass cover, Good, HSG B						
	3,657	93	Weighted Average						
	593		16.22% Per	vious Area					
	3,064		83.78% Imp	ervious Ar	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 49S: Driveway



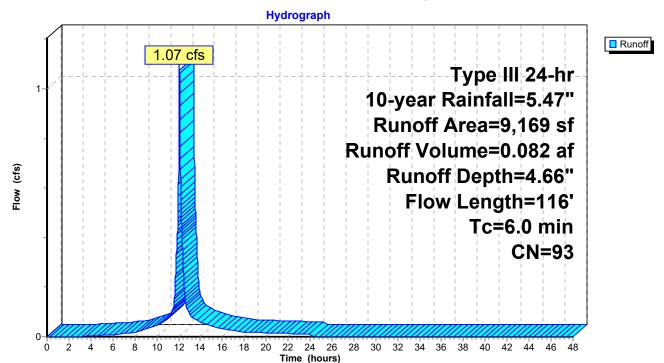
Summary for Subcatchment 60S: Parking Lot

1.07 cfs @ 12.08 hrs, Volume= Runoff 0.082 af, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Area (sf)	CN	Description						
	7,202	98	Paved park	Paved parking, HSG C					
	1,482	74	>75% Ġras	75% Grass cover, Good, HSG C					
	485	70	Voods, Good, HSG C						
	9,169	93	Weighted Average						
	1,967		21.45% Pervious Area						
	7,202	•	78.55% lmp	ervious Ar	ea				
			-						
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
0.4	24	0.0200	1.10		Sheet Flow,				
					Smooth surfaces n= 0.011 P2= 3.61"				
0.8	92	0.0100	2.03		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
1.2	116	Total,	Increased t	o minimum	Tc = 6.0 min				

Subcatchment 60S: Parking Lot



Summary for Subcatchment 61S: Parking Lot

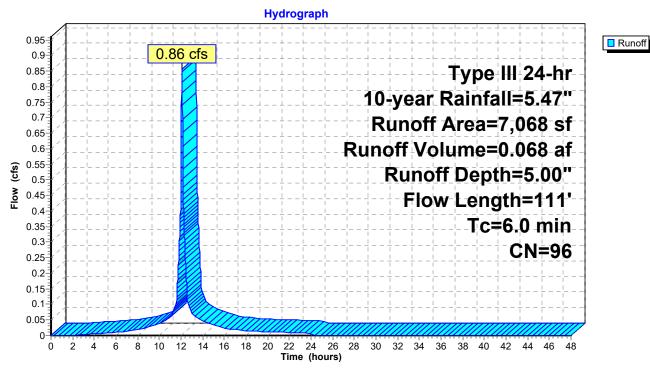
Runoff = 0.86 cfs @ 12.08 hrs, Volume= 0.068 af, Depth= 5.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN [Description		
	4,574	98 F	Paved park	ing, HSG C	
	1,927	98 F	Paved park	ing, HSG B	3
	335	74 >	∙75% Ġras	s cover, Go	ood, HSG C
	232	61 >	75% Gras	s cover, Go	ood, HSG B
	7,068	96 V	Veighted A	verage	
	567	3	3.02% Perv	ious Area	
	6,501	ç	1.98% Imp	pervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
0.4	24	0.0200	1.10		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 3.61"
0.7	87	0.0100	2.03		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps

1.1 Total, Increased to minimum Tc = 6.0 min

Subcatchment 61S: Parking Lot



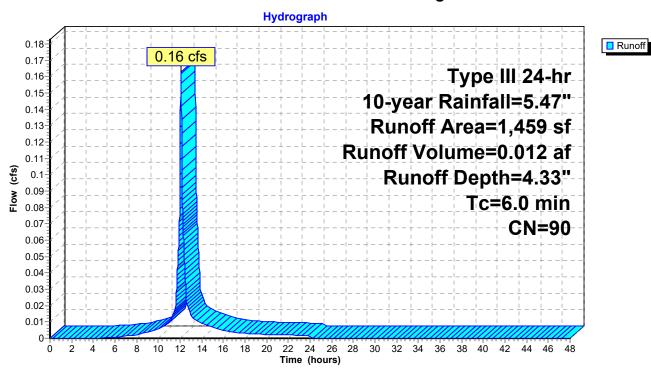
Summary for Subcatchment 63S: Building

Runoff = 0.16 cfs @ 12.08 hrs, Volume= 0.012 af, Depth= 4.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN	Description							
		956	98	Roofs, HSG	G C						
		503	74	>75% Gras	>75% Grass cover, Good, HSG C						
		1,459	90	Weighted A	Weighted Average						
		503		34.48% Per	vious Area						
		956		65.52% Imp	pervious Ar	ea					
	Τ.	1 41.	01		0	D in the					
	Tc	Length	Slope	e Velocity	Capacity	Description					
(r	min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
	6.0					Direct Entry					

Subcatchment 63S: Building



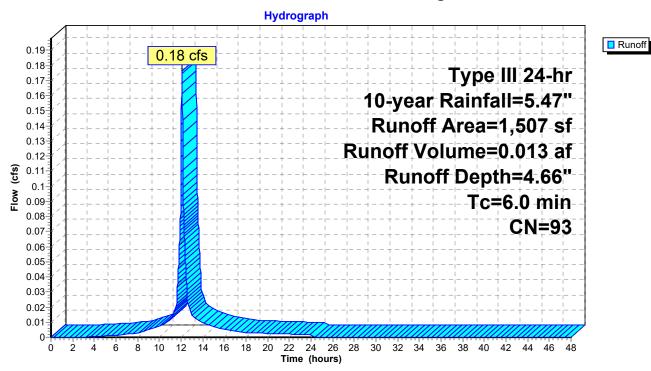
Summary for Subcatchment 64S: Building

Runoff = 0.18 cfs @ 12.08 hrs, Volume= 0.013 af, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN	Description					
		1,224	98	Roofs, HSG	G C				
		283	74	>75% Gras	s cover, Go	od, HSG C			
		1,507	93	Weighted Average					
		283		18.78% Per	vious Area				
		1,224		81.22% lmp	pervious Ar	ea			
	_		٥.						
	Tc	Length	Slope	,	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry			

Subcatchment 64S: Building



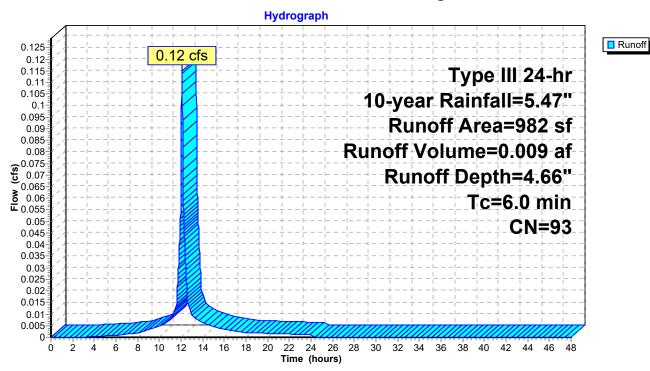
Summary for Subcatchment 65S: Building

Runoff = 0.12 cfs @ 12.08 hrs, Volume= 0.009 af, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

_	Α	rea (sf)	CN	Description							
		794	98	Roofs, HSG	C						
_		188	74	>75% Gras	>75% Grass cover, Good, HSG C						
		982	93	Weighted A	Weighted Average						
		188		19.14% Pervious Area							
		794		80.86% Impervious Area							
	т.	1 41-	Cl		0	Daganindian					
	Tc	Length	Slope	,	Capacity	Description					
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
	6.0					Direct Entry					

Subcatchment 65S: Building



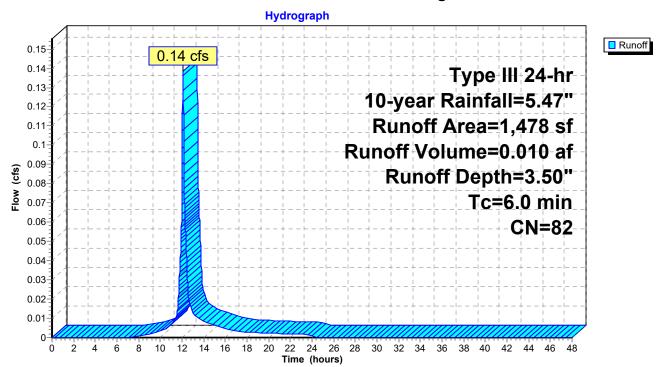
Summary for Subcatchment 66S: Building

Runoff = 0.14 cfs @ 12.09 hrs, Volume= 0.010 af, Depth= 3.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description				
	25	98	Paved parki	ng, HSG C	,		
	161	98	Roofs, HSG	Č			
	476	98	Roofs, HSG	В			
	486	74	>75% Grass	s cover, Go	ood, HSG C		
	330	61	>75% Grass	s cover, Go	ood, HSG B		
•	1,478	82	Weighted Average				
	816		55.21% Per	vious Area			
	662		44.79% Imp	ervious Ar	ea		
				_			
Тс	Length	Slope	•	Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment 66S: Building



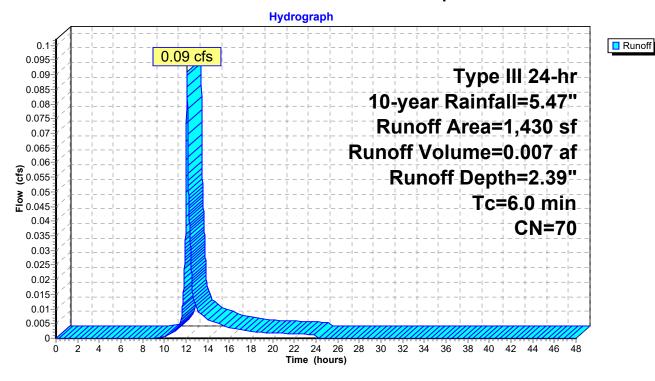
Summary for Subcatchment 67S: Landscape

Runoff = 0.09 cfs @ 12.09 hrs, Volume= 0.007 af, Depth= 2.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN	Description						
	331	98	Paved park	ing, HSG B	В				
	1,099	61	>75% Gras	s cover, Go	lood, HSG B				
	1,430	70	Weighted Average						
	1,099		76.85% Pervious Area						
	331		23.15% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u>'</u>				
6.0					Direct Entry,				

Subcatchment 67S: Landscape



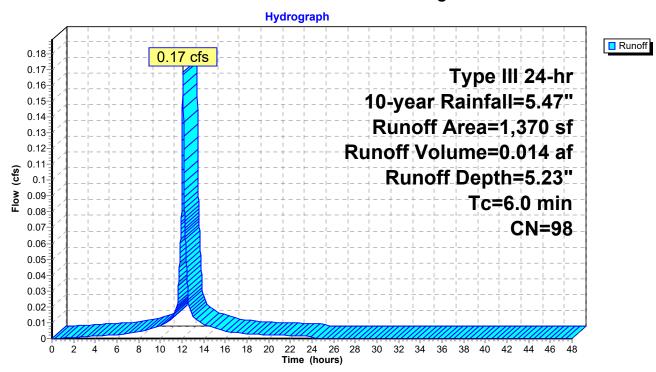
Summary for Subcatchment 68S: Building

Runoff = 0.17 cfs @ 12.08 hrs, Volume= 0.014 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

_	Α	rea (sf)	CN	Description							
		424	98	Roofs, HSG	Roofs, HSG C						
_		946	98	Roofs, HSG	Roofs, HSG B						
_		1,370	98	Weighted A	Weighted Average						
		1,370		100.00% Impervious Area							
	Tc	Length	Slop	,	Capacity	Description					
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
	6.0					Direct Entry					

Subcatchment 68S: Building



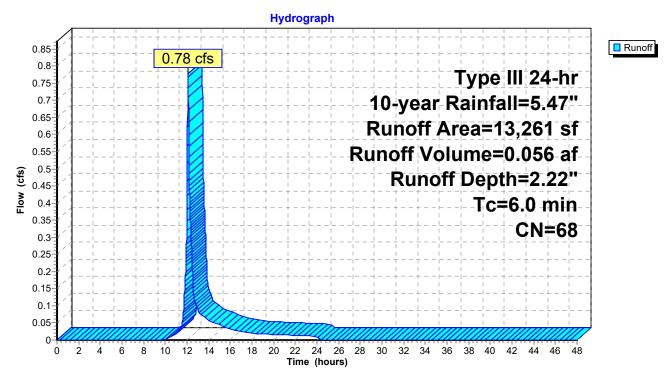
Summary for Subcatchment 69S: To Pond

Runoff = 0.78 cfs @ 12.09 hrs, Volume= 0.056 af, Depth= 2.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

Area (sf)	CN	Description						
224	98	Paved parking, HSG B						
6,374	74	>75% Grass cover, Good, HSG C						
6,663	61	>75% Grass cover, Good, HSG B						
13,261	68	Weighted Average						
13,037		98.31% Pervious Area						
224		1.69% Impervious Area						
Tc Length	Slop	pe Velocity Capacity Description						
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)						
6.0		Direct Entry.						

Subcatchment 69S: To Pond



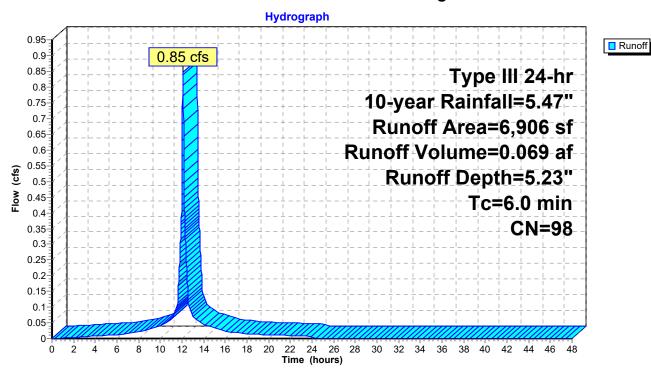
Summary for Subcatchment 70S: Building

Runoff = 0.85 cfs @ 12.08 hrs, Volume= 0.069 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

_	Α	rea (sf)	CN	Description					
		6,802	98	Roofs, HSG C					
_		104	98	Roofs, HSG	B				
		6,906	98	Weighted Average					
		6,906		100.00% Impervious Area					
	Тс	Length	Slope	,	Capacity	• • • • • • • • • • • • • • • • • • •			
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	6.0					Direct Entry			

Subcatchment 70S: Building



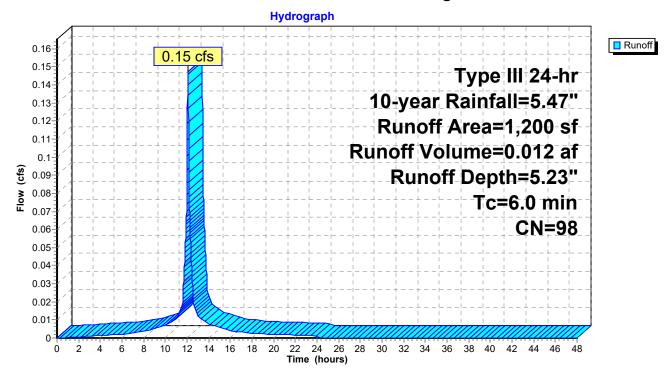
Summary for Subcatchment 80S: Building

Runoff = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

A	rea (sf)	CN I	Description						
	1,200	98 F	8 Roofs, HSG C						
	1,200	•	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment 80S: Building



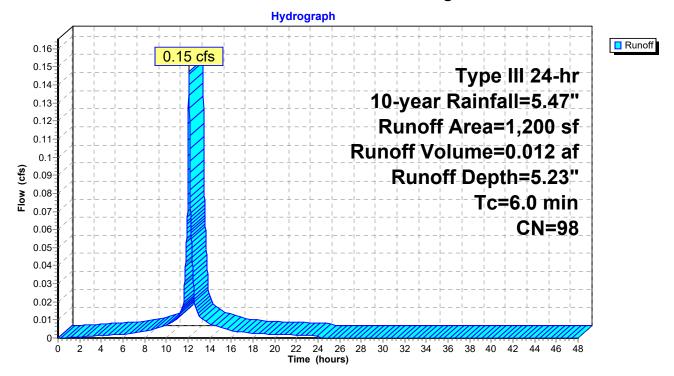
Summary for Subcatchment 81S: Building

Runoff = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af, Depth= 5.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	rea (sf)	CN I	Description				
	1,200	98 I	Roofs, HSG C				
	1,200	•	100.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry,		

Subcatchment 81S: Building



Summary for Subcatchment 82S: Parking Lot

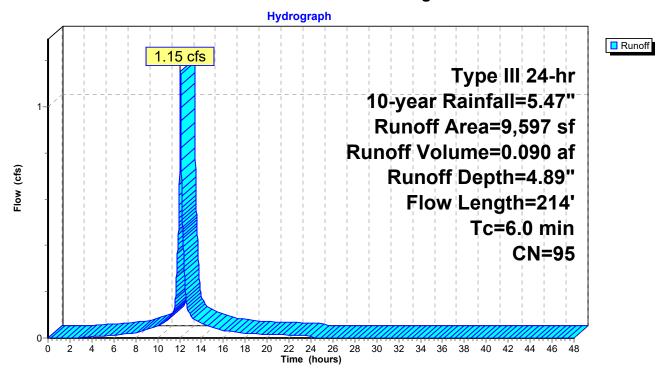
Runoff = 1.15 cfs @ 12.08 hrs, Volume= 0.090 af, Depth= 4.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

	Α	rea (sf)	CN	Description						
		8,215	98	98 Paved parking, HSG C						
		460	98	1 3'						
		302	74	>75% Ġras	s cover, Go	ood, HSG C				
		620	61	>75% Gras	s cover, Go	ood, HSG B				
		9,597	95	95 Weighted Average						
		922		9.61% Perv	ious Area					
		8,675		90.39% Imp	pervious Ar	ea				
	Tc	Length	Slope	,	Capacity	Description				
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
	2.2	29	0.063°	1 0.22		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.61"				
	0.4	95	0.0400	4.06		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.7	90	0.0100	2.03		Shallow Concentrated Flow,				
_						Paved Kv= 20.3 fps				
	2 2	21/	Total	Increased t	a minimum	$T_{c} = 6.0 \text{ min}$				

3.3 214 Total, Increased to minimum Tc = 6.0 min

Subcatchment 82S: Parking Lot



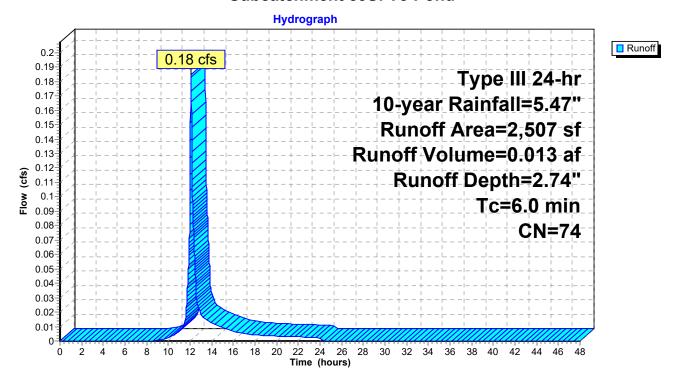
Summary for Subcatchment 83S: To Pond

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 0.013 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=5.47"

_	Α	rea (sf)	CN	Description				
		2,507	74	75% Grass cover, Good, HSG C				
		2,507		100.00% Pervious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	•		
	6.0					Direct Entry,		

Subcatchment 83S: To Pond



Summary for Reach 2.1R: Wetland Flow Path

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 18.63 cfs @ 12.05 hrs, Volume= 1.395 af

Outflow = 16.73 cfs @ 12.09 hrs, Volume= 1.395 af, Atten= 10%, Lag= 2.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.53 fps, Min. Travel Time= 2.3 min Avg. Velocity = 0.72 fps, Avg. Travel Time= 7.9 min

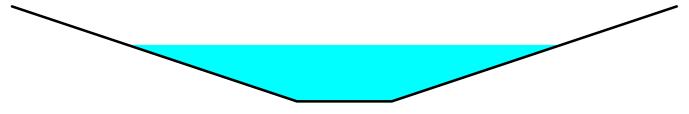
Peak Storage= 2,278 cf @ 12.09 hrs Average Depth at Peak Storage= 1.19' Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 54.59 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

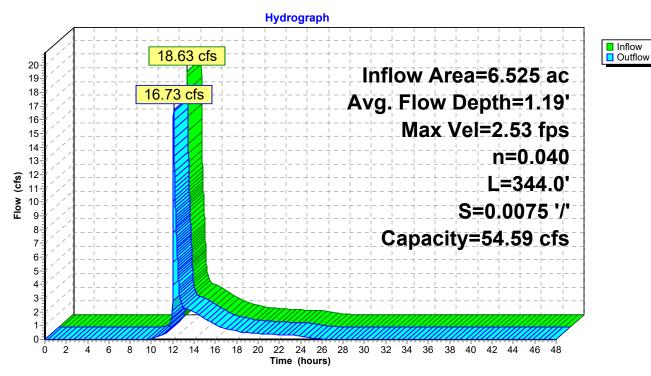
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 344.0' Slope= 0.0075 '/'

Inlet Invert= 52.58', Outlet Invert= 50.00'



Reach 2.1R: Wetland Flow Path



Summary for Reach 2.2R: Wetland Flow Path

Inflow Area = 17.445 ac, 10.07% Impervious, Inflow Depth > 2.74" for 10-year event

Inflow = 19.28 cfs @ 12.48 hrs, Volume= 3.990 af

Outflow = 19.04 cfs @ 12.48 hrs, Volume= 3.990 af, Atten= 1%, Lag= 0.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 5.28 fps, Min. Travel Time= 0.8 min Avg. Velocity = 1.35 fps, Avg. Travel Time= 3.2 min

Peak Storage= 949 cf @ 12.48 hrs Average Depth at Peak Storage= 0.81'

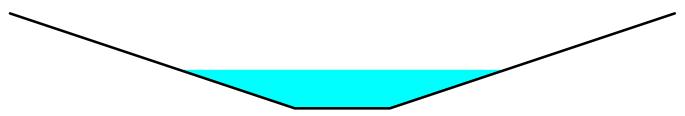
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 141.18 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

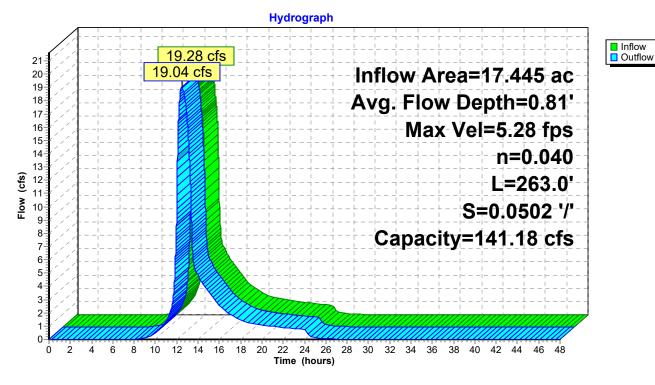
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 263.0' Slope= 0.0502 '/'

Inlet Invert= 46.52', Outlet Invert= 33.33'



Reach 2.2R: Wetland Flow Path



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Reach 2R: Wetland Flow Path

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 18.70 cfs @ 12.05 hrs, Volume= 1.395 af

Outflow = 18.63 cfs @ 12.05 hrs, Volume= 1.395 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 5.02 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.45 fps, Avg. Travel Time= 1.7 min

Peak Storage= 560 cf @ 12.05 hrs Average Depth at Peak Storage= 0.83'

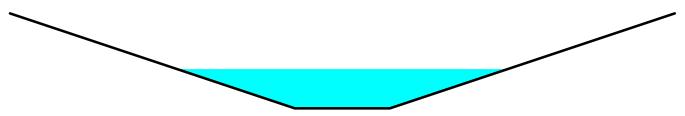
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 133.09 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

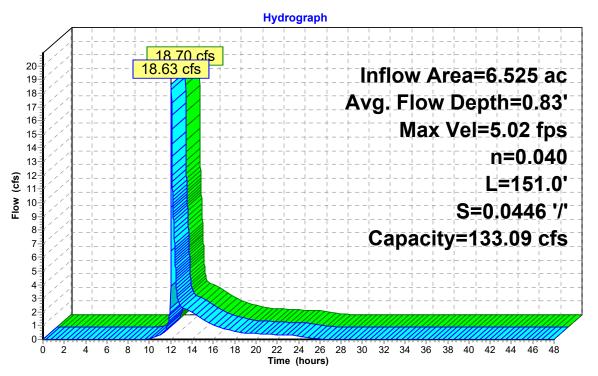
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 151.0' Slope= 0.0446 '/'

Inlet Invert= 59.31', Outlet Invert= 52.58'



Reach 2R: Wetland Flow Path



Summary for Reach 10R: Swale

Inflow Area = 0.230 ac, 42.36% Impervious, Inflow Depth > 3.73" for 10-year event

Inflow = 0.30 cfs @ 12.42 hrs, Volume= 0.072 af

Outflow = 0.30 cfs @ 12.43 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 0.64 fps, Min. Travel Time= 1.1 min Avg. Velocity = 0.20 fps, Avg. Travel Time= 3.7 min

Peak Storage= 20 cf @ 12.43 hrs Average Depth at Peak Storage= 0.11'

Bank-Full Depth= 1.00' Flow Area= 7.0 sf, Capacity= 16.22 cfs

4.00' x 1.00' deep channel, n= 0.035 Earth, dense weeds

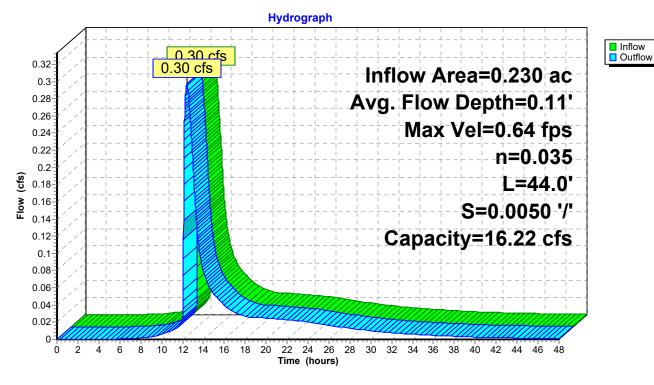
Side Slope Z-value= 3.0 '/' Top Width= 10.00'

Length= 44.0' Slope= 0.0050 '/'

Inlet Invert= 53.60', Outlet Invert= 53.38'



Reach 10R: Swale



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Reach 22R: Wetland Flow Path

Inflow Area = 0.361 ac, 74.83% Impervious, Inflow Depth > 4.56" for 10-year event

Inflow = 0.44 cfs @ 12.46 hrs, Volume= 0.137 af

Outflow = 0.44 cfs @ 12.51 hrs, Volume= 0.137 af, Atten= 0%, Lag= 3.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 0.80 fps, Min. Travel Time= 4.4 min Avg. Velocity = 0.44 fps, Avg. Travel Time= 8.0 min

Peak Storage= 116 cf @ 12.51 hrs Average Depth at Peak Storage= 0.05'

Bank-Full Depth= 2.00' Flow Area= 60.0 sf, Capacity= 392.58 cfs

10.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

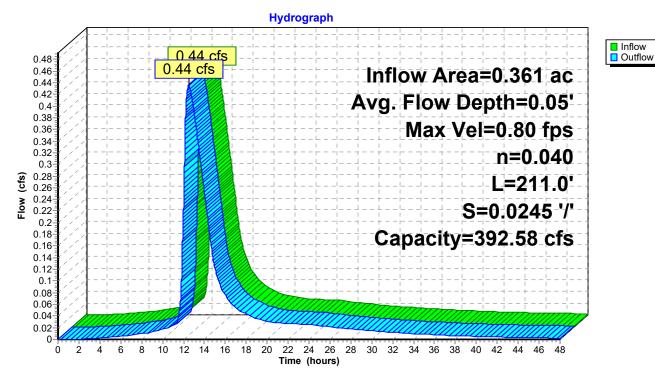
Side Slope Z-value= 10.0 '/' Top Width= 50.00'

Length= 211.0' Slope= 0.0245 '/'

Inlet Invert= 55.16', Outlet Invert= 50.00'



Reach 22R: Wetland Flow Path



Summary for Reach 23R: Wetland Flow Path

Inflow Area = 6.886 ac, 10.36% Impervious, Inflow Depth > 2.67" for 10-year event

Inflow = 17.02 cfs @ 12.09 hrs, Volume= 1.532 af

Outflow = 16.91 cfs @ 12.10 hrs, Volume= 1.532 af, Atten= 1%, Lag= 0.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.54 fps, Min. Travel Time= 0.6 min Avg. Velocity = 0.74 fps, Avg. Travel Time= 2.9 min

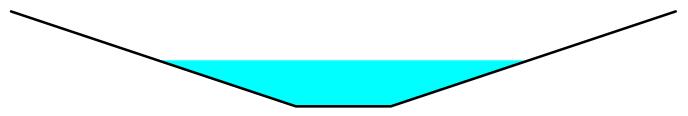
Peak Storage= 625 cf @ 12.10 hrs Average Depth at Peak Storage= 0.97' Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 85.86 cfs

2.00' x 2.00' deep channel, n= 0.040 Winding stream, pools & shoals

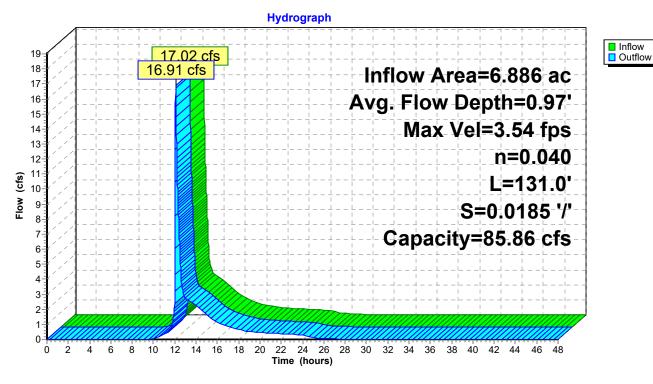
Side Slope Z-value= 3.0 '/' Top Width= 14.00'

Length= 131.0' Slope= 0.0185 '/'

Inlet Invert= 50.00', Outlet Invert= 47.57'



Reach 23R: Wetland Flow Path



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Reach 69R: Swale

Inflow Area = 1.681 ac, 63.59% Impervious, Inflow Depth > 4.00" for 10-year event

Inflow = 2.52 cfs @ 12.38 hrs, Volume= 0.560 af

Outflow = 2.52 cfs @ 12.38 hrs, Volume= 0.560 af, Atten= 0%, Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.22 fps, Min. Travel Time= 0.2 min Avg. Velocity = 0.34 fps, Avg. Travel Time= 0.8 min

Peak Storage= 35 cf @ 12.38 hrs Average Depth at Peak Storage= 0.40'

Bank-Full Depth= 1.00' Flow Area= 7.0 sf, Capacity= 14.11 cfs

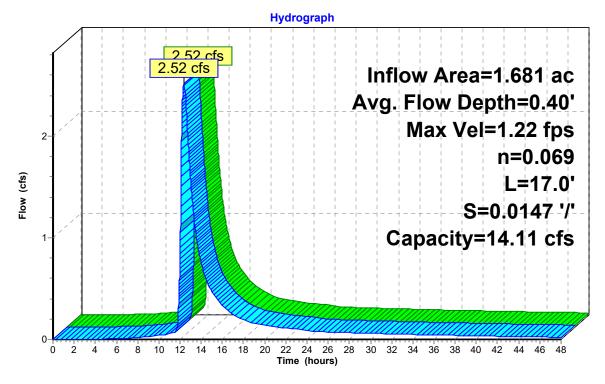
4.00' x 1.00' deep channel, n= 0.069 Riprap, 6-inch Side Slope Z-value= 3.0 '/' Top Width= 10.00'

Length= 17.0' Slope= 0.0147 '/'

Inlet Invert= 45.25', Outlet Invert= 45.00'



Reach 69R: Swale



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Reach 70R: Woodland Flow Path

Inflow Area = 1.681 ac, 63.59% Impervious, Inflow Depth > 4.00" for 10-year event

Inflow = 2.52 cfs @ 12.38 hrs, Volume= 0.560 af

Outflow = 2.52 cfs @ 12.40 hrs, Volume= 0.560 af, Atten= 0%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 0.57 fps, Min. Travel Time= 1.7 min Avg. Velocity = 0.16 fps, Avg. Travel Time= 5.8 min

Peak Storage= 250 cf @ 12.40 hrs Average Depth at Peak Storage= 0.33'

Bank-Full Depth= 1.00' Flow Area= 20.0 sf, Capacity= 21.20 cfs

10.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

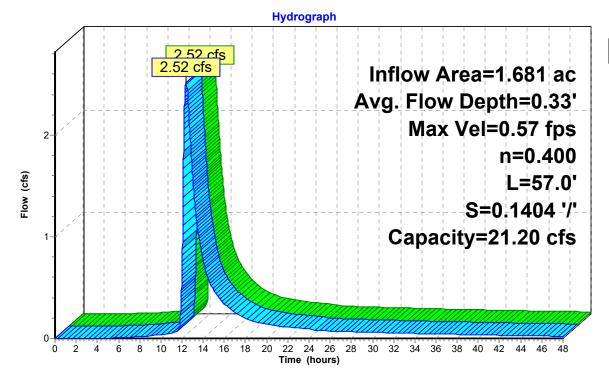
Side Slope Z-value= 10.0 '/' Top Width= 30.00'

Length= 57.0' Slope= 0.1404 '/'

Inlet Invert= 45.00', Outlet Invert= 37.00'



Reach 70R: Woodland Flow Path



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Reach 71R: Stream

Inflow Area = 1.681 ac, 63.59% Impervious, Inflow Depth > 4.00" for 10-year event

Inflow = 2.52 cfs @ 12.40 hrs, Volume= 0.560 af

Outflow = 2.52 cfs @ 12.41 hrs, Volume= 0.560 af, Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.12 fps, Min. Travel Time= 0.4 min Avg. Velocity = 0.93 fps, Avg. Travel Time= 1.4 min

Peak Storage= 65 cf @ 12.41 hrs Average Depth at Peak Storage= 0.28' Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 31.12 cfs

2.00' x 1.00' deep channel, n= 0.040 Winding stream, pools & shoals

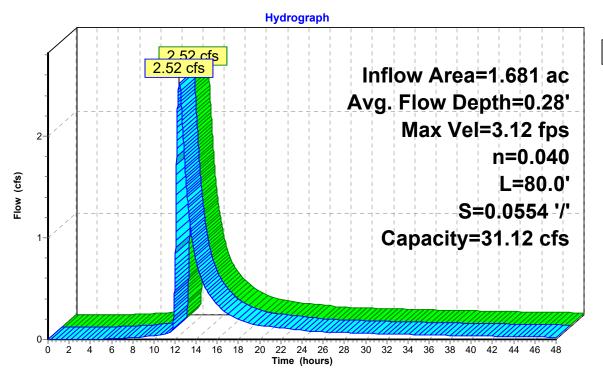
Side Slope Z-value= 3.0 '/' Top Width= 8.00'

Length= 80.0' Slope= 0.0554 '/'

Inlet Invert= 37.00', Outlet Invert= 32.57'



Reach 71R: Stream



Summary for Reach 72R: Stream

Inflow Area = 1.681 ac, 63.59% Impervious, Inflow Depth > 4.00" for 10-year event

Inflow = 2.52 cfs @ 12.41 hrs, Volume= 0.560 af

Outflow = 2.52 cfs @ 12.42 hrs, Volume= 0.559 af, Atten= 0%, Lag= 0.6 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.81 fps, Min. Travel Time= 0.8 min Avg. Velocity = 0.51 fps, Avg. Travel Time= 2.9 min

Peak Storage= 122 cf @ 12.42 hrs Average Depth at Peak Storage= 0.29

Bank-Full Depth= 1.00' Flow Area= 7.0 sf, Capacity= 25.40 cfs

4.00' x 1.00' deep channel, n= 0.040 Winding stream, pools & shoals

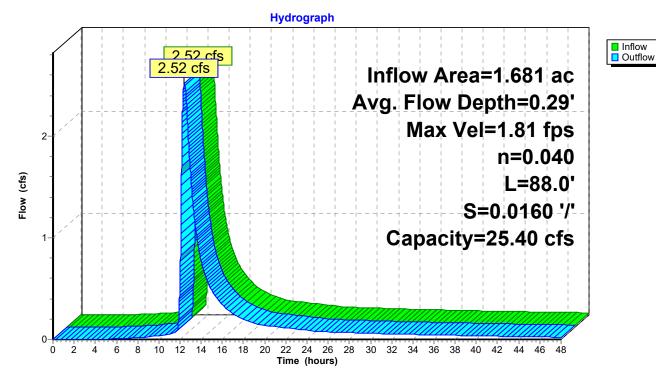
Side Slope Z-value= 3.0 '/' Top Width= 10.00'

Length= 88.0' Slope= 0.0160 '/'

Inlet Invert= 32.57', Outlet Invert= 31.16'



Reach 72R: Stream



HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Reach 73R: Stream

Inflow Area = 2.013 ac, 65.70% Impervious, Inflow Depth > 4.09" for 10-year event

Inflow = 3.01 cfs @ 12.42 hrs, Volume= 0.686 af

Outflow = 3.00 cfs @ 12.45 hrs, Volume= 0.685 af, Atten= 0%, Lag= 1.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.92 fps, Min. Travel Time= 2.7 min Avg. Velocity = 0.54 fps, Avg. Travel Time= 9.5 min

Peak Storage= 483 cf @ 12.45 hrs Average Depth at Peak Storage= 0.32'

Bank-Full Depth= 1.00' Flow Area= 7.0 sf, Capacity= 25.38 cfs

4.00' x 1.00' deep channel, n= 0.040 Winding stream, pools & shoals

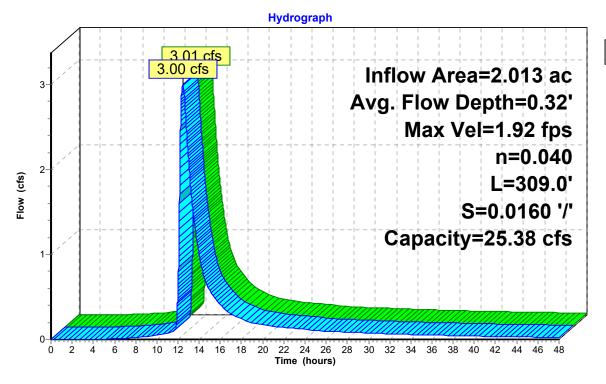
Side Slope Z-value= 3.0 '/' Top Width= 10.00'

Length= 309.0' Slope= 0.0160 '/'

Inlet Invert= 31.16', Outlet Invert= 26.22'



Reach 73R: Stream



Summary for Reach 83R: Woodland Flow Path

Inflow Area = 0.333 ac, 76.36% Impervious, Inflow Depth > 4.56" for 10-year event

Inflow = 0.50 cfs @ 12.40 hrs, Volume= 0.127 af

Outflow = 0.49 cfs @ 12.49 hrs, Volume= 0.126 af, Atten= 1%, Lag= 5.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 0.29 fps, Min. Travel Time= 6.9 min Avg. Velocity = 0.09 fps, Avg. Travel Time= 23.6 min

Peak Storage= 205 cf @ 12.49 hrs Average Depth at Peak Storage= 0.15'

Bank-Full Depth= 1.00' Flow Area= 20.0 sf, Capacity= 17.44 cfs

10.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

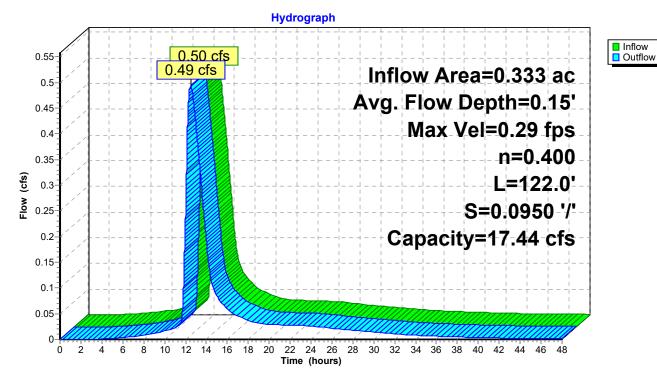
Side Slope Z-value= 10.0 '/' Top Width= 30.00'

Length= 122.0' Slope= 0.0950 '/'

Inlet Invert= 42.75', Outlet Invert= 31.16'



Reach 83R: Woodland Flow Path



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 1P: 15" RCP

Inflow Area = 1.746 ac, 13.99% Impervious, Inflow Depth > 2.95" for 10-year event

4.64 cfs @ 12.14 hrs, Volume= Inflow 0.430 af

3.78 cfs @ 12.22 hrs, Volume= Outflow = 0.430 af, Atten= 18%, Lag= 4.9 min

3.78 cfs @ 12.22 hrs, Volume= Primary 0.430 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 53.53' @ 12.22 hrs Surf.Area= 2,365 sf Storage= 682 cf

Plug-Flow detention time= 1.0 min calculated for 0.430 af (100% of inflow)

Center-of-Mass det. time= 1.0 min (874.1 - 873.1)

Volume	Invert	Avail.Storage	Storage Description
#1	52.26'	9,237 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

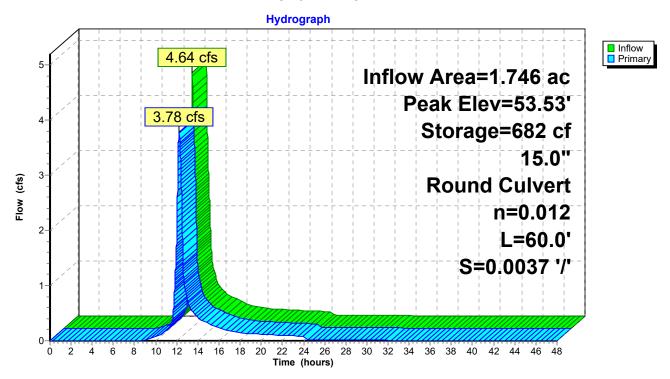
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
52.26	10	0	0
53.00	85	35	35
54.00	4,401	2,243	2,278
55.00	9,516	6,959	9,237

Device	Routing	Invert	Outlet Devices
#1	Primary	52.26'	15.0" Round Culvert

L= 60.0' RCP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 52.26' / 52.04' S= 0.0037 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=3.78 cfs @ 12.22 hrs HW=53.53' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.78 cfs @ 3.78 fps)

Pond 1P: 15" RCP



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 2.1P: 24" RCP

Inflow Area = 17.092 ac, 9.01% Impervious, Inflow Depth = 2.71" for 10-year event

Inflow = 35.88 cfs @ 12.18 hrs, Volume= 3.867 af

Outflow = 18.94 cfs @ 12.54 hrs, Volume= 3.867 af, Atten= 47%, Lag= 21.7 min

Primary = 18.94 cfs @ 12.54 hrs, Volume= 3.867 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 51.40' @ 12.51 hrs Surf.Area= 21,429 sf Storage= 24,417 cf

Plug-Flow detention time= 7.9 min calculated for 3.866 af (100% of inflow) Center-of-Mass det. time= 7.9 min (870.7 - 862.8)

Volume Invert Avail.Storage Storage Description

#1 47.57' 80,071 cf Custom Stage Data (Prismatic)Listed below (Recalc)

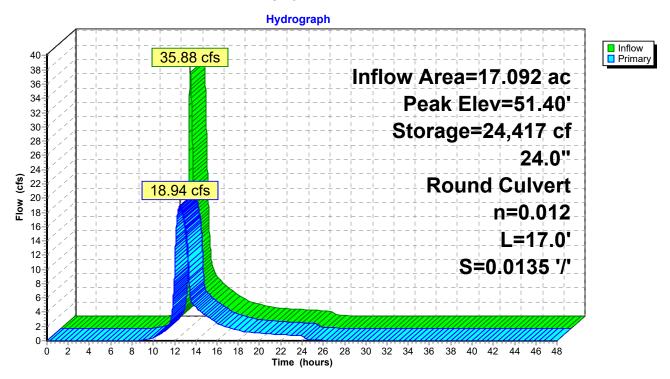
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
47.57	100	0	0
48.00	164	57	57
49.00	2,506	1,335	1,392
50.00	7,196	4,851	6,243
51.00	14,807	11,002	17,244
52.00	31,534	23,171	40,415
53.00	47,779	39,657	80,071

Device	Routing	Invert	Outlet Devices	
#1	Primary	47 57'	24 0" Pound Culve	rt

L= 17.0' RCP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 47.57' / 47.34' S= 0.0135 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

Primary OutFlow Max=18.42 cfs @ 12.54 hrs HW=51.39' TW=49.91' (Dynamic Tailwater) 1=Culvert (Inlet Controls 18.42 cfs @ 5.86 fps)

Pond 2.1P: 24" RCP



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 2P: 12" CPP

Inflow Area = 6.525 ac, 6.78% Impervious, Inflow Depth = 2.57" for 10-year event

Inflow = 23.92 cfs @ 12.00 hrs, Volume= 1.395 af

Outflow = 18.70 cfs @ 12.05 hrs, Volume= 1.395 af, Atten= 22%, Lag= 2.6 min

Primary = 18.70 cfs @ 12.05 hrs, Volume= 1.395 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 60.93' @ 12.05 hrs Surf.Area= 15,490 sf Storage= 11,719 cf

Plug-Flow detention time= 44.2 min calculated for 1.395 af (100% of inflow)

Center-of-Mass det. time= 44.1 min (876.6 - 832.4)

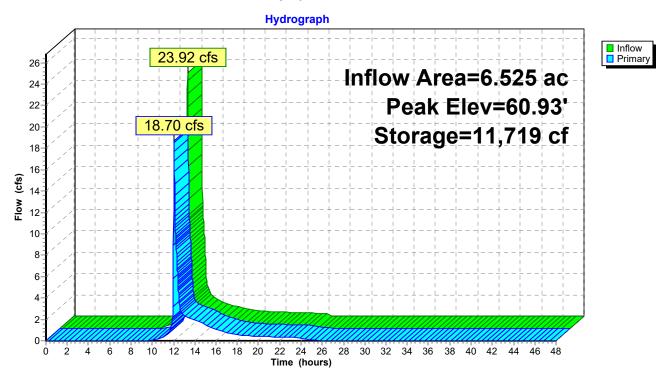
Volume	Inve	ert Avail.Sto	rage Sto	orage Description	
#1	59.6	35,7	64 cf Cu	stom Stage Data (P	rismatic)Listed below (Recalc)
Elevation Surf.Area		Inc.Sto	re Cum.Store		
(fee	et)	(sq-ft)	(cubic-fee	et) (cubic-feet)	
59.0	65	100		0 0	
60.0	00	6,996	1,24	1,242	
61.0	00	16,111	11,58	54 12,795	
62.0	00	29,827	22,96	35,764	
Device	Routing	Invert	Outlet D	evices	
#1	Primary	59.65'	12.0" R	ound Culvert	
			L= 9.0'	CPP, projecting, no	headwall, Ke= 0.900
			Inlet / Ou	utlet Invert= 59.65' / \$	59.31' S= 0.0378 '/' Cc= 0.900
				2, Flow Area= 0.79 s	
#2	Primary	60.75'		•	oad-Crested Rectangular Weir
			`	,	0.80 1.00 1.20 1.40 1.60 1.80 2.00
				0 3.50 4.00 4.50 5	
			,	0 /	.69 2.68 2.67 2.67 2.65 2.66 2.66
			2.68 2.7	'2 2.73 2.76 2.79 <i>2</i>	2.88 3.07 3.32

Primary OutFlow Max=18.66 cfs @ 12.05 hrs HW=60.93' TW=60.13' (Dynamic Tailwater)

1=Culvert (Inlet Controls 2.64 cfs @ 3.36 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 16.02 cfs @ 1.01 fps)

Pond 2P: 12" CPP



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 10P: Porous Pavement

Inflow Area = 0.234 ac, 98.98% Impervious, Inflow Depth > 5.20" for 10-year event

Inflow 0.09 cfs @ 21.94 hrs, Volume= 0.101 af

0.04 cfs @ 30.54 hrs, Volume= Outflow 0.097 af, Atten= 59%, Lag= 515.6 min

0.04 cfs @ 30.54 hrs, Volume= 0.097 af Discarded = Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 57.53' @ 30.54 hrs Surf.Area= 2,761 sf Storage= 1,632 cf Flood Elev= 59.55' Surf.Area= 2,761 sf Storage= 2,808 cf

Plug-Flow detention time= 434.2 min calculated for 0.097 af (95% of inflow)

Center-of-Mass det. time= 386.3 min (1,842.4 - 1,456.2)

<u>Volume</u>	Invert	Ava	il.Storage	Storage Descrip	tion		
#1	56.05'		3,513 cf	Custom Stage	Data (Conic)Listed	below	
Elevation	on Su	ırf.Area	Voids	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	(sq-ft)	
56.0)5	2,761	0.0	0	0	2,761	
57.3	30	2,761	40.0	1,381	1,381	2,994	
57.5	55	2,761	40.0	276	1,657	3,040	
58.5	55	2,761	5.0	138	1,795	3,227	
59.2	22	2,761	40.0	740	2,535	3,351	
59.5		2,761	30.0	273	2,808	3,413	
60.0	00	624	100.0	705	3,513	5,551	
Device	Routing	In	vert Out	let Devices			
#1	Discarded	56	6.05' 0.5 0	00 in/hr Exfiltration	on over Wetted are	a Phase-In= 0.01'	
#2	Secondary	56	6.34' 12.0	" Round Culver	t		
				• • • • • • • • • • • • • • • • • • •	e edge headwall, 🧍		
			Inle	t / Outlet Invert= 5	6.34' / 56.25' S = 0	0.0050 '/' Cc= 0.900	

n= 0.012, Flow Area= 0.79 sf

Limited to weir flow at low heads

24.0" x 24.0" Horiz. Orifice/Grate C= 0.600

Discarded OutFlow Max=0.04 cfs @ 30.54 hrs HW=57.53' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

59.55'

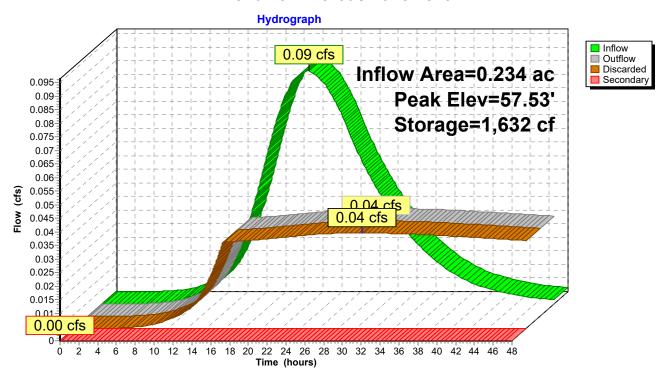
Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=56.05' TW=56.25' (Dynamic Tailwater)

#3

Device 2

-2=Culvert (Controls 0.00 cfs)
-3=Orifice/Grate (Controls 0.00 cfs)

Pond 10P: Porous Pavement



Summary for Pond 11P: YD #3-1

Inflow Area = 0.013 ac, 65.82% Impervious, Inflow Depth = 4.33" for 10-year event

Inflow = 0.07 cfs @ 12.08 hrs, Volume= 0.005 af

Outflow = 0.07 cfs @ 12.08 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Primary = 0.07 cfs @ 12.08 hrs, Volume= 0.005 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

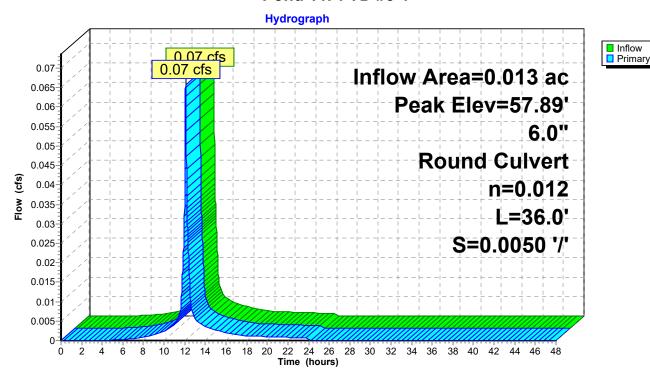
Peak Elev= 57.89' @ 12.43 hrs

Flood Elev= 60.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	57.16'	6.0" Round Culvert
			L= 36.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 57.16' / 56.98' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.20 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=57.59' TW=57.60' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 11P: YD #3-1



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 12P: YD #7

Inflow Area = 0.043 ac, 65.12% Impervious, Inflow Depth = 4.33" for 10-year event

Inflow = 0.21 cfs @ 12.08 hrs, Volume= 0.016 af

Outflow = 0.21 cfs @ 12.08 hrs, Volume= 0.016 af, Atten= 0%, Lag= 0.0 min

Primary = 0.21 cfs @ 12.08 hrs, Volume= 0.016 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

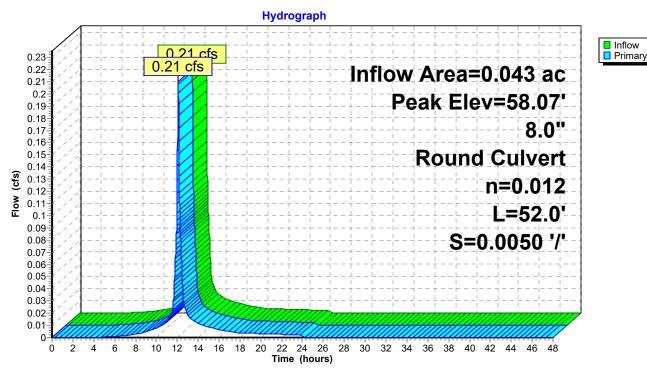
Peak Elev= 58.07' @ 12.10 hrs

Flood Elev= 59.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	57.76'	8.0" Round Culvert
	-		L= 52.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 57.76' / 57.50' S= 0.0050 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.20 cfs @ 12.08 hrs HW=58.07' TW=57.79' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.20 cfs @ 1.91 fps)

Pond 12P: YD #7



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 13P: YD #6

Inflow Area = 0.074 ac, 68.76% Impervious, Inflow Depth = 4.42" for 10-year event

Inflow = 0.37 cfs @ 12.08 hrs, Volume= 0.027 af

Outflow = 0.37 cfs @ 12.08 hrs, Volume= 0.027 af, Atten= 0%, Lag= 0.0 min

Primary = 0.37 cfs @ 12.08 hrs, Volume= 0.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

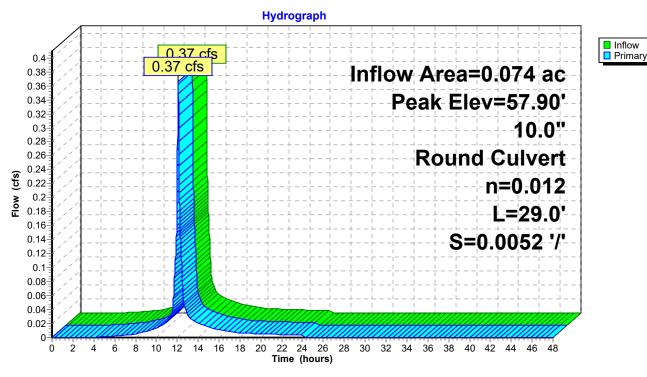
Peak Elev= 57.90' @ 12.42 hrs

Flood Elev= 60.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	57.33'	10.0" Round Culvert
			L= 29.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 57.33' / 57.18' S= 0.0052 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.55 sf

Primary OutFlow Max=0.33 cfs @ 12.08 hrs HW=57.79' TW=57.69' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.33 cfs @ 1.56 fps)

Pond 13P: YD #6



Summary for Pond 14P: YD #5

Inflow Area = 0.098 ac, 70.95% Impervious, Inflow Depth = 4.48" for 10-year event

Inflow = 0.49 cfs @ 12.08 hrs, Volume= 0.036 af

Outflow = 0.49 cfs @ 12.08 hrs, Volume= 0.036 af, Atten= 0%, Lag= 0.0 min

Primary = 0.49 cfs @ 12.08 hrs, Volume= 0.036 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

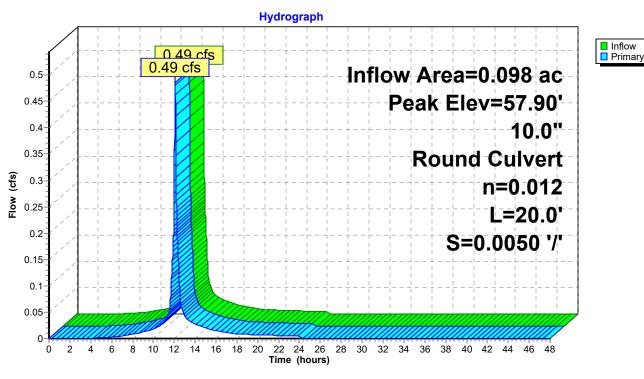
Peak Elev= 57.90' @ 12.42 hrs

Flood Elev= 60.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	57.08'	10.0" Round Culvert
			L= 20.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 57.08' / 56.98' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.55 sf

Primary OutFlow Max=0.40 cfs @ 12.08 hrs HW=57.69' TW=57.63' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.40 cfs @ 1.31 fps)

Pond 14P: YD #5



Summary for Pond 15P: YD #4

Inflow Area = 0.129 ac, 64.17% Impervious, Inflow Depth = 4.29" for 10-year event

Inflow = 0.62 cfs @ 12.08 hrs, Volume= 0.046 af

Outflow = 0.62 cfs @ 12.08 hrs, Volume= 0.046 af, Atten= 0%, Lag= 0.0 min

Primary = 0.62 cfs @ 12.08 hrs, Volume= 0.046 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

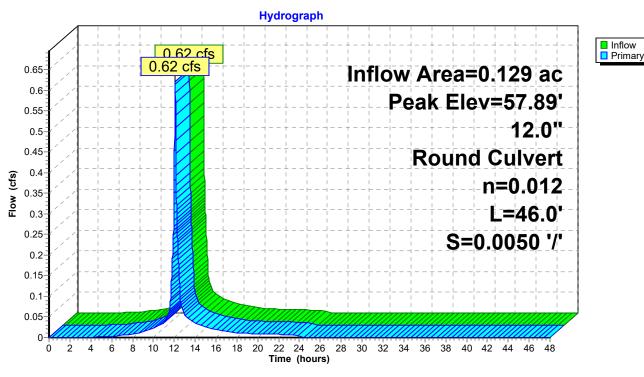
Peak Elev= 57.89' @ 12.42 hrs

Flood Elev= 60.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	56.81'	12.0" Round Culvert
			L= 46.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 56.81' / 56.58' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.44 cfs @ 12.08 hrs HW=57.63' TW=57.60' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.44 cfs @ 0.87 fps)

Pond 15P: YD #4



Summary for Pond 16P: CB #3

Inflow Area = 0.164 ac, 59.41% Impervious, Inflow Depth = 4.17" for 10-year event

Inflow = 0.77 cfs @ 12.09 hrs, Volume= 0.057 af

Outflow = 0.77 cfs @ 12.09 hrs, Volume= 0.057 af, Atten= 0%, Lag= 0.0 min

Primary = 0.77 cfs @ 12.09 hrs, Volume= 0.057 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

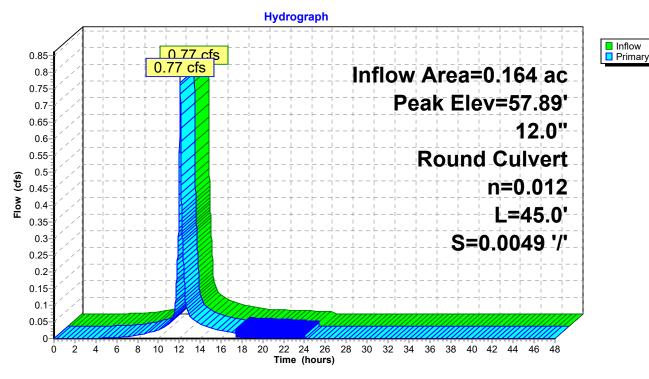
Peak Elev= 57.89' @ 12.42 hrs

Flood Elev= 61.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	56.48'	12.0" Round Culvert
			L= 45.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 56.48' / 56.26' S= 0.0049 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.45 cfs @ 12.09 hrs HW=57.60' TW=57.59' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.45 cfs @ 0.63 fps)

Pond 16P: CB #3



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Printed 10/6/2025

Summary for Pond 17P: Bio #1 / OS #1

Inflow Area = 0.230 ac, 42.36% Impervious, Inflow Depth = 3.76" for 10-year event

Inflow 0.98 cfs @ 12.09 hrs. Volume= 0.072 af

0.30 cfs @ 12.42 hrs, Volume= Outflow = 0.072 af, Atten= 70%, Lag= 19.8 min

0.30 cfs @ 12.42 hrs, Volume= Primary 0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 56.25' Surf.Area= 487 sf Storage= 37 cf

Peak Elev= 57.89' @ 12.42 hrs Surf.Area= 1,182 sf Storage= 1,367 cf (1,331 cf above start)

Flood Elev= 59.25' Surf.Area= 1,950 sf Storage= 3,480 cf (3,444 cf above start)

Plug-Flow detention time= 283.6 min calculated for 0.071 af (98% of inflow)

Center-of-Mass det. time= 266.2 min (1,065.8 - 799.6)

Volume	Inver	t Ava	il.Storage	e Storage Descri	iption		
#1	54.75	5'	3,480 c	f Custom Stage	Data (Conic)Listed	l below (Recalc)	
□ 14:			\	lus a Ottama	Orana Otama	\M_+4_A_==	
Elevation		Surf.Area	Voids	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
54.	75	487	0.0	0	0	487	
56.2	25	487	5.0	37	37	604	
57.0	00	774	100.0	469	505	899	
58.0	00	1,239	100.0	997	1,503	1,377	
59.0		1,797	100.0	1,509	3,012	1,952	
59.2		1,950	100.0	468	3,480	2,110	
		,,,,,,,			2, 122	_,	
Device	Routing	In	vert O	utlet Devices			
#1	Primary	53	3.75' 15	5.0" Round Culve	ert		
	,		L=	: 30.0' CPP. squa	are edge headwall, I	Ke= 0.500	
						0.0050 '/' Cc= 0.900	
				0.012, Flow Area			
#2	Device 1	53		0" Vert. Underdra			
#3	Device 2		-			over Wetted area abo	ve 56 25'
110	D01100 2	00			ea = 604 sf Phase-l		VC 00.20
#4	Device 1	57		0" Vert. Orifice/G		11- 0.01	
#5	Device 1				Orifice/Grate C= (n 600	
π0	Device 1	50		mited to weir flow a		3.000	

Primary OutFlow Max=0.30 cfs @ 12.42 hrs HW=57.89' TW=53.71' (Dynamic Tailwater)

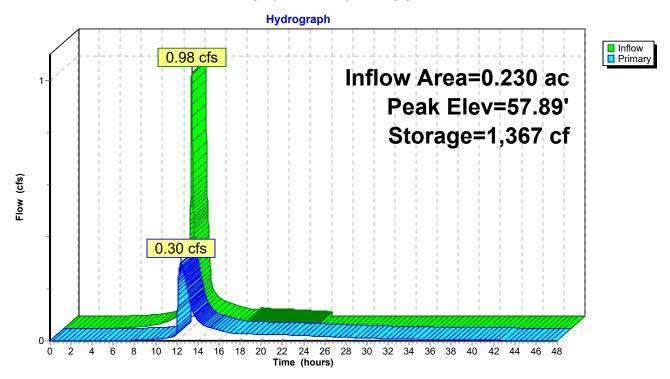
-1=Culvert (Passes 0.30 cfs of 11.08 cfs potential flow)

⁻²⁼Underdrain (Passes 0.04 cfs of 0.84 cfs potential flow)
-3=Exfiltration throiugh Media (Exfiltration Controls 0.04 cfs)

⁻⁴⁼Orifice/Grate (Orifice Controls 0.26 cfs @ 2.93 fps)

⁻⁵⁼Orifice/Grate (Controls 0.00 cfs)

Pond 17P: Bio #1 / OS #1



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 21P: CB #8

Inflow Area = 0.060 ac,100.00% Impervious, Inflow Depth = 5.23" for 10-year event

Inflow = 0.32 cfs @ 12.08 hrs, Volume= 0.026 af

Outflow = 0.32 cfs @ 12.08 hrs, Volume= 0.026 af, Atten= 0%, Lag= 0.0 min

Primary = 0.32 cfs @ 12.08 hrs, Volume= 0.026 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

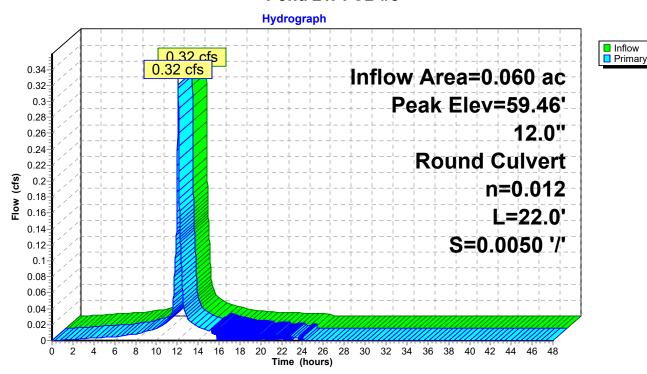
Peak Elev= 59.46' @ 12.47 hrs

Flood Elev= 60.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	57.86'	12.0" Round Culvert
	•		L= 22.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 57.86' / 57.75' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=59.12' TW=59.14' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 21P: CB #8



Printed 10/6/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 22P: Bio #2 / OS #9

Inflow Area = 0.361 ac, 74.83% Impervious, Inflow Depth = 4.61" for 10-year event

Inflow 1.74 cfs @ 12.08 hrs, Volume= 0.139 af

0.44 cfs @ 12.46 hrs, Volume= Outflow = 0.137 af, Atten= 75%, Lag= 22.4 min

0.44 cfs @ 12.46 hrs, Volume= Primary 0.137 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 57.75' Surf.Area= 1,027 sf Storage= 77 cf

Peak Elev= 59.46' @ 12.46 hrs Surf.Area= 2,116 sf Storage= 2,731 cf (2,654 cf above start)

Flood Elev= 60.00' Surf.Area= 2,511 sf Storage= 3,983 cf (3,906 cf above start)

Plug-Flow detention time= 262.9 min calculated for 0.136 af (98% of inflow)

Center-of-Mass det. time= 240.1 min (999.6 - 759.5)

Volume	Inve	rt Ava	il.Storage	e Storage Descrip	tion		
#1	56.2	5'	3,983 c	f Custom Stage I	Data (Conic)Listed	below (Recalc)	
Elevation		Surf.Area	Voids	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
56.2	25	1,027	0.0	0	0	1,027	
57.7	75	1,027	5.0	77	77	1,197	
58.0	00	1,175	100.0	275	352	1,348	
59.0	00	1,809	100.0	1,481	1,833	1,997	
60.0	00	2,511	100.0	2,150	3,983	2,718	
Device	Routing	In	vert Oı	ıtlet Devices			
#1	Primary	55	5.25' 15	.0" Round Culvert	t		
	•		L=	18.0' CPP, square	e edge headwall, K	e= 0.500	
			Inl	et / Outlet Invert= 5	5.25' / 55.16' S= 0	.0050 '/' Cc= 0.900	
			n=	0.012, Flow Area=	= 1.23 sf		
#2	Device 1	55	5.25' 4. 0)" Vert. Underdrair	n C= 0.600		
#3	Device 2	57	'.75' 2.	500 in/hr Exfiltration	on through Media o	ver Wetted area above	57.75
			Ex	cluded Wetted area	a = 1,197 sf Phase-	In= 0.01'	
#4	Device 1	58	3.50' 4. 0)" Vert. Orifice/Gra	te C= 0.600		
#5	Device 1	59		. 0" x 24.0" Horiz. C mited to weir flow at		.600	

Primary OutFlow Max=0.44 cfs @ 12.46 hrs HW=59.46' TW=55.21' (Dynamic Tailwater)

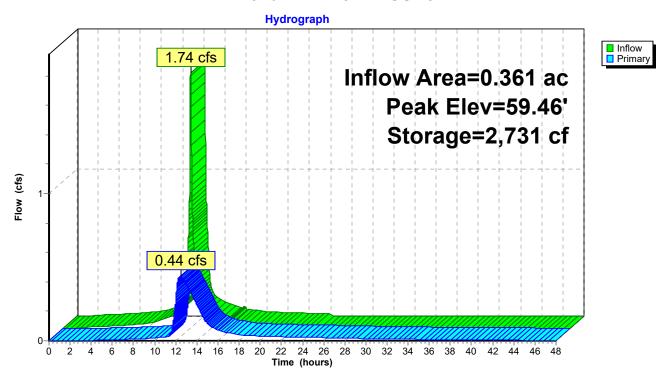
1=Culvert (Passes 0.44 cfs of 11.19 cfs potential flow)

⁻²⁼Underdrain (Passes 0.06 cfs of 0.84 cfs potential flow) **-3=Exfiltration through Media** (Exfiltration Controls 0.06 cfs)

⁻⁴⁼Orifice/Grate (Orifice Controls 0.37 cfs @ 4.28 fps)

⁻⁵⁼Orifice/Grate (Controls 0.00 cfs)

Pond 22P: Bio #2 / OS #9



5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 30P: CB #10-2

Inflow Area = 0.189 ac, 53.10% Impervious, Inflow Depth = 4.01" for 10-year event

Inflow = 0.87 cfs @ 12.09 hrs, Volume= 0.063 af

Outflow = 0.87 cfs @ 12.09 hrs, Volume= 0.063 af, Atten= 0%, Lag= 0.0 min

Primary = 0.87 cfs @ 12.09 hrs, Volume= 0.063 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

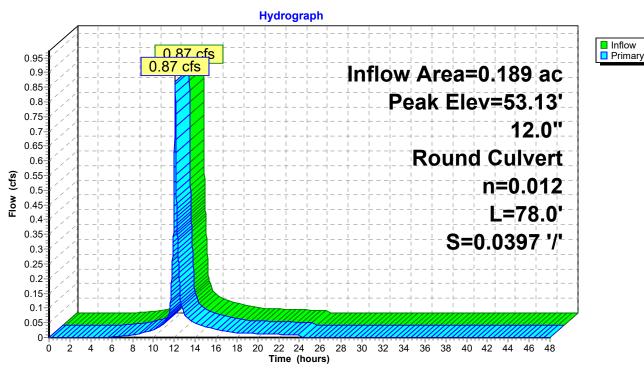
Peak Elev= 53.13' @ 12.09 hrs

Flood Elev= 56.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	52.65'	12.0" Round Culvert
			L= 78.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 52.65' / 49.55' S= 0.0397 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.87 cfs @ 12.09 hrs HW=53.13' TW=49.22' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.87 cfs @ 2.35 fps)

Pond 30P: CB #10-2



Summary for Pond 31P: CB #10-1

Inflow Area = 0.164 ac, 71.11% Impervious, Inflow Depth = 4.44" for 10-year event

Inflow = 0.81 cfs @ 12.08 hrs, Volume= 0.061 af

Outflow = 0.81 cfs @ 12.08 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min

Primary = 0.81 cfs @ 12.08 hrs, Volume= 0.061 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

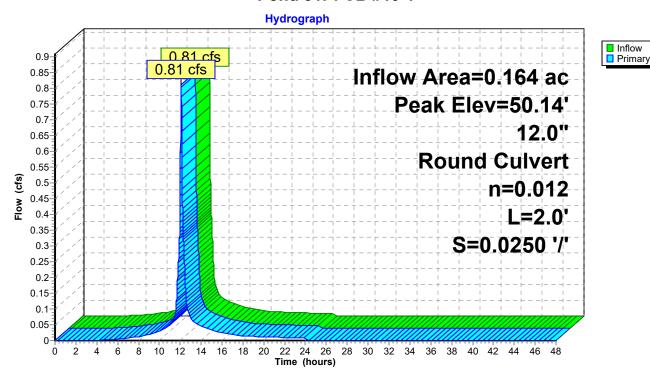
Peak Elev= 50.14' @ 12.08 hrs

Flood Elev= 53.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	49.60'	12.0" Round Culvert
	-		L= 2.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.60' / 49.55' S= 0.0250 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.81 cfs @ 12.08 hrs HW=50.13' TW=49.21' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.81 cfs @ 2.75 fps)

Pond 31P: CB #10-1



Summary for Pond 32P: DMH #10

Inflow Area = 17.445 ac, 10.07% Impervious, Inflow Depth > 2.74" for 10-year event

Inflow = 19.28 cfs @ 12.48 hrs, Volume= 3.990 af

Outflow = 19.28 cfs @ 12.48 hrs, Volume= 3.990 af, Atten= 0%, Lag= 0.0 min

Primary = 19.28 cfs @ 12.48 hrs, Volume= 3.990 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

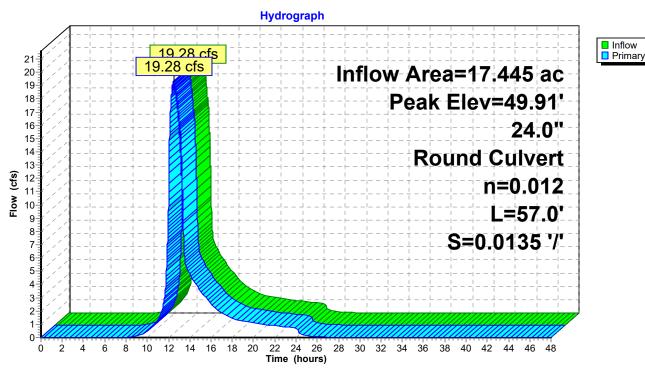
Peak Elev= 49.91' @ 12.48 hrs

Flood Elev= 53.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	47.29'	24.0" Round Culvert
	-		L= 57.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 47.29' / 46.52' S= 0.0135 '/' Cc= 0.900
			n= 0.012. Flow Area= 3.14 sf

Primary OutFlow Max=19.28 cfs @ 12.48 hrs HW=49.91' TW=47.33' (Dynamic Tailwater) 1=Culvert (Inlet Controls 19.28 cfs @ 6.14 fps)

Pond 32P: DMH #10



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 40P: 6" CPP

Inflow Area = 0.070 ac,100.00% Impervious, Inflow Depth = 5.23" for 10-year event

Inflow = 0.38 cfs @ 12.08 hrs, Volume= 0.031 af

Outflow = 0.38 cfs @ 12.08 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.0 min

Primary = 0.38 cfs @ 12.08 hrs, Volume= 0.031 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

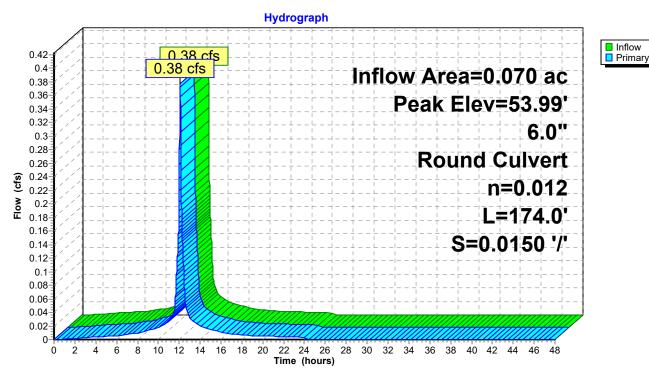
Peak Elev= 53.99' @ 12.08 hrs

Flood Elev= 56.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	53.58'	6.0" Round Culvert
			L= 174.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 53.58' / 50.97' S= 0.0150 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.20 sf

Primary OutFlow Max=0.38 cfs @ 12.08 hrs HW=53.99' TW=52.05' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.38 cfs @ 2.18 fps)

Pond 40P: 6" CPP



Summary for Pond 41P: YD #15

Inflow Area = 0.119 ac, 96.22% Impervious, Inflow Depth = 5.14" for 10-year event

Inflow = 0.63 cfs @ 12.08 hrs, Volume= 0.051 af

Outflow = 0.63 cfs @ 12.08 hrs, Volume= 0.051 af, Atten= 0%, Lag= 0.0 min

Primary = 0.63 cfs @ 12.08 hrs, Volume= 0.051 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

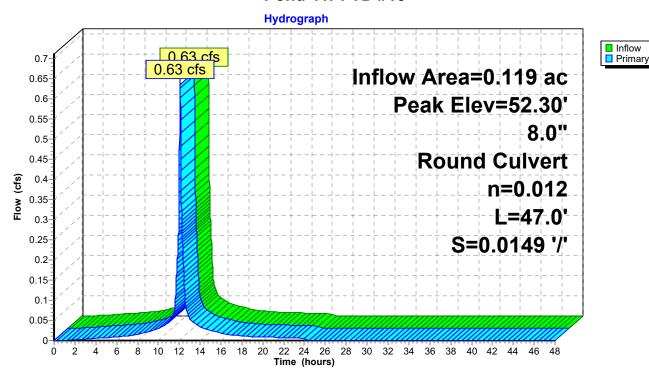
Peak Elev= 52.30' @ 12.12 hrs

Flood Elev= 55.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	50.80'	8.0" Round Culvert
			L= 47.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 50.80' / 50.10' S= 0.0149 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.40 cfs @ 12.08 hrs HW=52.05' TW=51.97' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.40 cfs @ 1.15 fps)

Pond 41P: YD #15



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 42P: YD #14-1

Inflow Area = 0.076 ac, 74.41% Impervious, Inflow Depth = 4.55" for 10-year event

Inflow = 0.38 cfs @ 12.08 hrs, Volume= 0.029 af

Outflow = 0.38 cfs @ 12.08 hrs, Volume= 0.029 af, Atten= 0%, Lag= 0.0 min

Primary = 0.38 cfs @ 12.08 hrs, Volume= 0.029 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

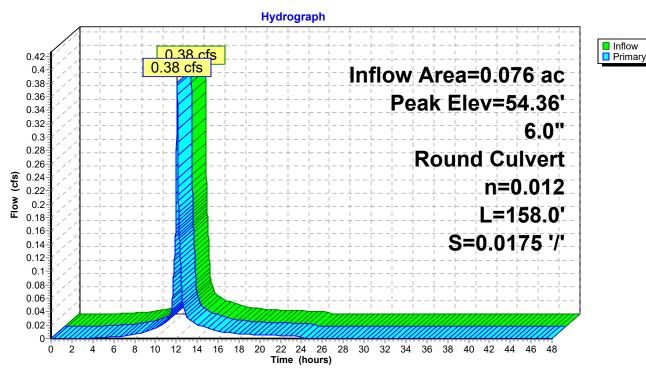
Peak Elev= 54.36' @ 12.08 hrs

Flood Elev= 56.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	53.95'	6.0" Round Culvert
			L= 158.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 53.95' / 51.19' S= 0.0175 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.20 sf

Primary OutFlow Max=0.38 cfs @ 12.08 hrs HW=54.36' TW=51.98' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.38 cfs @ 2.19 fps)

Pond 42P: YD #14-1



Summary for Pond 43P: 10" CPP

Inflow Area = 0.195 ac, 87.75% Impervious, Inflow Depth = 4.91" for 10-year event

Inflow = 1.02 cfs @ 12.08 hrs, Volume= 0.080 af

Outflow = 1.02 cfs @ 12.08 hrs, Volume= 0.080 af, Atten= 0%, Lag= 0.0 min

Primary = 1.02 cfs @ 12.08 hrs, Volume= 0.080 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

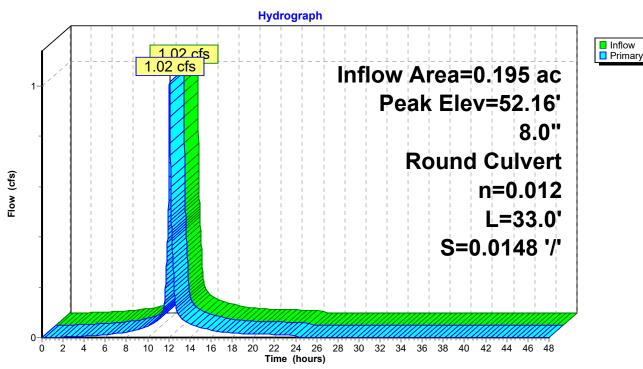
Peak Elev= 52.16' @ 12.12 hrs

Flood Elev= 56.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	50.10'	8.0" Round Culvert
			L= 33.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 50.10' / 49.61' S= 0.0148 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.35 sf

Primary OutFlow Max=0.89 cfs @ 12.08 hrs HW=51.98' TW=51.67' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.89 cfs @ 2.55 fps)

Pond 43P: 10" CPP



Summary for Pond 44P: CB #14

Inflow Area = 0.302 ac, 71.15% Impervious, Inflow Depth = 4.48" for 10-year event

Inflow = 1.48 cfs @ 12.08 hrs, Volume= 0.113 af

Outflow = 1.48 cfs @ 12.08 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min

Primary = 1.48 cfs @ 12.08 hrs, Volume= 0.113 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

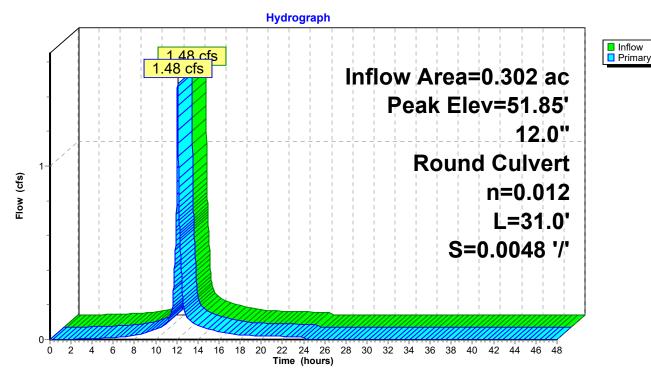
Peak Elev= 51.85' @ 12.13 hrs

Flood Elev= 54.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	49.05'	12.0" Round Culvert
			L= 31.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.05' / 48.90' S= 0.0048 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=1.05 cfs @ 12.08 hrs HW=51.68' TW=51.60' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.05 cfs @ 1.34 fps)

Pond 44P: CB #14



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 45P: CB #13-2

Inflow Area = 0.089 ac, 71.71% Impervious, Inflow Depth = 4.44" for 10-year event

Inflow = 0.44 cfs @ 12.08 hrs, Volume= 0.033 af

Outflow = 0.44 cfs @ 12.08 hrs, Volume= 0.033 af, Atten= 0%, Lag= 0.0 min

Primary = 0.44 cfs @ 12.08 hrs, Volume= 0.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

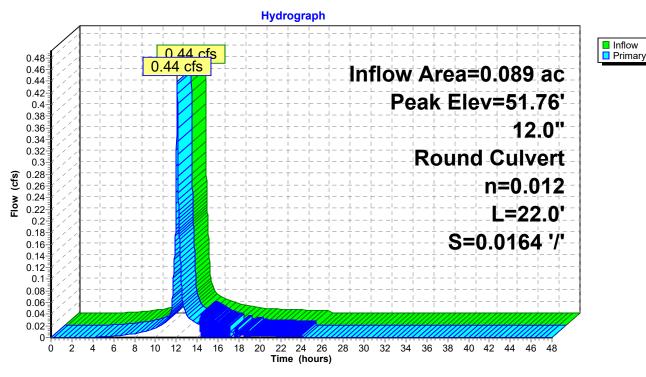
Peak Elev= 51.76' @ 12.34 hrs

Flood Elev= 54.65'

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Primary	49.25'	12.0" Round Culvert
	•		L= 22.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.25' / 48.89' S= 0.0164 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=51.54' TW=51.60' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 45P: CB #13-2



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 46P: CB #13-1

Inflow Area = 0.103 ac, 73.25% Impervious, Inflow Depth = 4.55" for 10-year event

Inflow = 0.52 cfs @ 12.08 hrs, Volume= 0.039 af

Outflow = 0.52 cfs @ 12.08 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.0 min

Primary = 0.52 cfs @ 12.08 hrs, Volume= 0.039 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

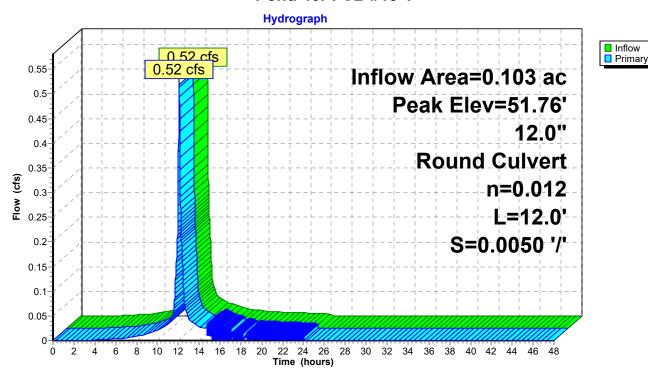
Peak Elev= 51.76' @ 12.14 hrs

Flood Elev= 54.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	48.95'	12.0" Round Culvert
	_		L= 12.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 48.95' / 48.89' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=51.55' TW=51.60' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 46P: CB #13-1



☐ Inflow☐ Primary

5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 47P: DMH #13

Inflow Area = 0.494 ac, 71.69% Impervious, Inflow Depth = 4.49" for 10-year event

Inflow = 2.43 cfs @ 12.08 hrs, Volume= 0.185 af

Outflow = 2.43 cfs @ 12.08 hrs, Volume= 0.185 af, Atten= 0%, Lag= 0.0 min

Primary = 2.43 cfs @ 12.08 hrs, Volume= 0.185 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

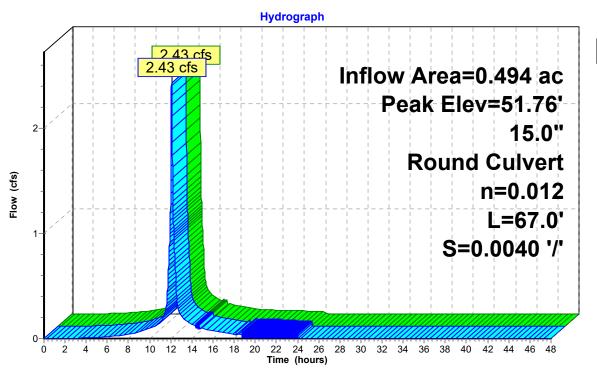
Peak Elev= 51.76' @ 12.33 hrs

Flood Elev= 54.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	48.79'	15.0" Round Culvert
			L= 67.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 48.79' / 48.52' S= 0.0040 '/' Cc= 0.900
			n= 0.012. Flow Area= 1.23 sf

Primary OutFlow Max=1.95 cfs @ 12.08 hrs HW=51.60' TW=51.49' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.95 cfs @ 1.59 fps)

Pond 47P: DMH #13



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 48P: CB #12-2

Inflow Area = 0.078 ac, 55.67% Impervious, Inflow Depth = 4.12" for 10-year event

Inflow = 0.37 cfs @ 12.09 hrs, Volume= 0.027 af

Outflow = 0.37 cfs @ 12.09 hrs, Volume= 0.027 af, Atten= 0%, Lag= 0.0 min

Primary = 0.37 cfs @ 12.09 hrs, Volume= 0.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

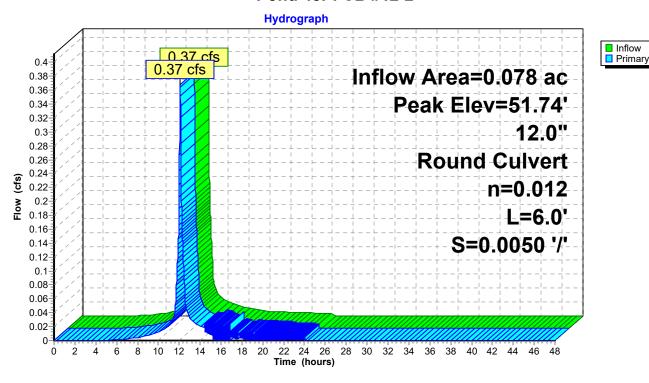
Peak Elev= 51.74' @ 12.35 hrs

Flood Elev= 52.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	48.70'	12.0" Round Culvert
			L= 6.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 48.70' / 48.67' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.09 hrs HW=51.45' TW=51.50' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 48P: CB #12-2



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 49P: CB #12-1

Inflow Area = 0.084 ac, 83.78% Impervious, Inflow Depth = 4.66" for 10-year event

Inflow = 0.43 cfs @ 12.08 hrs, Volume= 0.033 af

Outflow = 0.43 cfs @ 12.08 hrs, Volume= 0.033 af, Atten= 0%, Lag= 0.0 min

Primary = 0.43 cfs @ 12.08 hrs, Volume= 0.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

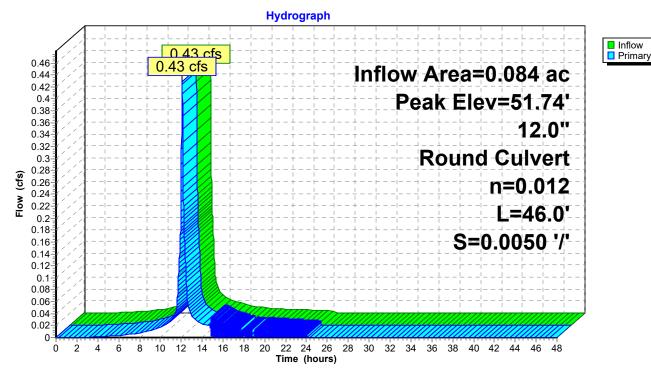
Peak Elev= 51.74' @ 12.35 hrs

Flood Elev= 52.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	48.90'	12.0" Round Culvert
	-		L= 46.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 48.90' / 48.67' S= 0.0050 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=51.44' TW=51.49' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 49P: CB #12-1



☐ Inflow☐ Primary

5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 50P: DMH #12

Inflow Area = 0.656 ac, 71.32% Impervious, Inflow Depth = 4.46" for 10-year event

Inflow = 3.23 cfs @ 12.08 hrs, Volume= 0.244 af

Outflow = 3.23 cfs @ 12.08 hrs, Volume= 0.244 af, Atten= 0%, Lag= 0.0 min

Primary = 3.23 cfs @ 12.08 hrs, Volume= 0.244 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

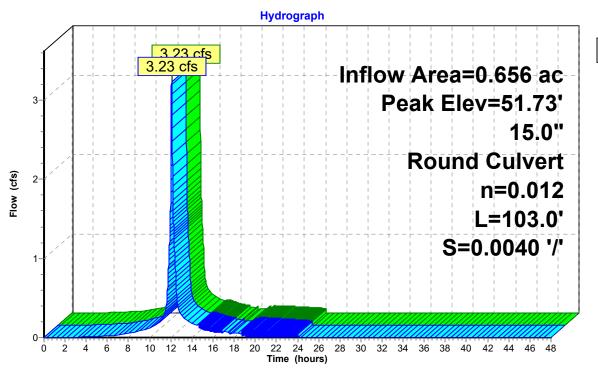
Peak Elev= 51.73' @ 12.34 hrs

Flood Elev= 54.00'

Device Rou	iting Invert	Outlet Devices
#1 Prin		15.0" Round Culvert L= 103.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 48.42' / 48.01' S= 0.0040 '/' Cc= 0.900 n= 0.012. Flow Area= 1.23 sf

Primary OutFlow Max=2.98 cfs @ 12.08 hrs HW=51.49' TW=51.17' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.98 cfs @ 2.43 fps)

Pond 50P: DMH #12



Summary for Pond 60P: CB #17-1-2

Inflow Area = 0.210 ac, 78.55% Impervious, Inflow Depth = 4.66" for 10-year event

Inflow = 1.07 cfs @ 12.08 hrs, Volume= 0.082 af

Outflow = 1.07 cfs @ 12.08 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min

Primary = 1.07 cfs @ 12.08 hrs, Volume= 0.082 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

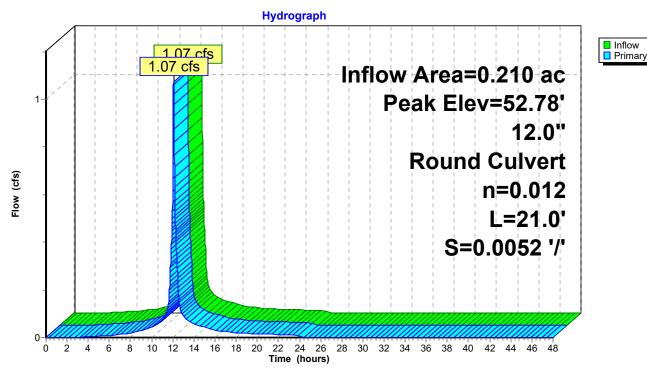
Peak Elev= 52.78' @ 12.12 hrs

Flood Elev= 54.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	50.40'	12.0" Round Culvert
			L= 21.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 50.40' / 50.29' S= 0.0052 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=52.50' TW=52.56' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 60P: CB #17-1-2



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 61P: CB #17-1-1

Inflow Area = 0.162 ac, 91.98% Impervious, Inflow Depth = 5.00" for 10-year event

Inflow = 0.86 cfs @ 12.08 hrs, Volume= 0.068 af

Outflow = 0.86 cfs @ 12.08 hrs, Volume= 0.068 af, Atten= 0%, Lag= 0.0 min

Primary = 0.86 cfs @ 12.08 hrs, Volume= 0.068 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

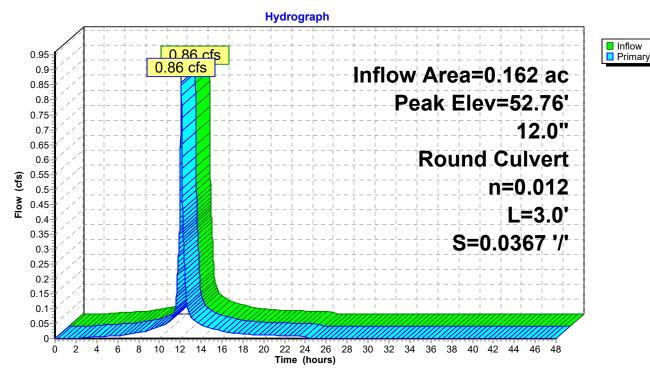
Peak Elev= 52.76' @ 12.12 hrs

Flood Elev= 54.90'

Device	Routing	Invert	Outlet Devices
#1	Primary	50.40'	12.0" Round Culvert
	-		L= 3.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 50.40' / 50.29' S= 0.0367 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=52.46' TW=52.55' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 61P: CB #17-1-1



Summary for Pond 62P: DMH #17-1

Inflow Area = 0.373 ac, 84.39% Impervious, Inflow Depth = 4.81" for 10-year event

Inflow = 1.93 cfs @ 12.08 hrs, Volume= 0.149 af

Outflow = 1.93 cfs @ 12.08 hrs, Volume= 0.149 af, Atten= 0%, Lag= 0.0 min

Primary = 1.93 cfs @ 12.08 hrs, Volume= 0.149 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

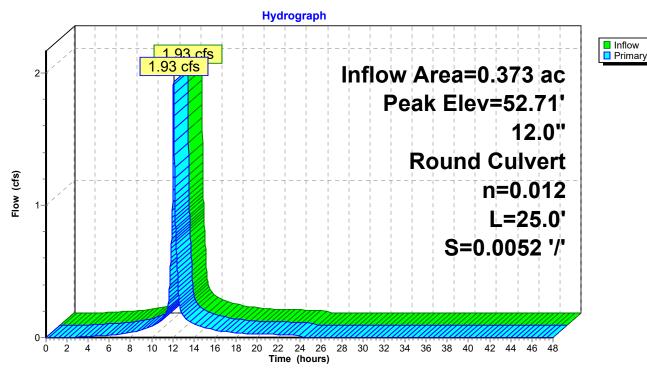
Peak Elev= 52.71' @ 12.11 hrs

Flood Elev= 54.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	50.19'	12.0" Round Culvert
			L= 25.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 50.19' / 50.06' S= 0.0052 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=1.52 cfs @ 12.08 hrs HW=52.56' TW=52.40' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.52 cfs @ 1.93 fps)

Pond 62P: DMH #17-1



Summary for Pond 63P: YD #21

Inflow Area = 0.033 ac, 65.52% Impervious, Inflow Depth = 4.33" for 10-year event

Inflow = 0.16 cfs @ 12.08 hrs, Volume= 0.012 af

Outflow = 0.16 cfs @ 12.08 hrs, Volume= 0.012 af, Atten= 0%, Lag= 0.0 min

Primary = 0.16 cfs @ 12.08 hrs, Volume= 0.012 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

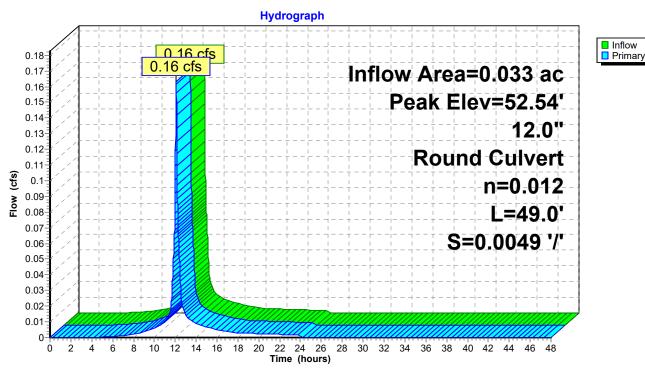
Peak Elev= 52.54' @ 12.14 hrs

Flood Elev= 55.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	51.70'	12.0" Round Culvert
			L= 49.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 51.70' / 51.46' S= 0.0049 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=52.03' TW=52.10' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 63P: YD #21



Summary for Pond 64P: YD #20

Inflow Area = 0.068 ac, 73.50% Impervious, Inflow Depth = 4.50" for 10-year event

Inflow = 0.34 cfs @ 12.08 hrs, Volume= 0.026 af

Outflow = 0.34 cfs @ 12.08 hrs, Volume= 0.026 af, Atten= 0%, Lag= 0.0 min

Primary = 0.34 cfs @ 12.08 hrs, Volume= 0.026 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

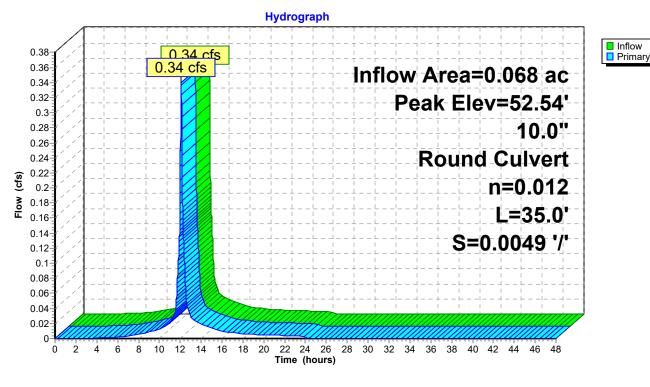
Peak Elev= 52.54' @ 12.13 hrs

Flood Elev= 55.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	51.28'	10.0" Round Culvert
			L= 35.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 51.28' / 51.11' S= 0.0049 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.55 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=52.09' TW=52.23' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 64P: YD #20



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 65P: YD #19

Inflow Area = 0.091 ac, 75.33% Impervious, Inflow Depth = 4.54" for 10-year event

Inflow = 0.45 cfs @ 12.08 hrs, Volume= 0.034 af

Outflow = 0.45 cfs @ 12.08 hrs, Volume= 0.034 af, Atten= 0%, Lag= 0.0 min

Primary = 0.45 cfs @ 12.08 hrs, Volume= 0.034 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

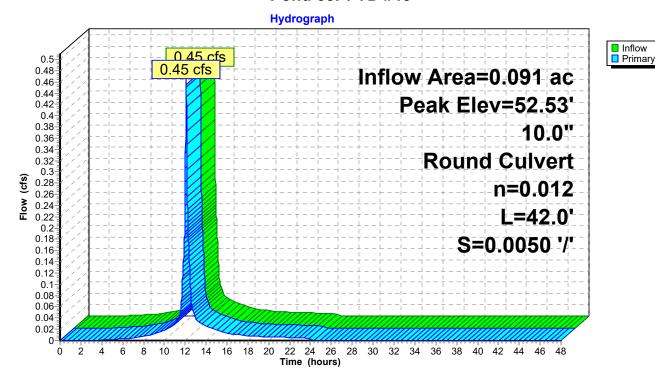
Peak Elev= 52.53' @ 12.12 hrs

Flood Elev= 56.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	51.00'	10.0" Round Culvert
	-		L= 42.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 51.00' / 50.79' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.55 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=52.23' TW=52.33' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 65P: YD #19



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 66P: YD #18

Inflow Area = 0.125 ac, 67.01% Impervious, Inflow Depth = 4.26" for 10-year event

Inflow = 0.59 cfs @ 12.09 hrs, Volume= 0.044 af

Outflow = 0.59 cfs @ 12.09 hrs, Volume= 0.044 af, Atten= 0%, Lag= 0.0 min

Primary = 0.59 cfs @ 12.09 hrs, Volume= 0.044 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

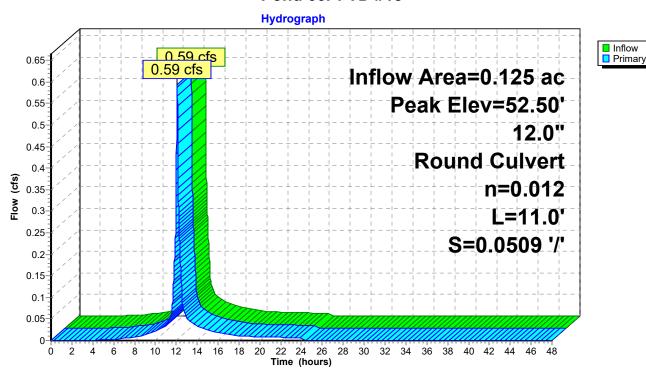
Peak Elev= 52.50' @ 12.11 hrs

Flood Elev= 55.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	50.62'	12.0" Round Culvert
			L= 11.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 50.62' / 50.06' S= 0.0509 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.09 hrs HW=52.34' TW=52.41' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Pond 66P: YD #18



☐ Inflow☐ Primary

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 67P: CB #17

Inflow Area = 0.530 ac, 76.52% Impervious, Inflow Depth = 4.53" for 10-year event

Inflow = 2.62 cfs @ 12.08 hrs, Volume= 0.200 af

Outflow = 2.62 cfs @ 12.08 hrs, Volume= 0.200 af, Atten= 0%, Lag= 0.0 min

Primary = 2.62 cfs @ 12.08 hrs, Volume= 0.200 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

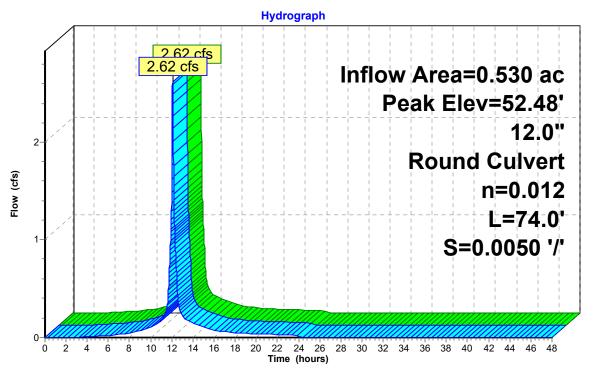
Peak Elev= 52.48' @ 12.10 hrs

Flood Elev= 55.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	49.96'	12.0" Round Culvert
	_		L= 74.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.96' / 49.59' S= 0.0050 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=2.46 cfs @ 12.08 hrs HW=52.40' TW=51.87' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.46 cfs @ 3.13 fps)





Summary for Pond 68P: DMH #16

Inflow Area = 0.562 ac, 77.83% Impervious, Inflow Depth = 4.57" for 10-year event

Inflow = 2.78 cfs @ 12.08 hrs, Volume= 0.214 af

Outflow = 2.78 cfs @ 12.08 hrs, Volume= 0.214 af, Atten= 0%, Lag= 0.0 min

Primary = 2.78 cfs @ 12.08 hrs, Volume= 0.214 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

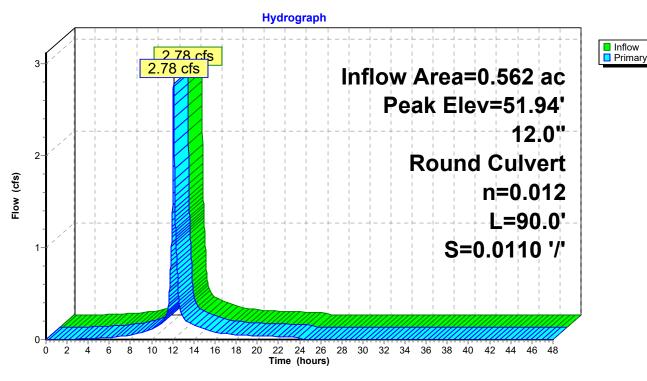
Peak Elev= 51.94' @ 12.11 hrs

Flood Elev= 56.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	49.49'	12.0" Round Culvert
			L= 90.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.49' / 48.50' S= 0.0110 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=2.68 cfs @ 12.08 hrs HW=51.87' TW=51.17' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.68 cfs @ 3.41 fps)

Pond 68P: DMH #16



Volume

Invert

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Printed 10/6/2025

Summary for Pond 69P: Bio #3 / OS #11

Inflow Area = 1.681 ac, 63.59% Impervious, Inflow Depth = 4.16" for 10-year event

Inflow 7.64 cfs @ 12.09 hrs. Volume= 0.583 af

2.52 cfs @ 12.38 hrs, Volume= Outflow = 0.560 af, Atten= 67%, Lag= 17.7 min

2.52 cfs @ 12.38 hrs, Volume= Primary 0.560 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 48.00' Surf.Area= 1,706 sf Storage= 128 cf

Peak Elev= 51.69' @ 12.38 hrs Surf.Area= 4,603 sf Storage= 11,307 cf (11,179 cf above start)

Flood Elev= 53.00' Surf.Area= 6,019 sf Storage= 18,236 cf (18,109 cf above start)

Avail.Storage Storage Description

Plug-Flow detention time= 299.9 min calculated for 0.557 af (95% of inflow)

Center-of-Mass det. time= 270.6 min (1,050.4 - 779.8)

#1	46.5	0'	18,236 c	f Custom Stage I	Data (Conic)Listed	below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	(sq-ft)
46.5	50	1,706	0.0	0	0	1,706
48.0	00	1,706	5.0	128	128	1,926
49.0	00	2,381	100.0	2,034	2,162	2,619
50.0	00	3,056	100.0	2,711	4,874	3,319
51.0	00	3,935	100.0	3,486	8,360	4,223
52.0		4,918	100.0	4,417	12,777	5,233
53.0	00	6,019	100.0	5,459	18,236	6,365
Device	Routing	In	vert Ou	ıtlet Devices		
#1	Primary	45	5.50' 18	.0" Round Culvert		
				50.0' CPP, square	•	
						.0050 '/' Cc= 0.900
""				0.012, Flow Area=		
#2	Device 1	_		" Vert. Underdrain		
#3	Device 2	48				over Wetted area above 48.00'
<i>!! 4</i>	D : 4	50		cluded Wetted area		
#4	Device 1		-)" W x 30.0" H Vert		
#5	Device 1	52		. 0" x 48.0" Horiz. C nited to weir flow at		.600

Primary OutFlow Max=2.52 cfs @ 12.38 hrs HW=51.69' TW=45.65' (Dynamic Tailwater)

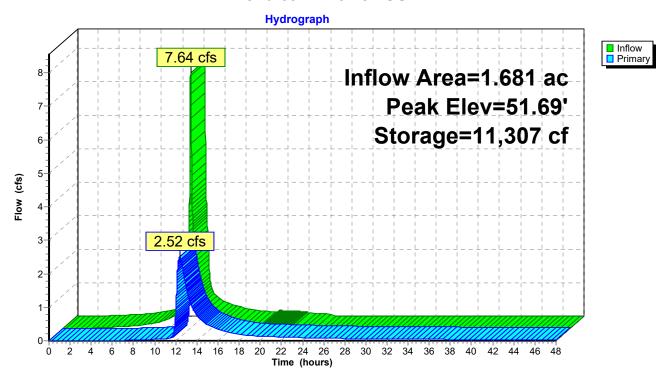
-1=Culvert (Passes 2.52 cfs of 19.85 cfs potential flow)

-2=Underdrain (Passes 0.17 cfs of 1.03 cfs potential flow)
-3=Exfiltration through Media (Exfiltration Controls 0.17 cfs)

-4=Orifice/Grate (Orifice Controls 2.35 cfs @ 4.17 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Pond 69P: Bio #3 / OS #11



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 80P: 6" CPP

Inflow Area = 0.028 ac,100.00% Impervious, Inflow Depth = 5.23" for 10-year event

Inflow = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af

Outflow = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af, Atten= 0%, Lag= 0.0 min

Primary = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

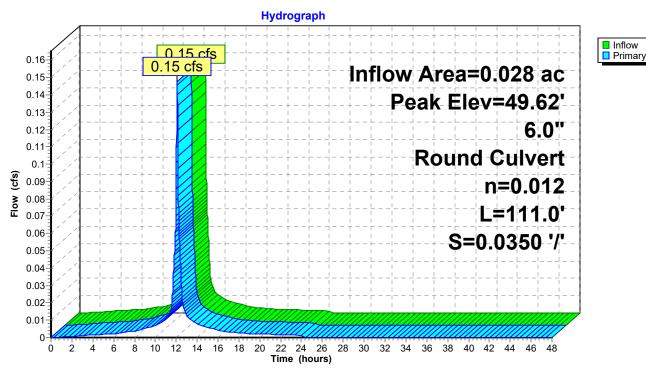
Peak Elev= 49.62' @ 12.08 hrs

Flood Elev= 52.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	49.39'	6.0" Round Culvert
	-		L= 111.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.39' / 45.50' S= 0.0350 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.20 sf

Primary OutFlow Max=0.15 cfs @ 12.08 hrs HW=49.62' TW=47.91' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.15 cfs @ 1.64 fps)

Pond 80P: 6" CPP



Summary for Pond 81P: 6" CPP

Inflow Area = 0.028 ac,100.00% Impervious, Inflow Depth = 5.23" for 10-year event

Inflow = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af

Outflow = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af, Atten= 0%, Lag= 0.0 min

Primary = 0.15 cfs @ 12.08 hrs, Volume= 0.012 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

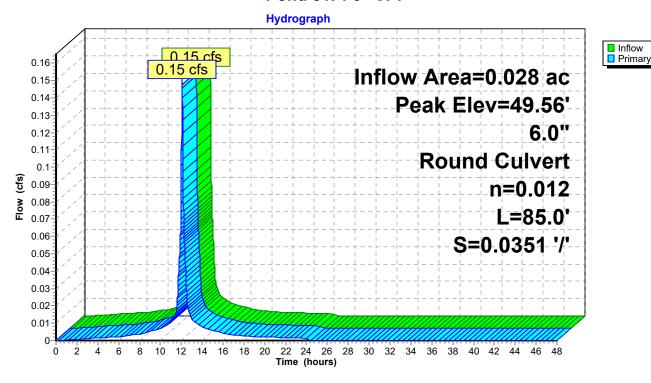
Peak Elev= 49.56' @ 12.08 hrs

Flood Elev= 52.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	49.33'	6.0" Round Culvert
			L= 85.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 49.33' / 46.35' S= 0.0351 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.15 cfs @ 12.08 hrs HW=49.56' TW=47.99' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.15 cfs @ 1.64 fps)

Pond 81P: 6" CPP



☐ Inflow☐ Primary

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 82P: CB #23

Inflow Area = 0.248 ac, 91.46% Impervious, Inflow Depth = 4.92" for 10-year event

Inflow = 1.30 cfs @ 12.08 hrs, Volume= 0.102 af

Outflow = 1.30 cfs @ 12.08 hrs, Volume= 0.102 af, Atten= 0%, Lag= 0.0 min

Primary = 1.30 cfs @ 12.08 hrs, Volume= 0.102 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

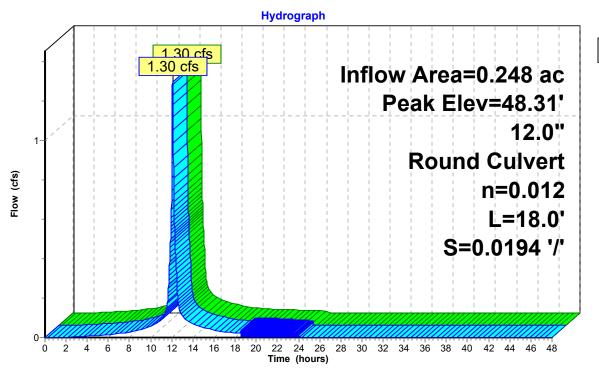
Peak Elev= 48.31' @ 12.39 hrs

Flood Elev= 49.85'

Device	Routing	Invert	Outlet Devices
#1	Primary	45.85'	12.0" Round Culvert
			L= 18.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 45.85' / 45.50' S= 0.0194 '/' Cc= 0.900
			n= 0.012. Flow Area= 0.79 sf

Primary OutFlow Max=1.07 cfs @ 12.08 hrs HW=47.99' TW=47.91' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.07 cfs @ 1.36 fps)

Pond 82P: CB #23



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 83P: Bio #4 / OS #22

Inflow Area = 0.333 ac, 76.36% Impervious, Inflow Depth = 4.57" for 10-year event

Inflow 1.63 cfs @ 12.08 hrs, Volume= 0.127 af

0.50 cfs @ 12.40 hrs, Volume= Outflow = 0.127 af, Atten= 70%, Lag= 18.9 min

0.50 cfs @ 12.40 hrs, Volume= Primary 0.127 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 45.50' Surf.Area= 315 sf Storage= 24 cf

Peak Elev= 48.30' @ 12.40 hrs Surf.Area= 1,352 sf Storage= 2,220 cf (2,196 cf above start)

Flood Elev= 49.00' Surf.Area= 1,695 sf Storage= 3,283 cf (3,260 cf above start)

Plug-Flow detention time= 216.3 min calculated for 0.126 af (99% of inflow)

Center-of-Mass det. time= 208.5 min (977.7 - 769.2)

Volume	Inve	ert Ava	il.Stora	ge Storage Descr	iption	
#1	44.0	0'	3,283	cf Custom Stage	e Data (Prismatio	Listed below (Recalc)
Clayeti	- n	Curf Area	Voido	Ina Ctara	Cum Store	
Elevation		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
44.(00	315	0.0	0	0	
45.5	50	315	5.0	24	24	
46.0	00	455	100.0	193	216	
47.0		789	100.0	622	838	
48.0		1,203	100.0	996	1,834	
49.0		1,695	100.0	1,449	3,283	
49.0	<i>.</i>	1,095	100.0	1,449	3,203	
Device	Routing	In	vert (Outlet Devices		
#1	Primary			5.0" Round Culve	art	
π ι	1 minary	70		.= 26.0' CPP, squa		I Ko- 0 500
						S= 0.0058 '/'
						5- 0.0056 / CC- 0.900
				= 0.012, Flow Are		
#2	Device 1	_		.0" Vert. Underdra		
#3	Device 2	45	5.50' 2	2.500 in/hr Exfiltrat	tion through Med	lia over Surface area above 45.50'
			Е	Excluded Surface a	rea = 315 sf Phas	se-In= 0.01'
#4	Device 1	47	'.05' 4	.0" Vert. Orifice/G	O" Vert. Orifice/Grate C= 0.600	
#5	Device 1	48	3.50' 2	4.0" x 24.0" Horiz	Orifice/Grate	C= 0.600

Primary OutFlow Max=0.50 cfs @ 12.40 hrs HW=48.30' TW=42.90' (Dynamic Tailwater)

Limited to weir flow at low heads

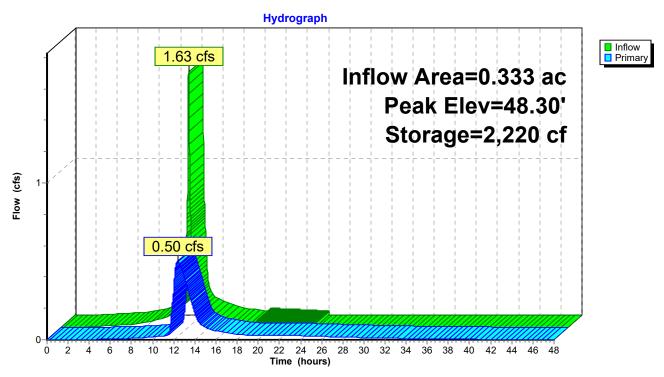
-1=Culvert (Passes 0.50 cfs of 12.78 cfs potential flow)

⁻²⁼Underdrain (Passes 0.06 cfs of 0.95 cfs potential flow)
-3=Exfiltration through Media (Exfiltration Controls 0.06 cfs)

⁻⁴⁼Orifice/Grate (Orifice Controls 0.44 cfs @ 5.02 fps)

⁻⁵⁼Orifice/Grate (Controls 0.00 cfs)

Pond 83P: Bio #4 / OS #22



Summary for Link 100L: POA #100 - SW

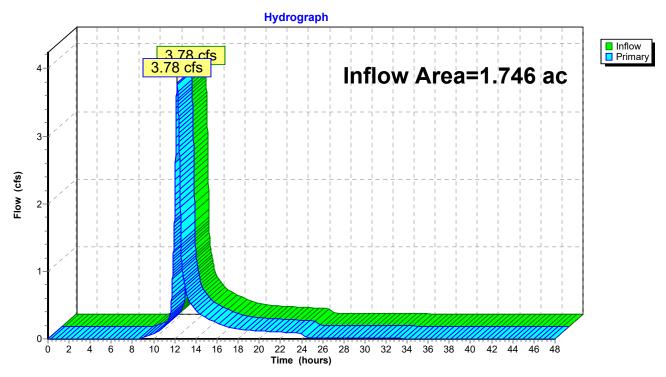
Inflow Area = 1.746 ac, 13.99% Impervious, Inflow Depth > 2.95" for 10-year event

Inflow = 3.78 cfs @ 12.22 hrs, Volume= 0.430 af

Primary = 3.78 cfs @ 12.22 hrs, Volume= 0.430 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 100L: POA #100 - SW



Summary for Link 200L: POA #200 - South

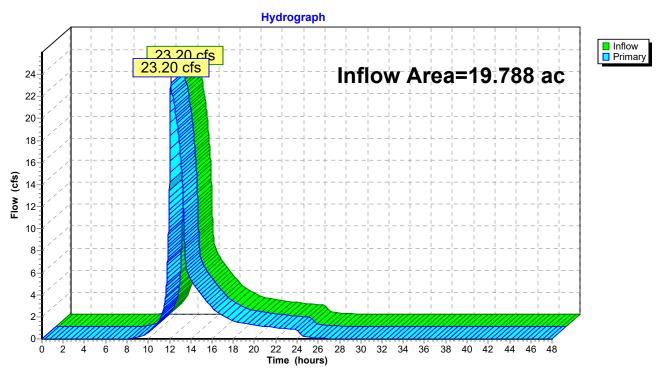
Inflow Area = 19.788 ac, 10.08% Impervious, Inflow Depth = 2.78" for 10-year event

Inflow = 23.20 cfs @ 12.12 hrs, Volume= 4.580 af

Primary = 23.20 cfs @ 12.12 hrs, Volume= 4.580 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 200L: POA #200 - South



Summary for Link 300L: POA #300 - SE

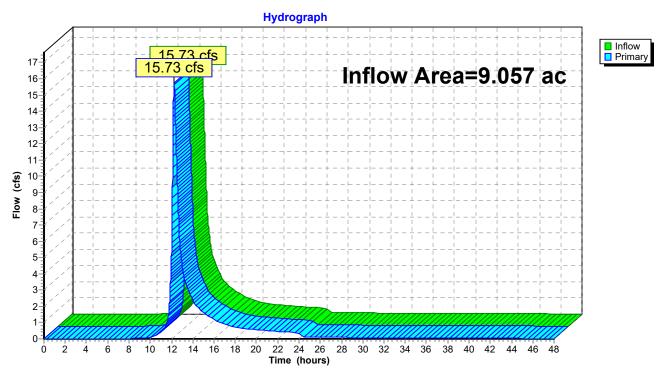
Inflow Area = 9.057 ac, 16.31% Impervious, Inflow Depth > 2.64" for 10-year event

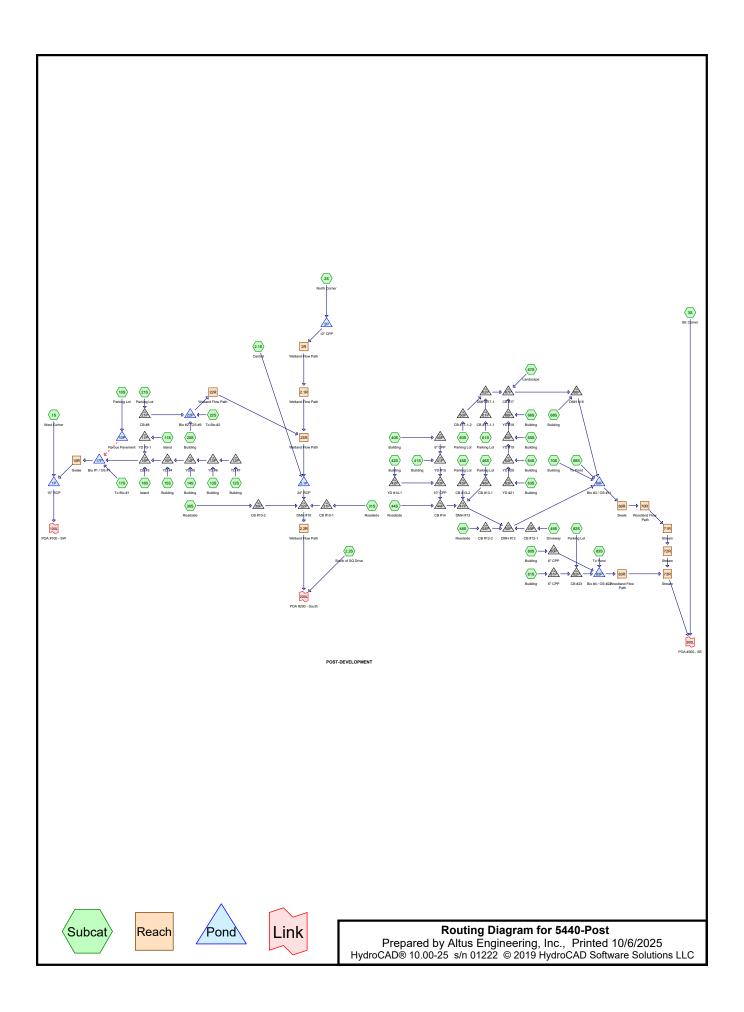
Inflow = 15.73 cfs @ 12.24 hrs, Volume= 1.989 af

Primary = 15.73 cfs @ 12.24 hrs, Volume= 1.989 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 300L: POA #300 - SE





Tc=6.0 min CN=74 Runoff=0.42 cfs 0.030 af

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Printed 10/6/2025

Page 2

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

rtodon rodding by byn oto	ind motiod
Subcatchment 1S: West Corner	Runoff Area=66,034 sf 9.68% Impervious Runoff Depth=4.09" Flow Length=366' Tc=9.8 min CN=75 Runoff=6.39 cfs 0.516 af
Subcatchment 2.1S: Central	Runoff Area=444,574 sf 8.10% Impervious Runoff Depth=3.98" Flow Length=1,341' Tc=15.8 min CN=74 Runoff=35.31 cfs 3.386 af
Subcatchment 2.2S: South of SQ Driv	Runoff Area=102,088 sf 10.13% Impervious Runoff Depth=4.30" Flow Length=391' Tc=6.0 min CN=77 Runoff=11.79 cfs 0.840 af
Subcatchment 2S: North Corner	Runoff Area=284,221 sf 6.78% Impervious Runoff Depth=3.77" Tc=0.0 min CN=72 Runoff=35.33 cfs 2.050 af
Subcatchment3S: SE Corner	Runoff Area=306,834 sf 2.19% Impervious Runoff Depth=3.35" Flow Length=1,149' Tc=15.9 min CN=68 Runoff=20.35 cfs 1.969 af
Subcatchment 10S: Parking Lot	Runoff Area=10,182 sf 98.98% Impervious Runoff Depth>6.65" Tc=790.0 min CN=98 Runoff=0.11 cfs 0.130 af
Subcatchment 11S: Island	Runoff Area=588 sf 65.82% Impervious Runoff Depth=5.75" Tc=6.0 min CN=90 Runoff=0.09 cfs 0.006 af
Subcatchment 12S: Building	Runoff Area=1,878 sf 65.12% Impervious Runoff Depth=5.75" Tc=6.0 min CN=90 Runoff=0.27 cfs 0.021 af
Subcatchment 13S: Building	Runoff Area=1,349 sf 73.83% Impervious Runoff Depth=5.99" Tc=6.0 min CN=92 Runoff=0.20 cfs 0.015 af
Subcatchment 14S: Building	Runoff Area=1,021 sf 77.86% Impervious Runoff Depth=6.10" Tc=6.0 min CN=93 Runoff=0.15 cfs 0.012 af
Subcatchment 15S: Building	Runoff Area=1,376 sf 43.24% Impervious Runoff Depth=5.07" Tc=6.0 min CN=84 Runoff=0.18 cfs 0.013 af
Subcatchment 16S: Island	Runoff Area=928 sf 26.51% Impervious Runoff Depth=4.63" Tc=6.0 min CN=80 Runoff=0.11 cfs 0.008 af
Subcatchment 17S: To Bio #1	Runoff Area=2,893 sf 0.28% Impervious Runoff Depth=3.98" Tc=6.0 min CN=74 Runoff=0.31 cfs 0.022 af
Subcatchment 20S: Building	Runoff Area=9,168 sf 100.00% Impervious Runoff Depth=6.69" Tc=6.0 min CN=98 Runoff=1.43 cfs 0.117 af
Subcatchment 21S: Parking Lot	Runoff Area=2,610 sf 100.00% Impervious Runoff Depth=6.69" Tc=6.0 min CN=98 Runoff=0.41 cfs 0.033 af
Subcatchment 22S: To Bio #2	Runoff Area=3,961 sf 0.00% Impervious Runoff Depth=3.98"

5440-Post	Type III 24-hr	25-year Rainfall=6.93"
Prepared by Altus Engineering, Inc.		Printed 10/6/2025
HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solution	ns LLC	Page 3

Tc=6.0 min CN=70 Runoff=0.14 cfs 0.010 af

Tc=6.0 min CN=98 Runoff=0.21 cfs 0.018 af

Runoff Area=1,370 sf 100.00% Impervious Runoff Depth=6.69"

Subcatchment 68S: Building

Try dree 7 tb @ To . oo Ze om o 1222 @ Ze Te Try dr	Tage o
Subcatchment 30S: Roadside	Runoff Area=8,230 sf 53.10% Impervious Runoff Depth=5.41" Flow Length=204' Tc=6.0 min CN=87 Runoff=1.15 cfs 0.085 af
Subcatchment31S: Roadside	Runoff Area=7,135 sf 71.11% Impervious Runoff Depth=5.87" Flow Length=143' Tc=6.0 min CN=91 Runoff=1.06 cfs 0.080 af
Subcatchment 40S: Building	Runoff Area=3,066 sf 100.00% Impervious Runoff Depth=6.69" Tc=6.0 min CN=98 Runoff=0.48 cfs 0.039 af
Subcatchment41S: Building	Runoff Area=2,125 sf 90.78% Impervious Runoff Depth=6.45" Tc=6.0 min CN=96 Runoff=0.33 cfs 0.026 af
Subcatchment 42S: Building	Runoff Area=3,298 sf 74.41% Impervious Runoff Depth=5.99" Tc=6.0 min CN=92 Runoff=0.49 cfs 0.038 af
Subcatchment 44S: Roadside	Runoff Area=4,663 sf 40.92% Impervious Runoff Depth=5.07" Tc=6.0 min CN=84 Runoff=0.62 cfs 0.045 af
Subcatchment 45S: Parking Lot	Runoff Area=3,860 sf 71.71% Impervious Runoff Depth=5.87" Tc=6.0 min CN=91 Runoff=0.57 cfs 0.043 af
Subcatchment 46S: Parking Lot	Runoff Area=4,489 sf 73.25% Impervious Runoff Depth=5.99" Tc=6.0 min CN=92 Runoff=0.67 cfs 0.051 af
Subcatchment48S: Roadside	Runoff Area=3,415 sf 55.67% Impervious Runoff Depth=5.53" Tc=6.0 min CN=88 Runoff=0.49 cfs 0.036 af
Subcatchment49S: Driveway	Runoff Area=3,657 sf 83.78% Impervious Runoff Depth=6.10" Tc=6.0 min CN=93 Runoff=0.55 cfs 0.043 af
Subcatchment 60S: Parking Lot	Runoff Area=9,169 sf 78.55% Impervious Runoff Depth=6.10" Flow Length=116' Tc=6.0 min CN=93 Runoff=1.39 cfs 0.107 af
Subcatchment 61S: Parking Lot	Runoff Area=7,068 sf 91.98% Impervious Runoff Depth=6.45" Flow Length=111' Tc=6.0 min CN=96 Runoff=1.09 cfs 0.087 af
Subcatchment 63S: Building	Runoff Area=1,459 sf 65.52% Impervious Runoff Depth=5.75" Tc=6.0 min CN=90 Runoff=0.21 cfs 0.016 af
Subcatchment 64S: Building	Runoff Area=1,507 sf 81.22% Impervious Runoff Depth=6.10" Tc=6.0 min CN=93 Runoff=0.23 cfs 0.018 af
Subcatchment 65S: Building	Runoff Area=982 sf 80.86% Impervious Runoff Depth=6.10" Tc=6.0 min CN=93 Runoff=0.15 cfs 0.011 af
Subcatchment 66S: Building	Runoff Area=1,478 sf 44.79% Impervious Runoff Depth=4.85" Tc=6.0 min CN=82 Runoff=0.19 cfs 0.014 af
Subcatchment 67S: Landscape	Runoff Area=1,430 sf 23.15% Impervious Runoff Depth=3.56"

Printed 10/6/2025

Page 4

Subcatchment 69S: To Pond Runoff Area=13,261 sf 1.69% Impervious Runoff Depth=3.35"

Tc=6.0 min CN=68 Runoff=1.19 cfs 0.085 af

Subcatchment 70S: Building Runoff Area=6,906 sf 100.00% Impervious Runoff Depth=6.69"

Tc=6.0 min CN=98 Runoff=1.08 cfs 0.088 af

Subcatchment 80S: Building Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=6.69"

Tc=6.0 min CN=98 Runoff=0.19 cfs 0.015 af

Subcatchment 81S: Building Runoff Area=1,200 sf 100.00% Impervious Runoff Depth=6.69"

Tc=6.0 min CN=98 Runoff=0.19 cfs 0.015 af

Subcatchment 82S: Parking Lot Runoff Area=9,597 sf 90.39% Impervious Runoff Depth=6.34"

Flow Length=214' Tc=6.0 min CN=95 Runoff=1.47 cfs 0.116 af

Subcatchment83S: To Pond Runoff Area=2,507 sf 0.00% Impervious Runoff Depth=3.98"

Tc=6.0 min CN=74 Runoff=0.27 cfs 0.019 af

Reach 2.1R: Wetland Flow Path

Avg. Flow Depth=1.53' Max Vel=2.91 fps Inflow=31.17 cfs 2.050 af

 $n = 0.040 \quad L = 344.0' \quad S = 0.0075 \; \text{$''$} \quad Capacity = 54.59 \; \text{cfs} \quad Outflow = 29.25 \; \text{cfs} \quad 2.050 \; \text{af} \quad Capacity = 54.59 \; \text{cfs} \quad Outflow = 29.25 \; \text{cfs} \quad Outflow$

Reach 2.2R: Wetland Flow Path Avg. Flow Depth=0.86' Max Vel=5.46 fps Inflow=23.05 cfs 5.780 af

n=0.040 L=263.0' S=0.0502 '/' Capacity=141.18 cfs Outflow=21.61 cfs 5.780 af

Reach 2R: Wetland Flow Path Avg. Flow Depth=1.05' Max Vel=5.74 fps Inflow=31.29 cfs 2.050 af

n=0.040 L=151.0' S=0.0446 '/' Capacity=133.09 cfs Outflow=31.17 cfs 2.050 af

Reach 10R: Swale Avg. Flow Depth=0.13' Max Vel=0.72 fps Inflow=0.40 cfs 0.098 af

n=0.035 L=44.0' S=0.0050 '/' Capacity=16.22 cfs Outflow=0.40 cfs 0.098 af

Reach 22R: Wetland Flow Path Avg. Flow Depth=0.09' Max Vel=1.11 fps Inflow=1.15 cfs 0.179 af

n=0.040 L=211.0' S=0.0245 '/' Capacity=392.58 cfs Outflow=1.09 cfs 0.179 af

Reach 23R: Wetland Flow Path Avg. Flow Depth=1.25' Max Vel=4.09 fps Inflow=29.59 cfs 2.229 af

n=0.040 L=131.0' S=0.0185'/' Capacity=85.86 cfs Outflow=29.49 cfs 2.229 af

Reach 69R: Swale Avg. Flow Depth=0.49' Max Vel=1.37 fps Inflow=3.70 cfs 0.752 af

n=0.069 L=17.0' S=0.0147'/' Capacity=14.11 cfs Outflow=3.70 cfs 0.752 af

Reach 70R: Woodland Flow Path

Avg. Flow Depth=0.41' Max Vel=0.64 fps Inflow=3.70 cfs 0.752 af

 $n = 0.400 \quad L = 57.0' \quad S = 0.1404 \; \text{$'$} \quad Capacity = 21.20 \; cfs \quad Outflow = 3.70 \; cfs \quad 0.752 \; af$

Reach 71R: Stream Avg. Flow Depth=0.35' Max Vel=3.49 fps Inflow=3.70 cfs 0.752 af

n=0.040 L=80.0' S=0.0554 '/' Capacity=31.12 cfs Outflow=3.70 cfs 0.752 af

Reach 72R: Stream Avg. Flow Depth=0.36' Max Vel=2.05 fps Inflow=3.70 cfs 0.752 af

n=0.040 L=88.0' S=0.0160'/' Capacity=25.40 cfs Outflow=3.70 cfs 0.751 af

Reach 73R: Stream Avg. Flow Depth=0.40' Max Vel=2.19 fps Inflow=4.56 cfs 0.917 af

n=0.040 L=309.0' S=0.0160 '/' Capacity=25.38 cfs Outflow=4.54 cfs 0.917 af

5440-Post Prepared by Altus Enginee HydroCAD® 10.00-25 s/n 0122	Type III 24-hr 25-year Rainfall=6.93" ering, Inc. Printed 10/6/2025 22 © 2019 HydroCAD Software Solutions LLC Page 5
Reach 83R: Woodland Flow	Path Avg. Flow Depth=0.20' Max Vel=0.36 fps Inflow=0.97 cfs 0.166 af n=0.400 L=122.0' S=0.0950'/ Capacity=17.44 cfs Outflow=0.87 cfs 0.166 af
Pond 1P: 15" RCP	Peak Elev=53.83' Storage=1,591 cf Inflow=6.73 cfs 0.614 af 15.0" Round Culvert n=0.012 L=60.0' S=0.0037 '/' Outflow=4.78 cfs 0.614 af
Pond 2.1P: 24" RCP	Peak Elev=52.31' Storage=51,030 cf Inflow=52.10 cfs 5.615 af 24.0" Round Culvert n=0.012 L=17.0' S=0.0135 '/' Outflow=22.79 cfs 5.615 af
Pond 2P: 12" CPP	Peak Elev=61.01' Storage=13,034 cf Inflow=35.33 cfs 2.050 af Outflow=31.29 cfs 2.050 af
Pond 10P: Porous Pavemen	nt Peak Elev=59.19' Storage=2,502 cf Inflow=0.11 cfs 0.130 af carded=0.04 cfs 0.105 af Secondary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.105 af
Pond 11P: YD #3-1	Peak Elev=58.23' Inflow=0.09 cfs 0.006 af 6.0" Round Culvert n=0.012 L=36.0' S=0.0050 '/' Outflow=0.09 cfs 0.006 af
Pond 12P: YD #7	Peak Elev=58.25' Inflow=0.27 cfs 0.021 af 8.0" Round Culvert n=0.012 L=52.0' S=0.0050 '/' Outflow=0.27 cfs 0.021 af
Pond 13P: YD #6	Peak Elev=58.24' Inflow=0.48 cfs 0.036 af 10.0" Round Culvert n=0.012 L=29.0' S=0.0052 '/' Outflow=0.48 cfs 0.036 af
Pond 14P: YD #5	Peak Elev=58.23' Inflow=0.63 cfs 0.048 af 10.0" Round Culvert n=0.012 L=20.0' S=0.0050 '/' Outflow=0.63 cfs 0.048 af
Pond 15P: YD #4	Peak Elev=58.23' Inflow=0.81 cfs 0.061 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.81 cfs 0.061 af
Pond 16P: CB #3	Peak Elev=58.22' Inflow=1.01 cfs 0.076 af 12.0" Round Culvert n=0.012 L=45.0' S=0.0049 '/' Outflow=1.01 cfs 0.076 af
Pond 17P: Bio #1 / OS #1	Peak Elev=58.22' Storage=1,786 cf Inflow=1.32 cfs 0.098 af Outflow=0.40 cfs 0.098 af
Pond 21P: CB #8	Peak Elev=59.59' Inflow=0.41 cfs 0.033 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0050 '/' Outflow=0.41 cfs 0.033 af
Pond 22P: Bio #2 / OS #9	Peak Elev=59.59' Storage=3,010 cf Inflow=2.26 cfs 0.181 af Outflow=1.15 cfs 0.179 af
Pond 30P: CB #10-2	Peak Elev=53.21' Inflow=1.15 cfs 0.085 af 12.0" Round Culvert n=0.012 L=78.0' S=0.0397 '/' Outflow=1.15 cfs 0.085 af
Pond 31P: CB #10-1	Peak Elev=50.61' Inflow=1.06 cfs 0.080 af 12.0" Round Culvert n=0.012 L=2.0' S=0.0250 '/' Outflow=1.06 cfs 0.080 af
Pond 32P: DMH #10	Peak Elev=50.61' Inflow=23.05 cfs 5.780 af

Pond 40P: 6" CPP

Peak Elev=54.65' Inflow=0.48 cfs 0.039 af 6.0" Round Culvert n=0.012 L=174.0' S=0.0150 '/' Outflow=0.48 cfs 0.039 af

24.0" Round Culvert n=0.012 L=57.0' S=0.0135 '/' Outflow=23.05 cfs 5.780 af

Printed 10/6/2025

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Page 6

Pond 41P: YD #15	Peak Elev=53.63' Inflow=0.81 cfs 0.065 af 8.0" Round Culvert n=0.012 L=47.0' S=0.0149 '/' Outflow=0.81 cfs 0.065 af
Pond 42P: YD #14-1	Peak Elev=54.49' Inflow=0.49 cfs 0.038 af 6.0" Round Culvert n=0.012 L=158.0' S=0.0175 '/' Outflow=0.49 cfs 0.038 af
Pond 43P: 10" CPP	Peak Elev=53.38' Inflow=1.30 cfs 0.103 af 8.0" Round Culvert n=0.012 L=33.0' S=0.0148 '/' Outflow=1.30 cfs 0.103 af
Pond 44P: CB #14	Peak Elev=52.83' Inflow=1.92 cfs 0.149 af 12.0" Round Culvert n=0.012 L=31.0' S=0.0048 '/' Outflow=1.92 cfs 0.149 af
Pond 45P: CB #13-2	Peak Elev=52.63' Inflow=0.57 cfs 0.043 af 12.0" Round Culvert n=0.012 L=22.0' S=0.0164 '/' Outflow=0.57 cfs 0.043 af
Pond 46P: CB #13-1	Peak Elev=52.64' Inflow=0.67 cfs 0.051 af 12.0" Round Culvert n=0.012 L=12.0' S=0.0050 '/' Outflow=0.67 cfs 0.051 af
Pond 47P: DMH #13	Peak Elev=52.61' Inflow=3.17 cfs 0.243 af 15.0" Round Culvert n=0.012 L=67.0' S=0.0040 '/' Outflow=3.17 cfs 0.243 af
Pond 48P: CB #12-2	Peak Elev=52.38' Inflow=0.49 cfs 0.036 af 12.0" Round Culvert n=0.012 L=6.0' S=0.0050 '/' Outflow=0.49 cfs 0.036 af
Pond 49P: CB #12-1	Peak Elev=52.39' Inflow=0.55 cfs 0.043 af 12.0" Round Culvert n=0.012 L=46.0' S=0.0050 '/' Outflow=0.55 cfs 0.043 af
Pond 50P: DMH #12	Peak Elev=52.37' Inflow=4.20 cfs 0.322 af 15.0" Round Culvert n=0.012 L=103.0' S=0.0040 '/' Outflow=4.20 cfs 0.322 af
Pond 60P: CB #17-1-2	Peak Elev=54.43' Inflow=1.39 cfs 0.107 af 12.0" Round Culvert n=0.012 L=21.0' S=0.0052 '/' Outflow=1.39 cfs 0.107 af
Pond 61P: CB #17-1-1	Peak Elev=54.38' Inflow=1.09 cfs 0.087 af 12.0" Round Culvert n=0.012 L=3.0' S=0.0367 '/' Outflow=1.09 cfs 0.087 af
Pond 62P: DMH #17-1	Peak Elev=54.31' Inflow=2.48 cfs 0.194 af 12.0" Round Culvert n=0.012 L=25.0' S=0.0052'/' Outflow=2.48 cfs 0.194 af
Pond 63P: YD #21	Peak Elev=54.02' Inflow=0.21 cfs 0.016 af 12.0" Round Culvert n=0.012 L=49.0' S=0.0049 '/' Outflow=0.21 cfs 0.016 af
Pond 64P: YD #20	Peak Elev=54.02' Inflow=0.44 cfs 0.034 af 10.0" Round Culvert n=0.012 L=35.0' S=0.0049 '/' Outflow=0.44 cfs 0.034 af
Pond 65P: YD #19	Peak Elev=53.99' Inflow=0.59 cfs 0.045 af 10.0" Round Culvert n=0.012 L=42.0' S=0.0050 '/' Outflow=0.59 cfs 0.045 af
Pond 66P: YD #18	Peak Elev=53.95' Inflow=0.78 cfs 0.059 af 12.0" Round Culvert n=0.012 L=11.0' S=0.0509 '/' Outflow=0.78 cfs 0.059 af

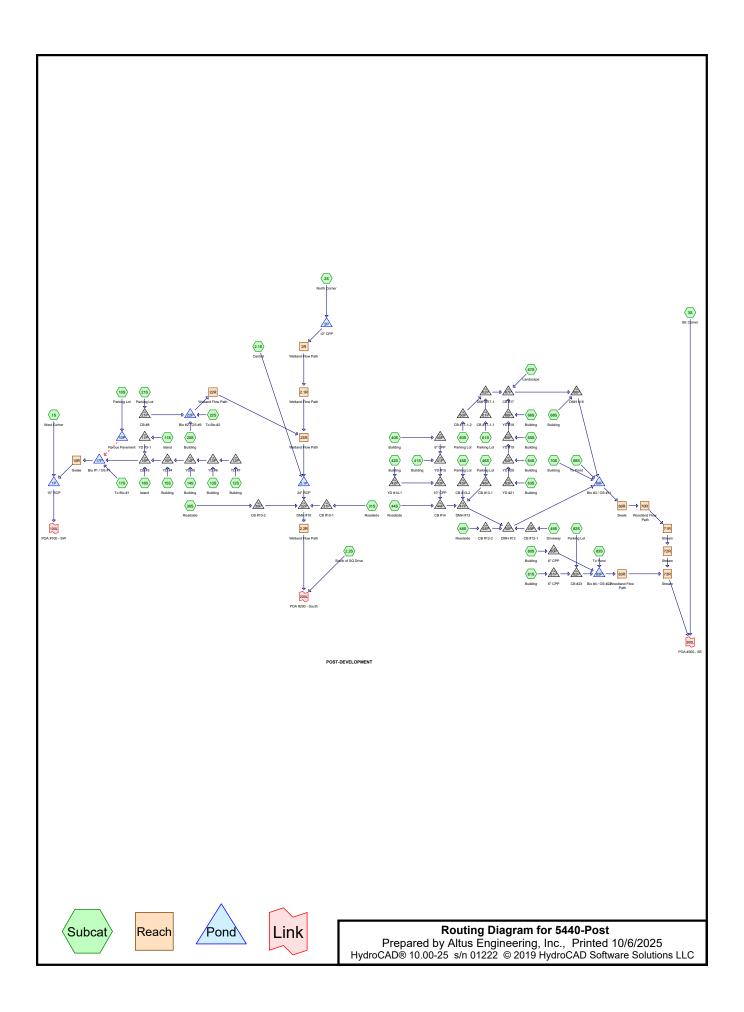
5440-Post Prepared by Altus Engineerii HydroCAD® 10.00-25 s/n 01222	Type III 24-hr 25-year Rainfall=6.93" ng, Inc. Printed 10/6/2025 © 2019 HydroCAD Software Solutions LLC Page 7
Pond 67P: CB #17	Peak Elev=53.91' Inflow=3.39 cfs 0.263 af 12.0" Round Culvert n=0.012 L=74.0' S=0.0050 '/' Outflow=3.39 cfs 0.263 af
Pond 68P: DMH #16	Peak Elev=52.96' Inflow=3.61 cfs 0.280 af 12.0" Round Culvert n=0.012 L=90.0' S=0.0110 '/' Outflow=3.61 cfs 0.280 af
Pond 69P: Bio #3 / OS #11	Peak Elev=52.21' Storage=13,815 cf Inflow=10.08 cfs 0.776 af Outflow=3.70 cfs 0.752 af
Pond 80P: 6" CPP	Peak Elev=49.66' Inflow=0.19 cfs 0.015 af 6.0" Round Culvert n=0.012 L=111.0' S=0.0350 '/' Outflow=0.19 cfs 0.015 af
Pond 81P: 6" CPP	Peak Elev=49.60' Inflow=0.19 cfs 0.015 af 6.0" Round Culvert n=0.012 L=85.0' S=0.0351 '/' Outflow=0.19 cfs 0.015 af
Pond 82P: CB #23	Peak Elev=48.60' Inflow=1.66 cfs 0.132 af 12.0" Round Culvert n=0.012 L=18.0' S=0.0194 '/' Outflow=1.66 cfs 0.132 af
Pond 83P: Bio #4 / OS #22	Peak Elev=48.56' Storage=2,590 cf Inflow=2.12 cfs 0.166 af Outflow=0.97 cfs 0.166 af
Link 100L: POA #100 - SW	Inflow=4.78 cfs 0.614 af Primary=4.78 cfs 0.614 af
Link 200L: POA #200 - South	Inflow=30.13 cfs 6.621 af

Link 300L: POA #300 - SE

Total Runoff Area = 30.826 ac Runoff Volume = 10.277 af Average Runoff Depth = 4.00" 87.20% Pervious = 26.879 ac 12.80% Impervious = 3.947 ac

Primary=30.13 cfs 6.621 af

Inflow=24.21 cfs 2.885 af Primary=24.21 cfs 2.885 af



Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 17P: Bio #1 / OS #1

Inflow Area = 0.230 ac, 42.36% Impervious, Inflow Depth = 10.03" for 100-year event

Inflow 2.03 cfs @ 12.08 hrs. Volume= 0.193 af

0.66 cfs @ 12.38 hrs, Volume= Outflow = 0.190 af, Atten= 67%, Lag= 17.7 min

0.66 cfs @ 12.38 hrs, Volume= Primary 0.190 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 56.25' Surf.Area= 487 sf Storage= 37 cf

Peak Elev= 58.78' @ 12.38 hrs Surf.Area= 1,664 sf Storage= 2,627 cf (2,591 cf above start)

Flood Elev= 59.25' Surf.Area= 1,950 sf Storage= 3,480 cf (3,444 cf above start)

Plug-Flow detention time= 193.7 min calculated for 0.190 af (98% of inflow)

Center-of-Mass det. time= 175.7 min (1,126.1 - 950.4)

Volume	Invert	Avail	.Storage	Storage Descrip	tion		_
#1	54.75'		3,480 cf	Custom Stage I	Data (Conic)Listed	below (Recalc)	
Elevation	on Si	urf.Area	Voids	Inc.Store	Cum.Store	Wet.Area	
(fee		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	(sq-ft)	
54.7		487	0.0	0	Ó	487	
56.2	25	487	5.0	37	37	604	
57.0	00	774	100.0	469	505	899	
58.0	00	1,239	100.0	997	1,503	1,377	
59.0	00	1,797	100.0	1,509	3,012	1,952	
59.2	25	1,950	100.0	468	3,480	2,110	
Device	Routing	Inv	ert Outl	et Devices			
#1		53.		" Round Culver	•		_
#1	Primary	55.			ւ e edge headwall, K	0= 0.500	
						.0050 '/' Cc= 0.900	
				.012, Flow Area=		.0030 / CC= 0.900	
#2	Device 1	53.		Vert. Underdrain			
#3	Device 2	56.				over Wetted area above 56	25'
,, 0	201.00 2	00.			a = 604 sf Phase-In		0
#4	Device 1	57.		Vert. Orifice/Gra			
#5	Device 1	58.	75' 24.0		Orifice/Grate C= 0	.600	

Primary OutFlow Max=0.66 cfs @ 12.38 hrs HW=58.78' TW=53.77' (Dynamic Tailwater)

-1=Culvert (Passes 0.66 cfs of 12.40 cfs potential flow)

⁻²⁼Underdrain (Passes 0.07 cfs of 0.93 cfs potential flow)
-3=Exfiltration throiugh Media (Exfiltration Controls 0.07 cfs)

⁻⁴⁼Orifice/Grate (Orifice Controls 0.47 cfs @ 5.41 fps)

⁻⁵⁼Orifice/Grate (Weir Controls 0.12 cfs @ 0.54 fps)

5440-Post

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Summary for Pond 22P: Bio #2 / OS #9

Inflow Area = 0.361 ac, 74.83% Impervious, Inflow Depth = 8.94" for 100-year event

Inflow 3.35 cfs @ 12.08 hrs. Volume= 0.269 af

2.92 cfs @ 12.13 hrs, Volume= Outflow = 0.268 af, Atten= 13%, Lag= 2.7 min

2.92 cfs @ 12.13 hrs, Volume= Primary 0.268 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 57.75' Surf.Area= 1,027 sf Storage= 77 cf

Peak Elev= 59.70' @ 12.13 hrs Surf.Area= 2,291 sf Storage= 3,273 cf (3,196 cf above start)

Flood Elev= 60.00' Surf.Area= 2,511 sf Storage= 3,983 cf (3,906 cf above start)

Plug-Flow detention time= 168.2 min calculated for 0.266 af (99% of inflow)

Center-of-Mass det. time= 155.5 min (907.2 - 751.7)

Volume	Inve	rt Ava	il.Storaç	ge Storage Descri	ption		
#1	56.25	5'	3,983	cf Custom Stage	Data (Conic)Listed	below (Recalc)	
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
56.2	25	1,027	0.0	0	0	1,027	
57.1 58.0	00	1,027 1,175	5.0	77 275	77 352	1,197 1,348	
59.0 60.0		1,809 2,511	100.0 100.0	1,481 2,150	1,833 3,983	1,997 2,718	
Device	Routing	ln	vert C	Outlet Devices			
#1	Primary	55	L Ir		re edge headwall, ł 55.25' / 55.16' S= (Ke= 0.500 0.0050 '/' Cc= 0.900	
#2 #3	Device 1 Device 2		5.25' 4 7.75' 2	.0" Vert. Underdra .500 in/hr Exfiltrati	in C= 0.600	over Wetted area above -ln= 0.01'	∍ 57.75'
#4 #5	Device 1 Device 1		3.50' 4 3.50' 2	.0" Vert. Orifice/Gr	rate C= 0.600 Orifice/Grate C= 0		

Primary OutFlow Max=2.92 cfs @ 12.13 hrs HW=59.70' TW=55.31' (Dynamic Tailwater)

-1=Culvert (Passes 2.92 cfs of 11.56 cfs potential flow)

2=Underdrain (Passes 0.07 cfs of 0.87 cfs potential flow) **3=Exfiltration through Media** (Exfiltration Controls 0.07 cfs)

-4=Orifice/Grate (Orifice Controls 0.43 cfs @ 4.90 fps)

-5=Orifice/Grate (Weir Controls 2.41 cfs @ 1.48 fps)

Summary for Pond 69P: Bio #3 / OS #11

Inflow Area = 1.681 ac, 63.59% Impervious, Inflow Depth = 8.44" for 100-year event 15.15 cfs @ 12.08 hrs, Volume= 1.181 af Inflow

10.39 cfs @ 12.17 hrs, Volume= Outflow 1.157 af, Atten= 31%, Lag= 5.0 min

Primary 10.39 cfs @ 12.17 hrs, Volume= 1.157 af

5440-Post

Prepared by Altus Engineering, Inc.

Printed 10/6/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 48.00' Surf.Area= 1,706 sf Storage= 128 cf

Peak Elev= 52.72' @ 12.17 hrs Surf.Area= 5,702 sf Storage= 16,606 cf (16,478 cf above start)

Flood Elev= 53.00' Surf.Area= 6,019 sf Storage= 18,236 cf (18,109 cf above start)

Plug-Flow detention time= 181.4 min calculated for 1.154 af (98% of inflow)

Center-of-Mass det. time= 165.0 min (931.7 - 766.8)

Volume	Inve	rt Ava	il.Storage	Storage Descrip	tion	
#1	46.50)'	18,236 cf	Custom Stage	Data (Conic)Listed	below (Recalc)
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
46.5 48.0	50	1,706 1,706	0.0 5.0	0 128	0 128	1,706 1,926
49.0	00	2,381	100.0	2,034	2,162	2,619
50.0 51.0	00	3,056 3,935	100.0 100.0	2,711 3,486	4,874 8,360	3,319 4,223
52.0 53.0		4,918 6,019	100.0 100.0	4,417 5,459	12,777 18,236	5,233 6,365
Device	Routing	In	vert Out	let Devices		
#1	Primary	45		" Round Culver	-	
			Inle			Ke= 0.500 0.0050 '/' Cc= 0.900
#2	Device 1	45		Vert. Underdrair		
#3	Device 2	48				over Wetted area above 48.00'
#4 #5	Device 1 Device 1		0.00' 4.0' 2.50' 48.0	' W x 30.0" H Ver	a = 1,926 sf Phase- t. Orifice/Grate C= Orifice/Grate C= 0 low heads	= 0.600

Primary OutFlow Max=10.37 cfs @ 12.17 hrs HW=52.72' TW=46.10' (Dynamic Tailwater)

-1=Culvert (Passes 10.37 cfs of 21.65 cfs potential flow)

-2=Underdrain (Passes 0.23 cfs of 1.08 cfs potential flow)
-3=Exfiltration through Media (Exfiltration Controls 0.23 cfs)

-4=Orifice/Grate (Orifice Controls 4.69 cfs @ 5.63 fps)

-5=Orifice/Grate (Weir Controls 5.45 cfs @ 1.54 fps)

Summary for Pond 83P: Bio #4 / OS #22

Inflow Area = 0.333 ac, 76.36% Impervious, Inflow Depth = 8.94" for 100-year event

3.12 cfs @ 12.08 hrs, Volume= 0.248 af Inflow

2.77 cfs @ 12.13 hrs, Volume= Outflow 0.248 af, Atten= 11%, Lag= 2.5 min

Primary 2.77 cfs @ 12.13 hrs, Volume= 0.248 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Starting Elev= 45.50' Surf.Area= 315 sf Storage= 24 cf

Peak Elev= 48.69' @ 12.13 hrs Surf.Area= 1,543 sf Storage= 2,783 cf (2,759 cf above start)

Flood Elev= 49.00' Surf.Area= 1.695 sf Storage= 3.283 cf (3.260 cf above start)

Prepared by Altus Engineering, Inc.

Printed 10/6/2025

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Plug-Flow detention time= 141.4 min calculated for 0.247 af (100% of inflow)

Center-of-Mass det. time= 137.0 min (894.3 - 757.3)

Volume	Inver	t Ava	il.Storage	Storage Descrip	otion	
#1	44.00	'	3,283 cf	Custom Stage	Data (Prismatic	Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
44.0	00	315	0.0	0	0	
45.5	50	315	5.0	24	24	
46.0	00	455	100.0	193	216	
47.0	00	789	100.0	622	838	
48.0		1,203	100.0	996	1,834	
49.0	00	1,695	100.0	1,449	3,283	
Device	Routing	In	vert Ou	tlet Devices		
#1	Primary	43	L= Inle	.0" Round Culver 26.0' CPP, squaret / Outlet Invert= 4 0.012, Flow Area	re edge headwall 43.00' / 42.85' S	, Ke= 0.500 = 0.0058 '/' Cc= 0.900
#2	Device 1	43		" Vert. Underdrai		
#3	Device 2	45	5.50' 2.5	00 in/hr Exfiltration	on through Med	ia over Surface area above 45.50'
			Ex	cluded Surface are	ea = 315 sf Phas	e-In= 0.01'
#4	Device 1	47	-	" Vert. Orifice/Gr		
#5	Device 1	48		.0" x 24.0" Horiz. Inited to weir flow a		= 0.600

Primary OutFlow Max=2.76 cfs @ 12.13 hrs HW=48.69' TW=43.05' (Dynamic Tailwater)

-1=Culvert (Passes 2.76 cfs of 13.30 cfs potential flow)

⁻²⁼Underdrain (Passes 0.07 cfs of 0.99 cfs potential flow)
-3=Exfiltration through Media (Exfiltration Controls 0.07 cfs)

⁻⁴⁼Orifice/Grate (Orifice Controls 0.51 cfs @ 5.85 fps)

⁻⁵⁼Orifice/Grate (Weir Controls 2.18 cfs @ 1.43 fps)

Section 5

Precipitation Table



Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

	Metadata for Point										
Smoothing	Yes										
State	New Hampshire										
Location	Location New Hampshire, United States										
Latitude	43.141 degrees North										
Longitude	70.907 degrees West										
Elevation	10 feet										
Date/Time	Tue Apr 15 2025 15:34:24 GMT-0400 (Eastern Daylight Time)										

Extreme Precipitation Estimates

	are the first process and the first process and the first process are the first process										- 1									
	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12h	· 24hr	+15%	1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.03							2.61	2	2.31	2.74	3.14	3.87	4.44	1yr
2yr	0.32	0.49	0.61	0.81	1.01	1.29	2yr	0.88	1.17	1.50	1.91	2.45	3.14	3.61	2.78	3.35	3.85	4.58	5.21	2yr
5yr	0.37	0.57	0.72	0.96	1.23	1.58	5yr	1.06	1.44	1.85	2.39	3.08	3.98	A	3.52	4.30	4.91	5.80	6.56	5yr
10yr	0.40	0.63	0.80	1.09	1.42	1.85	10yr	1.22	1.69	2.18	2.83	3.67	4.76	5.47	4.21	5.19	5.91	6.93	7.81	10yr
25yr	0.46	0.74	0.94	1.29	1.72	2.27	25yr	1.48	2.09	2.70	3.53	4.62	6.03	6.93	5.34	6.66	7.55	8.78	9.83	25yr
50yr	0.51	0.83	1.06	1.48	1.99	2.66	50yr	1.72	2.46	3.18	4.19	5.51	7.22	8.30	6.39	8.05	9.09	10.51	11.72	50yr
100yı	0.58	0.93	1.20	1.70	2.31	3.12	100yr	2.00	2.89	3.74	4.97	6.56	8.64	9.94	r 7.65	9.73	10.94	12.58	13.96	100yr
200yı	0.64	1.04	1.35	1.94	2.69	3.66	200yr	2.32	3.40	4.42	5.90	7.83	10.35	12.24 200 ₅	yr 9.16	11.77	13.18	15.07	16.65	200yr
500yı	0.75	1.24	1.61	2.34	3.29	4.52	500yr	2.84	4.22	5.49	7.39	9.88	13.15	15.74 500 <u>y</u>	yr 11.64	15.14	16.87	19.14	21.03	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.24	0.37	0.45	0.60	0.74	0.90	1yr	0.64	0.88	0.91	1.27	1.58	2.06	2.50	1yr	1.82	2.41	2.90	3.27	4.00	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.18	2yr	0.86	1.16	1.36	1.83	2.36	3.04	3.39	2yr	2.69	3.26	3.74	4.46	5.05	2yr
5yr	0.35	0.54	0.67	0.91	1.16	1.40	5yr	1.00	1.37	1.61	2.15	2.78	3.72	4.13	5yr	3.29	3.98	4.60	5.43	6.14	5yr
10yr	0.38	0.59	0.73	1.02	1.32	1.60	10yr	1.14	1.56	1.82	2.45	3.13	4.30	4.81	10yr	3.80	4.63	5.34	6.29	7.07	10yr
25yr	0.44	0.66	0.83	1.18	1.55	1.91	25yr	1.34	1.87	2.11	2.84	3.65	4.98	5.85	25yr	4.41	5.63	6.53	7.66	8.54	25yr
50yr	0.48	0.73	0.91	1.31	1.76	2.18	50yr	1.52	2.13	2.36	3.20	4.10	5.70	6.77	50yr	5.05	6.51	7.60	8.87	9.83	50yr
100yr	0.54	0.81	1.02	1.47	2.02	2.50	100yr	1.74	2.44	2.64	3.58	4.58	6.51	7.84	100yr	5.76	7.54	8.86	10.29	11.30	100yr
200yr	0.60	0.90	1.14	1.65	2.29	2.86	200yr	1.98	2.79	2.94	4.01	5.12	7.43	9.08	200yr	6.57	8.73	10.33	11.94	13.02	200yr
500yr	0.70	1.04	1.33	1.94	2.75	3.44	500yr	2.38	3.36	3.41	4.64	5.95	8.82	11.01	500yr	7.80	10.59	12.66	14.55	15.64	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.43	0.53	0.71	0.88	1.08	1yr	0.76	1.05	1.24	1.75	2.22	2.86	3.05	1yr	2.53	2.93	3.40	4.20	4.80	1yr
2yr	0.33	0.51	0.62	0.85	1.04	1.25	2yr	0.90	1.22	1.47	1.96	2.51	3.28	3.59	2yr	2.90	3.45	3.96	4.72	5.42	2yr
5yr	0.39	0.60	0.75	1.03	1.31	1.58	5yr	1.13	1.55	1.86	2.50	3.20	4.24	4.79	5yr	3.75	4.61	5.24	6.18	6.95	5yr
10yr	0.46	0.70	0.87	1.21	1.57	1.92	10yr	1.35	1.88	2.23	3.05	3.86	5.22	5.97	10yr	4.62	5.75	6.53	7.59	8.48	10yr
25yr	0.55	0.84	1.05	1.50	1.97	2.48	25yr	1.70	2.42	2.88	3.97	4.95	7.11	8.00	25yr	6.29	7.70	8.67	9.98	11.05	25yr
50yr	0.64	0.97	1.21	1.74	2.34	3.00	50yr	2.02	2.93	3.49	4.85	6.01	8.81	10.00	50yr	7.79	9.62	10.77	12.26	13.51	50yr
100yr	0.74	1.12	1.41	2.03	2.79	3.62	100yr	2.41	3.54	4.24	5.93	7.31	10.91	12.50	100yr	9.65	12.02	13.36	15.09	16.54	100yr
200yr	0.86	1.30	1.64	2.38	3.32	4.39	200yr	2.86	4.29	5.15	7.26	8.87	13.55	15.65	200yr	11.99	15.05	16.59	18.56	20.27	200yr
500yr	1.05	1.57	2.01	2.93	4.16	5.64	500yr	3.59	5.52	6.65	9.52	11.49	18.09	21.05	500yr	16.01	20.24	22.09	24.44	26.53	500yr

Section 6

NRCS Soils Report Site Specific Soil Survey Geotechnical Report

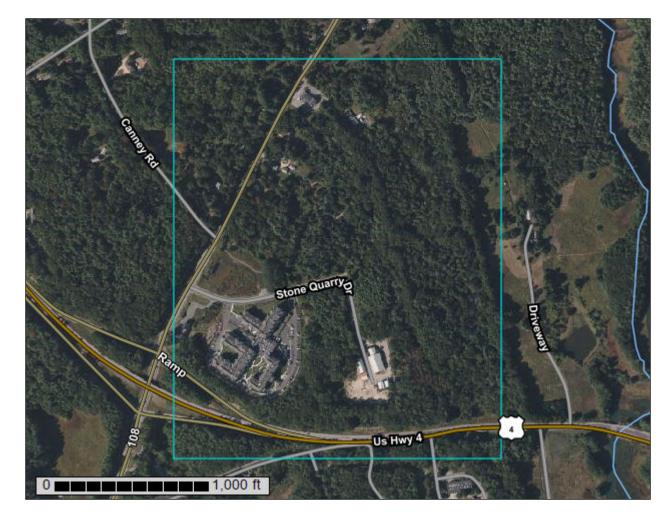




Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for **Strafford County, New Hampshire**



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
Soil Map	
Soil Map	6
Legend	7
Map Unit Legend	8
Map Unit Descriptions	
Strafford County, New Hampshire	
BzB—Buxton silt loam, 3 to 8 percent slopes	10
GIC—Gloucester fine sandy loam, 8 to 15 percent slopes	11
HcB—Hollis-Charlton fine sandy loams, 3 to 8 percent slopes	12
HdC—Hollis-Charlton very rocky fine sandy loams, 8 to 15 percent	
slopes	13
ScA—Scantic silt loam, 0 to 3 percent slopes	15
ScB—Scantic silt loam, 3 to 8 percent slopes	16
SfC—Suffield silt loam, 8 to 15 percent slopes	
Ta—Tidal marsh	19
W—Water	20

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND

Area of Interest (AOI)

Soils

Area of Interest (AOI)



Soil Map Unit Lines

◁



Soil Map Unit Points

Special Line Features

Streams and Canals

Nater Features

Special Point Features

Blowout 9

Borrow Pit

Closed Depression Clay Spot

Gravel Pit

Interstate Highways

Rails

ŧ

Fransportation

Major Roads Local Roads

US Routes

Gravelly Spot

Lava Flow

Landfill

Marsh or swamp

Aerial Photography

3ackground

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip Sodic Spot

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Very Stony Spot

8

Soil Map Unit Polygons

Wet Spot Other

Stony Spot

Spoil Area

W

contrasting soils that could have been shown at a more detailed Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of

Please rely on the bar scale on each map sheet for map measurements.

Coordinate System: Web Mercator (EPSG:3857) Web Soil Survey URL:

Source of Map: Natural Resources Conservation Service

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Strafford County, New Hampshire Version 25, Sep 3, 2024 Survey Area Data: Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Jun 19, 2020—Sep 20, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BzB	Buxton silt loam, 3 to 8 percent slopes	45.4	35.7%
GIC	Gloucester fine sandy loam, 8 to 15 percent slopes	0.9	0.7%
НсВ	Hollis-Charlton fine sandy loams, 3 to 8 percent slopes	36.0	28.3%
HdC	Hollis-Charlton very rocky fine sandy loams, 8 to 15 percent slopes	0.7	0.6%
ScA	Scantic silt loam, 0 to 3 percent slopes	36.3	28.5%
ScB	Scantic silt loam, 3 to 8 percent slopes	3.0	2.3%
SfC	Suffield silt loam, 8 to 15 percent slopes	4.2	3.3%
Та	Tidal marsh	0.0	0.0%
W	Water	0.8	0.6%
Totals for Area of Interest		127.3	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the

scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Strafford County, New Hampshire

BzB—Buxton silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9d6p

Elevation: 0 to 260 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Buxton and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Buxton

Setting

Parent material: Glaciomarine

Typical profile

H1 - 0 to 10 inches: silt loam
H2 - 10 to 28 inches: silty clay loam
H3 - 28 to 43 inches: silty clay

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Moderately well drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.20 in/hr)

Depth to water table: About 12 to 24 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C/D

Ecological site: F145XY006CT - Semi-Rich Moist Lake Plain

Hydric soil rating: No

Minor Components

Elmwood

Percent of map unit: 10 percent

Hydric soil rating: No

Not named

Percent of map unit: 5 percent

GIC—Gloucester fine sandy loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9d74

Elevation: 20 to 970 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Gloucester and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Gloucester

Setting

Parent material: Till

Typical profile

H1 - 0 to 14 inches: fine sandy loam

H2 - 14 to 28 inches: very gravelly loamy sand H3 - 28 to 40 inches: very gravelly coarse sand

Properties and qualities

Slope: 8 to 15 percent

Depth to restrictive feature: More than 80 inches Drainage class: Somewhat excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00

to 20.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 3.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: A

Ecological site: F144AY032NH - Dry Till Uplands

Hydric soil rating: No

Minor Components

Not named pan

Percent of map unit: 5 percent

Hollis

Percent of map unit: 5 percent

Hydric soil rating: No

Acton

Percent of map unit: 5 percent

Hydric soil rating: No

HcB—Hollis-Charlton fine sandy loams, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9d7j Elevation: 0 to 1,020 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 120 to 240 days

Farmland classification: Farmland of local importance

Map Unit Composition

Hollis and similar soils: 55 percent Charlton and similar soils: 35 percent Minor components: 10 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hollis

Setting

Parent material: Till

Typical profile

H1 - 0 to 14 inches: fine sandy loam H2 - 14 to 18 inches: bedrock

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: 10 to 20 inches to lithic bedrock

Drainage class: Well drained Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.60 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Very low (about 2.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: D

Ecological site: F144AY033MA - Shallow Dry Till Uplands

Description of Charlton

Setting

Parent material: Till

Typical profile

H1 - 0 to 13 inches: fine sandy loam
H2 - 13 to 36 inches: fine sandy loam
H3 - 36 to 40 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 5.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: A

Ecological site: F144AY034CT - Well Drained Till Uplands

Hydric soil rating: No

Minor Components

Not named

Percent of map unit: 5 percent

Hydric soil rating: No

Buxton

Percent of map unit: 5 percent

Hydric soil rating: No

HdC—Hollis-Charlton very rocky fine sandy loams, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9d7n

Elevation: 0 to 1,200 feet

Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 120 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Hollis and similar soils: 40 percent Charlton and similar soils: 30 percent Minor components: 30 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hollis

Setting

Parent material: Till

Typical profile

H1 - 0 to 14 inches: very stony fine sandy loam

H2 - 14 to 18 inches: bedrock

Properties and qualities

Slope: 8 to 15 percent

Surface area covered with cobbles, stones or boulders: 1.6 percent

Depth to restrictive feature: 10 to 20 inches to lithic bedrock

Drainage class: Well drained Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.60 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Very low (about 2.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: D

Ecological site: F144AY033MA - Shallow Dry Till Uplands

Hydric soil rating: No

Description of Charlton

Setting

Parent material: Till

Typical profile

H1 - 0 to 13 inches: very stony fine sandy loam

H2 - 13 to 36 inches: fine sandy loam H3 - 36 to 40 inches: gravelly loamy sand

Properties and qualities

Slope: 8 to 15 percent

Surface area covered with cobbles, stones or boulders: 1.6 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 5.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: A

Ecological site: F144AY034CT - Well Drained Till Uplands

Hydric soil rating: No

Minor Components

Rock outcrop

Percent of map unit: 10 percent

Hydric soil rating: No

Not named

Percent of map unit: 10 percent

Hydric soil rating: No

Woodbridge

Percent of map unit: 5 percent

Hydric soil rating: No

Suttor

Percent of map unit: 5 percent

Hydric soil rating: No

ScA—Scantic silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 9d8s

Elevation: 0 to 260 feet

Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Farmland of local importance

Map Unit Composition

Scantic and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scantic

Setting

Landform: Marine terraces

Typical profile

H1 - 0 to 13 inches: silt loam

H2 - 13 to 23 inches: silty clay loam H3 - 23 to 40 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Poorly drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

high (0.00 to 0.20 in/hr)

Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 6.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D

Ecological site: F144AY019NH - Wet Lake Plain

Hydric soil rating: Yes

Minor Components

Swanton

Percent of map unit: 5 percent Landform: Marine terraces Hydric soil rating: Yes

Not named wet

Percent of map unit: 5 percent Landform: Marine terraces Hydric soil rating: Yes

Biddeford

Percent of map unit: 5 percent Landform: Marine terraces Hydric soil rating: Yes

ScB—Scantic silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9d8t

Elevation: 0 to 260 feet

Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Farmland of local importance

Map Unit Composition

Scantic and similar soils: 85 percent *Minor components:* 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scantic

Setting

Landform: Marine terraces

Typical profile

H1 - 0 to 13 inches: silt loam
H2 - 13 to 23 inches: silty clay loam
H3 - 23 to 40 inches: silty clay

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Poorly drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

high (0.00 to 0.20 in/hr)

Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 6.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D

Ecological site: F144AY019NH - Wet Lake Plain

Hydric soil rating: Yes

Minor Components

Swanton

Percent of map unit: 5 percent Landform: Marine terraces Hydric soil rating: Yes

Not named wet

Percent of map unit: 5 percent Landform: Marine terraces Hydric soil rating: Yes

Buxton

Percent of map unit: 5 percent Hydric soil rating: No

SfC—Suffield silt loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9d8v

Elevation: 0 to 250 feet

Mean annual precipitation: 36 to 71 inches

Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Suffield and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Suffield

Typical profile

H1 - 0 to 19 inches: silt loam H2 - 19 to 28 inches: silt loam H3 - 28 to 41 inches: silty clay

Properties and qualities

Slope: 8 to 15 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: High

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

high (0.00 to 0.20 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: C

Ecological site: F144AY017NH - Well Drained Lake Plain

Hydric soil rating: No

Minor Components

Not named

Percent of map unit: 9 percent

Hydric soil rating: No

Buxton

Percent of map unit: 5 percent

Hydric soil rating: No

Rock outcrop

Percent of map unit: 1 percent

Ta—Tidal marsh

Map Unit Setting

National map unit symbol: 9d92

Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Tidal marsh: 98 percent Minor components: 2 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tidal Marsh

Setting

Landform: Marshes

Typical profile

H1 - 0 to 12 inches: peat

H2 - 12 to 46 inches: mucky peat H3 - 46 to 50 inches: silt loam H4 - 50 to 60 inches: loamy sand

Properties and qualities

Slope: 0 to 1 percent

Drainage class: Very poorly drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very

high (0.60 to 20.00 in/hr)

Depth to water table: About 0 inches

Frequency of flooding: Frequent

Frequency of ponding: Frequent

Maximum salinity: Very slightly saline to strongly saline (2.0 to 32.0 mmhos/cm)

Available water supply, 0 to 60 inches: Very high (about 14.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 8w

Hydric soil rating: Yes

Minor Components

Rock outcrop

Percent of map unit: 2 percent

W-Water

Map Unit Composition

Water (less than 40 acres): 100 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Michael Cuomo, Soil Scientist

6 York Pond Road, York, Maine 03909 207 363 4532

mcuomosoil@gmail.com

Erik Saari, Vice President Altus Engineering, Inc. 133 Court Street Portsmouth, NH 03801

24 February 2024

Dear Mr. Saari;

This report is in reference to the 22.5 acre undeveloped parcel located north of Stone Quarry Drive and east of Route 108 in Durham, NH. This is identified by the town as map 209, lot 33 and is owned by Riverwoods, Durham. A Site Specific Soil Map was prepared for this property in February 2024 to assist you in planning and permitting a residential development served by municipal sewer and water service. There were a few patches of thin snow and the wetlands were mostly iced over when this work was done.

This report is a component of the Site Specific Soil Survey and must be submitted with it for regulatory review. The both comply with 'Site Specific Soil Mapping Standards for New Hampshire and Vermont', SSSNNE Special Publication No.3 version 7.0, July 2021.

The Soil Survey is comprised of two components: a soil map and this report. The soil map is made by traversing the property and observing the soil at locations thought to be representative of the landform. Because soil is highly variable, it is not possible to represent every soil variation on a soil map. The soil map is a simplified two-dimensional interpretation of the complex three dimensional soil-landscape relationship. Standards allow for inclusions of soils other than those named in the soil map unit label, so long as dissimilar inclusions do not exceed 25% of the map unit. Boundaries between different soils are shown as lines on the soil map, but are generally diffuse transition zones.

Soil names were selected using best fit with soils already researched in detail and listed in the 'New Hampshire State-Wide Numerical Soils Legend' issued in 2011 by the USDA Natural Resource Conservation Service. The map unit design and the interpretations of soil properties in this report are specific to this site and were selected considering the proposed use. Therefore, the interpretations and limitation information in this report may be insufficient for some other uses. Hydrologic soil groups were assigned using 'KSAT Values for New Hampshire Soils' SSSNNE Special Publication No. 5, September 2009.

Twenty eight test pits were dug with an excavator and are the primary basis of this soil survey. Other soil observations were made with a soil auger, and two were recorded in detail to document the wetland soils. The test pit data and soil observations are attached at the rear of this report. Eight soil types were identified on this property and are described below. The regulated wetland boundaries were previously flagged and have been survey located.

Map Unit: 29

Soil Name: Woodbridge

Drainage class: moderately well drained.

Parent material: basal till.

Texture range: fine sandy loam and sandy loam.

Described in: test pit MC6.

Landscape position: upland sideslopes.

Landform: gently rolling to steep.

Slope range: 1 to 25%.

Bedrock class: very deep (more than 60").

Hydrologic group: C.

Flood hazard: none.

Inclusions: test pit MC12 is a non limiting inclusion in which the firm subsoil occurs deeper than is typical.

Use and management: land uses are slightly limited by the presence of basal till, which causes the ground water to perch after rain events and snow melt. This limitation can be overcome by appropriate grading and drainage.

Map Unit: 32

Soil Name: Boxford

Drainage class: moderately well drained.

Parent material: marine.

Texture range: silt loam over silty clay loam.

Described in: test pit MC10.

Landscape position: gently rolling uplands.

Landform: terraces. Slope range: 1 to 15%.

Bedrock class: very deep (more than 60").

Hydrologic group: C.

Flood hazard: none.

Inclusions: test pit MC2 is a limiting inclusion of Boxford-like soils with bedrock between 48 and 60 inches deep. Test pit MC4 is a non-limiting inclusion of Boxford-like soils underlain by glacial till.

Use and management: land uses are limited by seasonal wetness close to the surface. These are not wetland soils and may be drained or filled to overcome this limitation. Boxford soils are highly erodible and difficult to revegetate. These soils have low bearing strength when saturated.

Map Unit: 33

Soil Name: Scitico

Drainage class: poorly drained.

Parent material: marine.

Texture range: silt loam over silty clay loam.

Described in: soil observation A.

Landscape position: nearly level to gently sloping wetlands.

Landform: glacial deltas, bays.

Slope range: 1 to 8%.

Bedrock class: very deep (more than 60").

Hydrologic group: C.

Flood hazard: undefined flood hazard may exist along small stream channels.

Inclusions: Scitico map units include small stream channels, which are limiting inclusions. Scitico map units include small areas with stony and bouldery surface layers, which is not typical. Limited historical ditching and filling for trails has occurred.

Use and management: land uses are limited by frequent saturation to the surface, low bearing strength, and fine textures, which makes it difficult to work the soil when wet. These are regulated wetlands and may be not be drained or filled without permits.

Map Unit: 42

Soil Name: Canton

Drainage class: well drained.

Parent material: loose glacial till.

Texture range: fine sandy loam over loamy sand.

Described in: test pit MC1.

Landscape position: upland.

Landform: ground moraine.

Slope range: 1-15%.

Bedrock class: very deep (more than 60").

Hydrologic group: B.

Flood hazard: none.

Inclusions: none observed.

Use and management: this soil presents no limitations to development.

Map Unit: 85

Soil Name: Hollis

Drainage class: somewhat excessively drained.

Parent material: loose glacial till.

Texture range: fine sandy loam.

Described in: test pit MC15.

Landscape position: rolling upland.

Landform: scoured ridges.

Slope range: 8 to 25%.

Bedrock class: shallow (10-20").

Hydrologic group: D.

Flood hazard: none.

Inclusions: bedrock outcroppings are found in this map unit and are a limiting inclusion.

Use and management: land uses are limited by bedrock 0 to 20 inches from the soil surface, which can be overcome by blasting and filling.

Map Unit: 89

Soil Name: Chatfield

Drainage class: well drained.

Parent material: loose glacial till.

Texture range: fine sandy loam.

Described in: test pit MC8.

Landscape position: upland.

Landform: scoured ridges.

Slope range: 1 to 8%.

Bedrock class: moderately deep (20-40").

Hydrologic group: B.

Flood hazard: none.

Inclusions: none noted.

Use and management: land uses are limited by bedrock 20 to 40 inches from the soil surface, which can be overcome by blasting and filling.

Map Unit: 134

Soil Name: Maybid

Drainage class: very poorly drained.

Parent material: marine.

Texture range: silt loam over silty clay loam.

Described in: soil observation B.

Landscape position: lowland wetlands.

Landform: deltas, bays.

Slope range: 0 to 1%.

Bedrock class: very deep (more than 60").

Hydrologic group: D.

Flood hazard: undefined flood hazard may exist.

Inclusions: none noted.

Use and management: Land uses are very limited by long periods of saturation to the surface, low bearing strength, and fine soil texture. These are regulated wetlands and may be not be drained or filled without permits.

Map Unit: 953

Soil Name: Boxford SPD

The Boxford SPD (SPD for 'somewhat poorly drained') is very similar to the Boxford soils, which are moderately well drained.

Drainage class: somewhat poorly drained.

Parent material: marine.

Texture range: silt loam over silty clay loam.

Described in: test pit MC3.

Landscape position: lowland.

Landform: terrace.

Slope range: 1 to more than 25%.

Bedrock class: very deep (more than 60").

Hydrologic group: C.

Flood hazard: none.

Inclusions: test pits MC17 and MC27 are Boxford SPD non-limiting variants which are underlain by firm glacial till. Test pit MC26 found bedrock between 48 and 60 inches, which is limiting for some land uses.

Test pits MC22, MC23, and MC24 show a non-limiting inclusion with a thin layer of fill over subsoils typical to Boxford SPD and Boxford soils. This inclusion appears widespread in the south west corner of the property, but could not be accurately separated as a map unit. It was included in the Boxford SPD because it is also somewhat poorly drained. Other areas of Boxford SPD map units are covered with debris from past human occupation, such as broken concrete and brick piles.

Use and management: land uses are limited by seasonal wetness close to the surface. These are not wetland soils and may be drained or filled to overcome this limitation. Boxford SPD soils are highly erodible and difficult to revegetate. These soils have low bearing strength when saturated.

Please call if you have questions regarding this work.

Sincerely,

Michael Cuomo

NH Certified Soil Scientist #6

The following must appear on the soil map

Site Specific Soil Map Legend

		HYDROLOGIC	
MAP	SOIL	SOIL	DRAINAGE
SYMBOL	NAME	GROUP	CLASS
29	Woodbridge	С	moderately well drained
32	Boxford	С	moderately well drained
33	Scitico	С	poorly drained
42	Canton	В	well drained
85	Hollis	D	somewhat excessively drained
89	Chatfield	В	well drained
134	Maybid	D	very poorly drained
953	Boxford SPD	С	somewhat poorly drained

The letter at the end of the soil map unit label is the slope class. This represents the predominant slope of the map unit.

Slope Class

A 0-1%

B 1-8%

C 8-15%

D 15-25%

E +25%

This map is within the technical standards of the National Cooperative Soil Survey. It is a special purpose product intended for development planning and engineering interpretations. It was produced by a professional soil scientist, and is not a product of the USDA Natural Resources Conservation Service. There is a report that accompanies this map.

Michael Cuomo NH Cert. Soil Scientist #6 February 2024

Michael Cuomo, Soil Scientist

6 York Pond Road, York, Maine 03909 207 363 4532 mcuomosoil@gmail.com

TEST PIT DATA

Client: Altus Engineering

Location: Stone Quarry Drive, Durham, NH

Date: 12 and 14 February 2024

Test Pit Number: MC1

Depth
3" Description
Forest litter.

0-8" Dark brown (10YR 3/3) fine sandy loam, granular, friable.

8-18" Brownish yellow (10YR 6/6) fine sandy loam, blocky,

friable.

18-60" Light yellowish brown (2.5Y 6/3) loamy sand, blocky,

friable.

Depth to Seasonal High Water Table: none

Observed water: none Depth to bedrock: none

Soil name: Canton

Test Pit Number: MC2

0-10" Dark yellowish brown (10YR 3/4) silt loam, granular,

friable.

10-15" Yellowish brown (10YR 5/6) silt loam, blocky, friable.

15-24" Olive brown (2.5Y 4/3) silt loam, blocky, friable, redox.

24-50 Olive (5Y 4/3) silty clay loam, massive, firm, redox.

50"+ Bedrock.

Depth to Seasonal High Water Table: 15"

Observed water: none Depth to bedrock: 50"

Soil name: Boxford variant

Test Pit Number: MC3

0-8" Dark olive brown (2.5Y 3/3) silt loam, granular, friable.

8-10" Light olive brown (2.5Y 5/4) silt loam, blocky, friable.

10-14" Light yellowish brown (2.5Y 6/4) silt loam, blocky, friable, redox.

14-54 Olive (5Y 5/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 10"

Observed water: 12"
Depth to bedrock: none
Soil name: Boxford SPD

Percolation test: 22 minutes/inch at 9" depth

Depth Description

2" Forest litter.

0-8" Dark brown (10YR 3/3) silt loam, granular, friable.

8-15" Yellowish brown (10YR 5/6) silt loam, blocky, friable.

15-19" Light olive brown (2.5Y 5/4) silt loam, blocky, friable, redox.

19-40" Grayish brown (5Y 5/2) silty clay loam, massive, firm, redox.

40-60" Olive brown (2.5Y 4/4) stony fine sandy loam, massive, friable, redox.

Depth to Seasonal High Water Table: 15"

Observed water: 18" Depth to bedrock: none Soil name: Boxford variant

Test Pit Number: MC5

Description Depth

Forest litter.

0-7" Dark brown (10YR 3/3) fine sandy loam, granular, friable.

7-12" Brownish yellow (10YR 6/6) fine sandy loam, blocky, friable.

12-24" Light olive brown (2.5Y 5/6) loamy sand, blocky, friable.

24-60" Light yellowish brown (2.5Y 6/3) loamy sand, blocky, friable.

Depth to Seasonal High Water Table: none

Observed water: none Depth to bedrock: none

Soil name: Canton

Test Pit Number: MC6

Description <u>Depth</u> 3 "

Forest litter.

0-8" Dark brown (10YR 3/3) stony fine sandy loam, granular, friable.

8-16" Yellowish brown (10YR 5/4) stony fine sandy loam, blocky, friable.

16-21" Yellowish brown (10YR 5/6) stony sandy loam, blocky, friable.

21-55" Light olive brown (2.5Y 5/4) stony sandy loam, massive, firm, redox.

Depth to Seasonal High Water Table: 21"

Observed water: 50" Depth to bedrock: none Soil name: Woodbridge

<u>Depth</u> <u>Description</u>

1" Forest litter.

0-9" Very dark grayish brown (2.5Y 3/2) silt loam, granular, friable.

9-12" Light olive brown (2.5Y 5/4) silt loam, blocky, friable.

12-16" Light yellowish brown (2.5Y 6/3) silt loam, blocky, friable, redox.

16-60 Olive gray (5Y 5/2) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 12"

Observed water: none Depth to bedrock: none Soil name: Boxford SPD

Test Pit Number: MC8

<u>Depth</u> <u>Description</u>

3" Forest litter.

0-8" Very dark brown (10YR 2/2) fine sandy loam, granular, friable.

8-25" Dark yellowish brown (10YR 4/6) stony fine sandy loam, blocky, friable.

25-30" Light yellowish brown (2.5Y 6/4) stony fine sandy loam, blocky, friable.

Depth to Seasonal High Water Table: none

Observed water: none Depth to bedrock: 30" Soil name: Chatfield

Percolation test: 10 minutes/inch at 16" depth

Test Pit Number: MC9

<u>Depth</u> <u>Description</u>

2" Forest litter.

0-6" Dark brown (10YR 3/3) stony fine sandy loam, granular, friable.

6-21" Brownish yellow (10YR 6/6) stony fine sandy loam, blocky, friable.

21-55" Light yellowish brown (2.5Y 6/4) loamy sand, blocky, friable.

Depth to Seasonal High Water Table: none

Observed water: none Depth to bedrock: none

Soil name: Canton

Depth Description

0-14" Very dark grayish brown (10YR 3/2) silt loam, granular, friable.

14-20" Yellowish brown (10YR 5/6) silt loam, blocky, friable.

20-25" Light olive brown (2.5Y 5/4) silt loam, blocky, friable, redox.

25-56" Olive (5Y 4/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 20"

Observed water: 25"
Depth to bedrock: none
Soil name: Boxford

Test Pit Number: MC11

Depth
1" Description
Forest litter.

0-8" Very dark grayish brown (2.5Y 3/2) silt loam, granular, friable.

8-10" Light olive brown (2.5Y 5/4) silt loam, blocky, friable.

10-17" Light yellowish brown (2.5Y 6/3) silt loam, blocky, friable, redox.

17-60" Olive (5Y 5/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 10"

Observed water: none Depth to bedrock: none Soil name: Boxford SPD

Test Pit Number: MC12

0-14" Very dark brown (10YR 2/2) fine sandy loam, granular, friable.

14-28" Yellowish brown (10YR 5/6) fine sandy loam, blocky, friable.

28-44" Olive brown (2.5Y 4/4) stony fine sandy loam, blocky, friable, redox.

44-60" Olive brown (2.5Y 4/3) stony fine sandy loam, massive, firm, redox.

Depth to Seasonal High Water Table: 28"

Observed water: 40"
Depth to bedrock: none

Soil name: Woodbridge variant

Test Pit Number: MC13

<u>Depth</u>

<u>Description</u>

2" Forest litter.

- 0-9" Very dark grayish brown (10YR 3/2) stony fine sandy loam, granular, friable.
- 9-16" Dark yellowish brown (10YR 4/6) stony fine sandy loam, blocky, friable.
- 16-24" Light yellowish brown (2.5Y 6/4) stony fine sandy loam, blocky, friable, redox.
- 24-60" Olive brown (2.5Y 4/4) stony sandy loam, massive, firm, redox.

Depth to Seasonal High Water Table: 16"

Observed water: 24"
Depth to bedrock: none
Soil name: Woodbridge

Percolation test: 12 minutes/inch at 15" depth

Test Pit Number: MC14

Depth
1" Description
Forest litter.

- 0-9" Very dark grayish brown (2.5Y 3/2) silt loam, granular, friable.
- 9-12" Light olive brown (2.5Y 5/4) silt loam, blocky, friable, redox.
- 12-20" Olive brown (2.5Y 4/3) silt loam, blocky, firm, redox.
- 20-60" Olive gray (5Y 5/2) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 9"

Observed water: 18"
Depth to bedrock: none
Soil name: Boxford SPD

Test Pit Number: MC15

Depth
1" Description
1 Forest litter.

- 0-8" Very dark grayish brown (10YR 3/2) bouldery fine sandy loam, granular, friable.
- 8-20" Yellowish brown (10YR 5/6) bouldery fine sandy loam, blocky, friable.

20"+ Bedrock.

Depth to Seasonal High Water Table: none

Observed water: none Depth to bedrock: 20" Soil name: Hollis Test Pit Number: MC16

Depth
Description
The Porest litter.

0-8" Very dark grayish brown (2.5Y 3/2) silt loam, granular, friable.

8-12" Light olive brown (2.5Y 5/4) silt loam, blocky, friable.

12-16" Olive brown (2.5Y 4/4) silt loam, blocky, friable, redox.

16-60" Olive gray (5Y 5/2) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 12"

Observed water: none Depth to bedrock: none Soil name: Boxford SPD

Test Pit Number: MC17

Depth
1" Description
Forest litter.

0-9" Dark brown (10YR 3/3) silt loam, granular, friable.

9-11" Light olive brown (2.5Y 5/4) silt loam, blocky, friable.

11-13" Olive brown (2.5Y 4/4) silt loam, blocky, friable, redox.

13-32" Olive gray (5Y 5/2) silty clay loam, massive, firm, redox.

32-60" Olive brown (2.5Y 4/4) stony fine sandy loam, massive, firm, redox.

Depth to Seasonal High Water Table: 11"

Observed water: 34"
Depth to bedrock: none

Soil name: Boxford SPD variant

Test Pit Number: MC18

Depth
2" Description
2 Forest litter.

0-9" Dark brown (10YR 3/3) fine sandy loam, granular, friable.

9-24" Yellowish brown (10YR 5/6) fine sandy loam, blocky,

24-66" Light olive brown (2.5Y 5/4) stony fine sandy loam, blocky, friable, redox.

Depth to Seasonal High Water Table: 24"

Observed water: none Depth to bedrock: none Soil name: Woodbridge Test Pit Number: MC19

Depth Description

0-11" Very dark grayish brown (10YR 3/2) very fine sandy loam, granular, friable.

11-19" Strong brown (7.5YR 5/6) very fine sandy loam, blocky, friable.

19-22" Light olive brown (2.5Y 5/4) silt loam, blocky, fredox.

22-56" Olive (5Y 4/4) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 19"

Observed water: 50"
Depth to bedrock: none
Soil name: Boxford

Test Pit Number: MC20 <u>Depth</u> <u>Description</u>

1" Forest litter.

0-9" Very dark grayish brown (10YR 3/2) silt loam, granular, friable.

9-11" Olive brown (2.5Y 4/4) silt loam, blocky, friable, redox.

11-18" Grayish brown (2.5Y 5/2) silt loam, blocky, friable, redox.

18-60" Olive gray (5Y 5/2) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 9"

Observed water: 30"
Depth to bedrock: none
Soil name: Boxford SPD

Test Pit Number: MC21

Depth
1" Description
Forest litter.

0-6" Dark brown (10YR 3/3) silt loam, granular, friable.

6-11" Light olive brown (2.5Y 5/4) silt loam, blocky, friable.

11-16" Grayish brown (2.5Y 5/2) silt loam, blocky, friable, redox.

16-55" Olive (5Y 4/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 11"

Observed water: 20"
Depth to bedrock: none
Soil name: Boxford SPD

Percolation test: 58 minutes/inch at 10" depth

Depth Description

0-6" Dark olive brown (2.5Y 3/3) fine sandy loam fill, massive, friable.

6-15" Light yellowish brown (2.5Y 6/3) gravelly loamy sand fill, massive, friable.

15-24" Dark grayish brown (2.5Y 4/2) silt loam, blocky, friable, redox.

24-55" Olive (5Y 4/4) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 15"

Observed water: none Depth to bedrock: none

Soil name: Excavated and filled over Boxford or Boxford SPD

Test Pit Number: MC23

<u>Depth</u> <u>Description</u>

0-8" Dark brown (10YR 3/3) fine sandy loam fill, granular, friable.

8-12" Light yellowish brown (2.5Y 6/3) gravelly loamy sand fill, massive, friable.

12-20" Light yellowish brown (2.5Y 6/4) silty clay loam, blocky, firm, redox.

20-58" Olive (5Y 4/4) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 12"

Observed water: none Depth to bedrock: none

Soil name: Excavated and filled over Boxford or Boxford SPD

Test Pit Number: MC24

Depth Description

0-7" Very dark grayish brown (10YR 3/2) fine sandy loam fill, granular, friable.

7-14" Light yellowish brown (2.5Y 6/3) gravelly loamy sand fill, massive, friable.

14-16" Very dark grayish brown (10YR 3/2) silt loam, massive, friable.

16-19" Light olive brown (2.5Y 5/3) silt loam, blocky, friable, redox.

19-60" Olive (5Y 4/4) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 16"

Observed water: none Depth to bedrock: none

Soil name: Excavated and filled over Boxford or Boxford SPD

Test Pit Number: MC25 Depth Description Forest litter. 0-9" Dark brown (10YR 3/3) stony fine sandy loam, granular, friable. 9-24"

Dark yellowish brown (10YR 4/6) stony fine sandy loam,

blocky, friable.

24-40" Light yellowish brown (10YR 6/4) stony loamy sand, blocky, friable.

40-60" Pale brown (2.5Y 7/3) stony loamy sand, massive, friable.

Depth to Seasonal High Water Table: none

Observed water: none Depth to bedrock: none Soil name: Canton

Test Pit Number: MC26

Depth Description Forest litter.

0-14" Very dark grayish brown (2.5Y 3/2) silt loam, granular, friable.

14-19" Light olive brown (2.5Y 5/4) silt loam, blocky, friable, redox.

19-30" Olive brown (2.5Y 4/4) silty clay loam, blocky, firm, redox.

30-50" Olive brown (2.5Y 4/3) stony fine sandy loam, massive, firm, redox.

50"+ Bedrock

Depth to Seasonal High Water Table: 14"

Observed water: 36" Depth to bedrock: 50"

Soil name: Boxford SPD variant

Test Pit Number: MC27

Depth Description

Forest litter

0-10" Very dark grayish brown (10YR 3/2) silt loam, granular, friable.

10-20" Yellowish brown (10YR 5/6) silt loam, blocky, friable.

20-30" Light olive brown (2.5Y 5/3) silty clay loam, massive, firm, redox.

30-50" Light olive brown (2.5Y 5/4) stony fine sandy loam, massive, firm, redox.

50"+ Bedrock.

Depth to Seasonal High Water Table: 20"

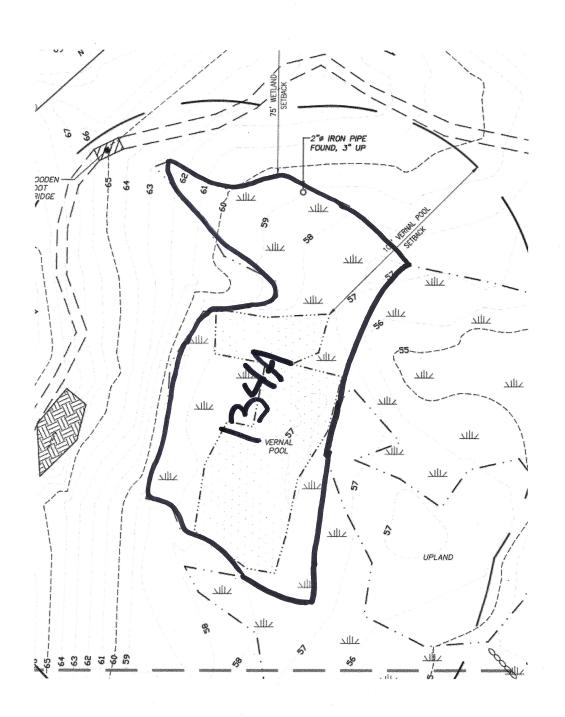
Observed water: none Depth to bedrock: 50"

Soil name: Boxford variant

```
Test Pit Number: MC28
Depth
             Description
1"
        Forest litter.
0-6"
        Very dark grayish brown (2.5Y 3/2) silt loam, granular,
6-8"
        Light olive brown (2.5Y 5/4) silt loam, blocky, friable.
8-11"
        Grayish brown (2.5Y 5/2) silt loam, blocky, friable, redox.
        Olive (5Y 4/3) silty clay loam, blocky, firm, redox.
Depth to Seasonal High Water Table: 8"
Observed water: 30"
Depth to bedrock: none
Soil name: Boxford SPD
Soil Observation A (with hand tools)
             Description
<u>Depth</u>
1"
        Forest litter.
0-10"
        Dark olive brown (2.5Y 3/3) silt loam, granular,
        friable, redox.
10-14"
        Light brownish gray (2.5Y 6/2) silt loam, blocky, friable,
14-19"
        Olive brown (2.5Y 4/3) silt loam, blocky, friable, redox.
19-40"
        Olive (5Y 4/3) silty clay loam, blocky, firm, redox.
Depth to Seasonal High Water Table: surface
Observed water: 12"
Depth to bedrock: none
Soil name: Scitico
Soil Observation B (with hand tools)
             Description
Depth
1 "
        Forest litter.
0-6"
        Black (2.5Y 2/1) silt loam, granular, friable, redox.
6-10"
        Light brownish gray (2.5Y 6/2) silt loam, blocky, friable,
        redox.
10-15"
        Dark grayish brown (2.5Y 4/2) silt loam, blocky, friable,
        redox.
15-20"
        Olive gray (5Y 5/2) silt loam, blocky, firm, redox.
        Olive (5Y 4/3) silty clay loam, blocky, firm, redox.
Depth to Seasonal High Water Table: surface
Observed water: 9"
Depth to bedrock: none
Soil name: Maybid
```



THIS SHOWS THE CORRECT CONFIGURATION OF THIS MAP UNIT



Michael Cuomo, Soil Scientist

6 York Pond Road, York, Maine 03909 207 363 4532 mcuomosoil@gmail.com

TEST PIT DATA

Client: Altus Engineering

Location: Stone Quarry Drive, Durham, NH

Date: 12 June 2025

Test Pit Number: A1

<u>Depth</u>	<u>Description</u>
0-9"	Dark brown (10YR 3/3) silt loam, granular, friable.
9-12"	Light olive brown (2.5Y 5/4) silt loam, blocky, friable.
12-19"	Light olive brown (2.5Y 5/3) silt loam, blocky, friable, redox.
19-75"	Olive (5Y 4/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 12"

Depth to Bedrock: none Observed Water: none

Estimated percolation rate: 60 min./inch

Test Pit Number: A2

<u>Depth</u>	<u>Description</u>
0-15"	Very dark grayish brown (10YR 3/2) very fine sandy loam, granular, friable.
15-17"	Reddish yellow (7.5YR 6/6) very fine sandy loam, blocky, friable.
17-25"	Light olive brown (2.5Y 5/4) very fine sandy loam, blocky, friable, redox.
25-80"	Olive (5Y 4/4) silt loam, massive, firm, redox.

Depth to Seasonal High Water Table: 17"

Depth to Bedrock: none Observed Water: 20"

Estimated percolation rate: 60 min./inch

<u>Depth</u>	<u>Description</u>
0-9"	Dark olive brown (2.5Y 3/3) silt loam, granular, friable.
9-14"	Light olive brown (2.5Y 5/4) silt loam, blocky, friable.
14-24"	Olive (5Y 4/4) silt loam, blocky, friable, redox.
24-60"	Olive (5Y 4/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 14"

Depth to Bedrock: none Observed Water: none

Estimated percolation rate: 60 min./inch

Test Pit Number: A4

<u>Depth</u>	<u>Description</u>
0-10"	Very dark grayish brown (2.5Y 3/2) silt loam fill, granular, friable.
10-14"	Light yellowish brown (2.5Y 6/4) loamy sand fill, massive, friable.
14-16"	Very dark grayish brown (2.5Y 3/2) silt loam, granular, friable, redox.
16-20"	Olive gray (5Y 5/2) silt loam, blocky, friable, redox.
20-66"	Olive (5Y 4/3) silty clay loam, massive, firm, redox.

Depth to Seasonal High Water Table: 14"

Depth to Bedrock: none Observed Water: 55"

Estimated percolation rate: 60 min./inch

Test Pit Number: A5

<u>Depth</u>	<u>Description</u>
0-8"	Very dark grayish brown (10YR 3/3) silt loam, granular, friable.
8-12"	Light olive brown (2.5Y 5/4) silt loam, blocky, friable.
12-19"	Light olive brown (2.5Y 5/3) silt loam, blocky, friable, redox.
19-48"	Olive (5Y 4/3) silty clay loam, massive, firm, redox.
48-72"	Olive brown (2.5Y 4/4) stony fine sandy loam, massive, friable, redox.
72-86"	Olive brown (2.5Y 4/4) stony loamy sand, massive, friable,
72 00	redox.

Depth to Seasonal High Water Table: 12"

Depth to Bedrock: none Observed Water: 60"

Estimated percolation rate: 60 min./inch

<u>Depth</u>	<u>Description</u>
0-9"	Very dark grayish brown (10YR 3/3) fine sandy loam, granular,
	friable.
9-16"	Brownish yellow (10YR 6/6) fine sandy loam, blocky, friable.
16-84"	Light yellowish brown (2.5Y 6/4) loamy sand, blocky, friable.
84-96"	Light olive brown (2.5Y 5/4) loamy sand, blocky, friable, redox.

Depth to Seasonal High Water Table: 84"

Depth to Bedrock: none Observed Water: none

Estimated percolation rate: 4 to 6 min./inch

Section 7

BMP and Riprap Sizing Calculations



FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.08)

Type/Node Name: Bioretention Pond #1 (HydroCAD Node #17P)

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable.

			0/)
X 0.22	_	Check if you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.0	8(a).
0.23	-	A = Area draining to the practice	
0.10	_	A _I = Impervious area draining to the practice	
	decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
	ac-in	WQV= 1" x Rv x A	
359	-	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
90	_	25% x WQV (check calc for sediment forebay volume)	
269		75% x WQV (check calc for surface sand filter volume)	
C	В	Method of Pretreatment? (not required for clean or roof runoff)	250/14/01/
	cf	V _{SED} = Sediment forebay volume, if used for pretreatment	<u>></u> 25%WQV
Calculate ti		if system IS NOT underdrained:	
	sf -	A _{SA} = Surface area of the practice	
	iph	Ksat _{DESIGN} = Design infiltration rate ¹	
	-	If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
	Yes/No	(Use the calculations below)	
-	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	< 72-hrs
Calculate ti	me to drair	if system IS underdrained:	
56.80	ft	E _{WQV} = Elevation of WQV (attach stage-storage table)	
0.01	cfs	Q_{WQV} = Discharge at the E_{WQV} (attach stage-discharge table)	
19.92	hours	T_{DRAIN} = Drain time = $2WQV/Q_{WQV}$	<u><</u> 72-hrs
54.75	feet	E _{FC} = Elevation of the bottom of the filter course material ²	
53.75	- feet	E _{UD} = Invert elevation of the underdrain (UD), if applicable	
57.00	- feet	E _{SHWT} = Elevation of SHWT (if none found, enter the lowest elevation of the test p	it)
51.75	-	E_{ROCK} = Elevation of bedrock (if none found, enter the lowest elevation of the test	
1.00		D _{FC to UD} = Depth to UD from the bottom of the filter course	≥ 1'
3.00	_	D _{FC to ROCK} = Depth to bedrock from the bottom of the filter course	_ ≥1'
(2.25)	_	D _{FC to SHWT} = Depth to SHWT from the bottom of the filter course	<u>≥</u> 1'
58.50		Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
59.25		Elevation of the top of the practice	
YES		50 peak elevation ≤ Elevation of the top of the practice	← yes
If a surface	sand filter	or underground sand filter is proposed:	•
YES	ac	Drainage Area check.	< 10 ac
	cf	V = Volume of storage ³ (attach a stage-storage table)	≥ 75%WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
_	-	Note what shoot in the plan set contains the filter course specification	
Sheet	•	Note what sheet in the plan set contains the filter course specification.	
Sheet	Yes/No	Access grate provided?	← yes

If a biore	etention area	is proposed:	
YES	ac	Drainage Area no larger than 5 ac?	← yes
1,05	54 cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> WQV
18	inches 3.0	D _{FC} = Filter course thickness	18", or 24" if within GPA
She	eet C-15	Note what sheet in the plan set contains the filter course specification	
3	3.0 :1	Pond side slopes	<u>> 3</u> :1
She	eet L1 & L2	Note what sheet in the plan set contains the planting plans and surface cover	
If porous	s pavement is	proposed:	
		Type of pavement proposed (Concrete? Asphalt? Pavers? Etc.)	
	acres	A _{SA} = Surface area of the pervious pavement	
	:1	Ratio of the contributing area to the pervious surface area	≤ 5:1
	inches	D _{FC} = Filter course thickness	12", or 18" if within GPA
She	eet C-14	Note what sheet in the plan set contains the filter course spec.	mod. 304.1 (see spec)

- 1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.
- 2. See lines 34, 40 and 48 for required depths of filter media.
- 3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:		
System is lined, SHWT not relevant		

Stage-Area-Storage for Pond 17P: Bio #1

Elevation	Surface	Wetted	Storage	
(feet)	(sq-ft)	(sq-ft)	(cubic-feet)	
54.75	487	487	0	
54.85	487	495	2 5	
54.95	487	503	5	
55.05	487	510	7	
55.15	487	518	10	
55.25	487	526	12	
55.35	487	534	15	
55.45	487	542	17	
55.55	487	550	19	
55.65	487	557	22	
55.75	487	565	24	
55.85	487	573	27	
55.95	487	581	29	
56.05	487	589	32	
56.15	487	597	34	
56.25	487	604	37	
56.35	521	640	87	
56.45	557	676	141	
56.55	594	714	198	
56.65	632	753	260	
56.75	671	793	325	MOV = 250 of
56.85	711	835	394	_ WQV = 359 cf
56.95	753	877	467	Ewqv = 56.80
57.05	795	920	544	
57.15	837	964	626	
57.25	880	1,008	712	Available
57.35	924	1,054	802	
57.45	970	1,100	897	WQV=1,054cf
57.55	1,016	1,148	996	
57.65	1,064	1,197	1,100	
57.75	1,113	1,247	1,209	
57.85	1,162	1,298	1,323	
57.95	1,213	1,351	1,441	
58.05	1,264	1,403	1,565	
58.15	1,316	1,457	1,694	
58.25	1,369	1,511	1,829	
58.35	1,423	1,566	1,968	
58.45	1,477	1,623	2,113	
58.55	1,533	1,680	2,264	
58.65	1,590	1,739	2,420	
58.75	1,648	1,798	2,582	
58.85	1,707	1,859	2,749	
58.95	1,767	1,921	2,923	
59.05	1,827	1,983	3,103	
59.15	1,888	2,046	3,288	
59.25	1,950	2,110	3,480	

Stage-Discharge for Pond 17P: Bio #1

			_
Elevation	Primary	Elevation	Primary
(feet)	(cfs)	(feet)	(cfs)
54.75	0.00	57.40	0.03
54.80	0.00	57.45	0.05
54.85	0.00	57.50	0.08
54.90	0.00	57.55	0.11
54.95	0.00	57.60	0.15
55.00	0.00	57.65	0.19
55.05	0.00	57.70	0.22
55.10	0.00	57.75	0.24
55.15	0.00	57.80	0.26
55.20	0.00	57.85	0.28
55.25	0.00	57.90	0.30
55.30	0.00	57.95	0.32
55.35	0.00	58.00	0.34
55.40	0.00	58.05	0.35
55.45	0.00	58.10	0.37
55.50	0.00	58.15	0.38
55.55	0.00	58.20	0.40
55.60	0.00	58.25	0.41
55.65	0.00	58.30	0.43
55.70	0.00	58.35	0.44
55.75	0.00	58.40	0.45
55.80	0.00	58.45	0.46
55.85	0.00	58.50	0.48
55.90	0.00	58.55	0.49
55.95	0.00	58.60	0.50
56.00	0.00	58.65	0.51
56.05	0.00	58.70	0.52
56.10	0.00	58.75	0.54
56.15	0.00	58.80	0.84
56.20	0.00	58.85	1.39
56.25	0.00	58.90	2.09
56.30	0.00	58.95	2.92
56.35	0.00	59.00	3.86
56.40	0.00	59.05	4.90
56.45	0.00	59.10	6.03
56.50	0.01	59.15	7.24
56.55	0.01	59.20	8.53
56.60	0.01	59.25	9.89
56.65	0.01		
56.70	0.01		
56.75	0.01		
56.80 56.85	0.01 0.01		0.01cfs
56.90	0.01		
56.95	0.01		
57.00	0.02		
57.05	0.02		
57.10	0.02		
57.15	0.02		
57.20	0.02		
57.25	0.02		
57.30	0.02		
57.35	0.03		

FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.08)

Type/Node Name: Bioretention Pond #2 (HydroCAD Node #22P)

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable.

		Chook if you reviewed the restrictions on unlined systems authored in Fau Wa 1500.0	19/2)
0.36	_	Check if you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.0	18(a).
	_	A = Area draining to the practice	
0.27		A ₁ = Impervious area draining to the practice	
	decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I) WQV= 1" x Rv x A	
947	ac-in	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
237	_	25% x WQV (check calc for sediment forebay volume)	
711	_	75% x WQV (check calc for surface sand filter volume)	
	CB	Method of Pretreatment? (not required for clean or roof runoff)	
	cf	V _{SED} = Sediment forebay volume, if used for pretreatment	≥ 25%WQV
Calculate ti		n if system IS NOT underdrained:	
	sf	A _{SA} = Surface area of the practice	
	-	Ksat _{DESIGN} = Design infiltration rate ¹	
	iph -		
	Vaa/Na	If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
	Yes/No	(Use the calculations below)	< 72 hrs
	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	≤ 72-hrs
		if system IS underdrained:	
58.45	_	E _{WQV} = Elevation of WQV (attach stage-storage table)	
0.02	cfs	Q_{WQV} = Discharge at the E_{WQV} (attach stage-discharge table)	
26.32	hours	$T_{DRAIN} = Drain time = 2WQV/Q_{WQV}$	<u><</u> 72-hrs
56.25	feet	E_{FC} = Elevation of the bottom of the filter course material ²	
55.25	feet	E _{UD} = Invert elevation of the underdrain (UD), if applicable	
55.08	feet	E_{SHWT} = Elevation of SHWT (if none found, enter the lowest elevation of the test p	oit)
50.75	feet	E _{ROCK} = Elevation of bedrock (if none found, enter the lowest elevation of the test	pit)
1.00	feet	$D_{FC \text{ to UD}}$ = Depth to UD from the bottom of the filter course	<u>≥</u> 1'
5.50	feet	$D_{FC \text{ to ROCK}}$ = Depth to bedrock from the bottom of the filter course	<u>≥</u> 1'
1.17	feet	D _{FC to SHWT} = Depth to SHWT from the bottom of the filter course	<u>≥</u> 1'
59.65	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
60.00	ft	Elevation of the top of the practice	
YES		50 peak elevation \leq Elevation of the top of the practice	← yes
If a surface	sand filter	or underground sand filter is proposed:	
YES	ac	Drainage Area check.	< 10 ac
	cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> 75%WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet	-	Note what sheet in the plan set contains the filter course specification.	
	Yes/No	Access grate provided?	← yes

If a b	iorete	ntion area i	is proposed:	
Υ	'ES	ac	Drainage Area no larger than 5 ac?	← yes
	1,000	cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> WQV
	18.0	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
	Sheet	C-15	Note what sheet in the plan set contains the filter course specification	
	3.0	:1	Pond side slopes	<u>> 3</u> :1
	Sheet	L1 & L2	Note what sheet in the plan set contains the planting plans and surface cover	
If po	rous p	avement is	proposed:	
			Type of pavement proposed (Concrete? Asphalt? Pavers? Etc.)	
		acres	A _{SA} = Surface area of the pervious pavement	
		:1	Ratio of the contributing area to the pervious surface area	≤ 5:1
		inches	D _{FC} = Filter course thickness	12", or 18" if within GPA
	Sheet		Note what sheet in the plan set contains the filter course spec.	mod. 304.1 (see spec)

- 1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.
- 2. See lines 34, 40 and 48 for required depths of filter media.
- 3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:					
·		·		·	
·		·		·	

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Stage-Area-Storage for Pond 22P: Bio #2

Elevation	Surface	Wetted	Storage	
(feet)	(sq-ft)	(sq-ft)	(cubic-feet)	
56.25	1,027	1,027	0	
56.35	1,027	1,038	5	
56.45	1,027	1,050	10	
56.55	1,027	1,061	15	
56.65	1,027	1,072	21	
56.75	1,027	1,084	26	
56.85	1,027	1,095	31	
56.95	1,027	1,107	36	
57.05	1,027	1,118	41	
57.15	1,027	1,129	46	
57.25	1,027	1,141	51	
57.35	1,027	1,152	56	
57.45	1,027	1,163	62	
57.55	1,027	1,175	67	
57.65	1,027	1,186	72	
57.75	1,027	1,197	77	
57.85	1,085	1,257	183	
57.95	1,145	1,317	294	
58.05	1,203	1,377	412	
58.15	1,261	1,437	535	
58.25	1,321	1,497	664	WQV = 947 cf
58.35	1,381	1,559	799	I
58.45	1,443	1,623	940	Ewqv = 58.45
58.55	1,507	1,688	1,088	Lowest Outlet = 58.50
58.65	1,572	1,754	1,242	Available WQV = 1,000 cf
58.75	1,638	1,822	1,402	7 (Valiable 77 Q V = 1,000 cl
58.85	1,705	1,891	1,569	
58.95	1,774	1,961	1,743	
59.05	1,841	2,030	1,924	
59.15	1,907	2,097	2,111	
59.25	1,974	2,166	2,305	
59.35	2,042	2,236	2,506	
59.45	2,111	2,307	2,714	
59.55	2,181	2,379	2,928	
59.65	2,252	2,452	3,150	
59.75	2,325	2,526	3,379	
59.85	2,398	2,602	3,615	
59.95	2,473	2,679	3,859	

Stage-Discharge for Pond 22P: Bio #2

Elevation	Primary	Elevation	Primary	Elevation	Primary	Elevation	Primary
(feet)	(cfs)	(feet)	(cfs)	(feet)	(cfs)	(feet)	(cfs)
56.25	0.00	57.31	0.00	58.37	0.02	59.43	0.43
56.27	0.00	57.33	0.00	58.39	0.02	59.45	0.44
56.29	0.00	57.35	0.00	58.41	0.02	59.47	0.44
56.31	0.00	57.37	0.00	58.43	0.02	59.49	0.45
56.33	0.00	57.39	0.00	58.45	0.02	Qwqv = 0) 02 cfe
56.35	0.00	57.41	0.00	58.47	0.03	-Q V q V $ Q$	7.02 CIS
56.37	0.00	57.43	0.00	58.49	0.03	59.55	0.76
56.39	0.00	57.45	0.00	58.51	0.03	59.57	0.95
56.41	0.00	57.47	0.00	58.53	0.03	59.59	1.18
56.43	0.00	57.49	0.00	58.55	0.03	59.61	1.43
56.45	0.00	57.51	0.00	58.57	0.04	59.63	1.71
56.47	0.00	57.53	0.00	58.59	0.05	59.65	2.01
56.49	0.00	57.55	0.00	58.61	0.06	59.67	2.33
56.51	0.00	57.57	0.00	58.63	0.07	59.69	2.67
56.53	0.00	57.59	0.00	58.65	0.08	59.71	3.02
56.55	0.00	57.61	0.00	58.67	0.10	59.73	3.39
56.57	0.00	57.63	0.00	58.69	0.11	59.75	3.78
56.59	0.00	57.65	0.00	58.71	0.12	59.77	4.19
56.61	0.00	57.67	0.00	58.73	0.14	59.79	4.61
56.63	0.00	57.69	0.00	58.75	0.16	59.81	5.04
56.65	0.00	57.71	0.00	58.77	0.17	59.83	5.49
56.67	0.00	57.73	0.00	58.79	0.19	59.85	5.96
56.69	0.00	57.75	0.00	58.81	0.20	59.87	6.43
56.71	0.00	57.77	0.00	58.83	0.21	59.89	6.92
56.73	0.00	57.79	0.00	58.85	0.22	59.91	7.42
56.75	0.00	57.81	0.00	58.87	0.23	59.93	7.93
56.77	0.00	57.83	0.00	58.89	0.24	59.95	8.46
56.79	0.00	57.85	0.00	58.91	0.25	59.97	9.00
56.81	0.00	57.87	0.00	58.93	0.26	59.99	9.54
56.83	0.00	57.89	0.00	58.95	0.27		0.0.
56.85	0.00	57.91	0.01	58.97	0.28		
56.87	0.00	57.93	0.01	58.99	0.28		
56.89	0.00	57.95	0.01	59.01	0.29		
56.91	0.00	57.97	0.01	59.03	0.30		
56.93	0.00	57.99	0.01	59.05	0.31		
56.95	0.00	58.01	0.01	59.07	0.32		
56.97	0.00	58.03	0.01	59.09	0.32		
56.99	0.00	58.05	0.01	59.11	0.33		
57.01	0.00	58.07	0.01	59.13	0.34		
57.03	0.00	58.09	0.01	59.15	0.34		
57.05	0.00	58.11	0.01	59.17	0.35		
57.07	0.00	58.13	0.01	59.19	0.36		
57.09	0.00	58.15	0.01	59.21	0.36		
57.11	0.00	58.17	0.01	59.23	0.37		
57.13	0.00	58.19	0.02	59.25	0.38		
57.15	0.00	58.21	0.02	59.27	0.38		
57.17	0.00	58.23	0.02	59.29	0.39		
57.19	0.00	58.25	0.02	59.31	0.40		
57.21	0.00	58.27	0.02	59.33	0.40		
57.23	0.00	58.29	0.02	59.35	0.41		
57.25	0.00	58.31	0.02	59.37	0.41		
57.27	0.00	58.33	0.02	59.39	0.42		
57.29	0.00	58.35	0.02	59.41	0.42		
		1		l			

FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.08)

Type/Node Name: Bioretention Pond #3 (HydroCAD Node #69P)

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable.

		Chook if you reviewed the restrictions on unlined systems authored in Fau Wa 1500.0	19/2)
1.68	_	Check if you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.0	18(a).
-	_	A = Area draining to the practice	
1.07	_	A ₁ = Impervious area draining to the practice	
	decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = $0.05 + (0.9 \times I)$	
	ac-in	WQV= 1" x Rv x A	
3,797 949	_	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
2,848	_	25% x WQV (check calc for sediment forebay volume) 75% x WQV (check calc for surface sand filter volume)	
	CB	Method of Pretreatment? (not required for clean or roof runoff)	
	cf		≥ 25%WQV
Cala Jarani		V _{SED} = Sediment forebay volume, if used for pretreatment	<u>></u> 23/8 VVQV
Calculate ti		n if system IS NOT underdrained:	
	_sf _	A _{SA} = Surface area of the practice	
	iph_	Ksat _{DESIGN} = Design infiltration rate ¹	
		If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
	Yes/No	(Use the calculations below)	
-	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	<u><</u> 72-hrs
Calculate ti	me to drair	if system IS underdrained:	
49.63	_ft	E _{WQV} = Elevation of WQV (attach stage-storage table)	
0.06	cfs	Q_{WQV} = Discharge at the E_{WQV} (attach stage-discharge table)	
35.16	hours	$T_{DRAIN} = Drain time = 2WQV/Q_{WQV}$	<u><</u> 72-hrs
46.50	feet	E _{FC} = Elevation of the bottom of the filter course material ²	
45.50	feet	E _{UD} = Invert elevation of the underdrain (UD), if applicable	
49.50	feet	E_{SHWT} = Elevation of SHWT (if none found, enter the lowest elevation of the test p	oit)
48.50	feet	E_{ROCK} = Elevation of bedrock (if none found, enter the lowest elevation of the test	: pit)
1.00	feet	$D_{FC \text{ to UD}}$ = Depth to UD from the bottom of the filter course	<u>≥</u> 1'
(2.00)	feet	$D_{FC \text{ to ROCK}}$ = Depth to bedrock from the bottom of the filter course	<u>≥</u> 1'
(3.00)	feet	$D_{FC \text{ to SHWT}}$ = Depth to SHWT from the bottom of the filter course	<u>≥</u> 1'
52.57	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
53.00	ft	Elevation of the top of the practice	
YES		50 peak elevation \leq Elevation of the top of the practice	← yes
If a surface	sand filter	or underground sand filter is proposed:	
YES	ac	Drainage Area check.	< 10 ac
	cf	V = Volume of storage ³ (attach a stage-storage table)	≥ 75%WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet	-	Note what sheet in the plan set contains the filter course specification.	
	Yes/No	Access grate provided?	← yes
-			!

If a biorete	ntion area i	is proposed:	
YES	ac	Drainage Area no larger than 5 ac?	← yes
4,799	cf	V = Volume of storage ³ (attach a stage-storage table)	≥ WQV
18.0	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet	C-15	Note what sheet in the plan set contains the filter course specification	
3.0	:1	Pond side slopes	<u>> 3</u> :1
Sheet	L1 & L2	Note what sheet in the plan set contains the planting plans and surface cover	
If porous p	avement is	proposed:	
		Type of pavement proposed (Concrete? Asphalt? Pavers? Etc.)	
	acres	A _{SA} = Surface area of the pervious pavement	
	:1	Ratio of the contributing area to the pervious surface area	≤ 5:1
	inches	D _{FC} = Filter course thickness	12", or 18" if within GPA
Sheet	- :	Note what sheet in the plan set contains the filter course spec.	mod. 304.1 (see spec)

- 1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.
- 2. See lines 34, 40 and 48 for required depths of filter media.
- 3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:		
System is lined, SHWT not relevant		

Prepared by Altus Engineering, Inc.

HydroCAD® 10.00-25 s/n 01222 © 2019 HydroCAD Software Solutions LLC

Stage-Area-Storage for Pond 69P: Bio #3 / OS #11

Elevat		Surface	Wetted	Storage	
	et)	(sq-ft)	(sq-ft)	(cubic-feet)	
46.		1,706	1,706	0	
46.		1,706	1,735	17	
46.		1,706	1,765	34	
47.		1,706	1,794	51	
47.		1,706	1,823	68	
47.		1,706	1,852	85	
47.		1,706	1,882	102	
47.	.90	1,706	1,911	119	
48.	.10	1,768	1,990	302	
48.	.30	1,897	2,122	668	
48.	.50	2,029	2,258	1,061	
48.	.70	2,167	2,399	1,480	
48.	.90	2,308	2,545	1,928	
49.	.10	2,445	2,685	2,403	
49.	.30	2,575	2,820	2,905	WQV = 3,797 cf
49.	.50	2,708	2,958	3,433	Ewgv = 49.63
49.	.70	2,845	3,100	3,989	LWQV = 49.03
49.	.90	2,985	3,245	4,572	Lowest Outlet = 50.00
50.	.10	3,139	3,404	5,183	Available WQV = 4,874 cf
50.	.30	3,308	3,578	5,828	Available WQV - 4,074 ci
50.	.50	3,482	3,757	6,507	
50.	.70	3,660	3,940	7,221	
50.	.90	3,842	4,127	7,971	
51.	.10	4,028	4,319	8,758	
51.	.30	4,218	4,514	9,583	
51.	.50	4,413	4,714	10,446	
51.	.70	4,612	4,918	11,348	
51.	.90	4,815	5,127	12,291	
52.	.10	5,023	5,341	13,274	
52.	.30	5,237	5,561	14,300	
52.	.50	5,455	5,785	15,369	
52.		5,677	6,014	16,482	
52.		5,904	6,247	17,640	

Stage-Discharge for Pond 69P: Bio #3 / OS #11

Elevation	Primary	Elevation	Primary	Elevation	Primary	Elevation	Primary
(feet)	(cfs)	(feet)	(cfs)	(feet)	(cfs)	(feet)	(cfs)
46.50	0.00	48.62	0.02	50.74	0.80	52.86	16.48
46.54	0.00	48.66	0.02	50.78	0.85	52.90	18.49
46.58	0.00	48.70	0.03	50.82	0.91	52.94	20.59
46.62	0.00	48.74	0.03	50.86	0.97	52.98	22.07
46.66	0.00	48.78	0.03	50.90	1.04		
46.70	0.00	48.82	0.03	50.94	1.10		
46.74	0.00	48.86	0.03	50.98	1.16		
46.78	0.00	48.90	0.03	51.02	1.23		
46.82	0.00	48.94	0.04	51.06	1.30		
46.86	0.00	48.98	0.04	51.10	1.37		
46.90	0.00	49.02	0.04	51.14	1.44		
46.94	0.00	49.06	0.04	51.18	1.51		
46.98	0.00	49.10	0.04	51.22	1.58		
47.02	0.00	49.14	0.04	51.26	1.65		
47.06	0.00	49.18	0.05	51.30	1.73		
47.10	0.00	49.22	0.05	51.34	1.81		
47.14	0.00	49.26	0.05	51.38	1.88		
47.18	0.00	49.30	0.05	51.42	1.96		
47.22	0.00	49.34	0.05	51.46	2.04		
47.26	0.00	49.38	0.05	51.50	2.12		
47.30	0.00	49.42	0.05	51.54	2.20		
47.34	0.00	49.46	0.06	51.58	2.28		
47.38	0.00	49.50	0.06	51.62	2.37		
47.42	0.00	49.54	0.06	51.66	2.45		
47.46	0.00	49.58	0.06	51.70	2.54		
47.50	0.00	49.62	0.06		6 cfs		
47.54	0.00	49.66	0.06				
47.58	0.00	49.70	0.07	51.82	2.80		
47.62	0.00	49.74	0.07	51.86	2.89		
47.66	0.00	49.78	0.07	51.90	2.98		
47.70	0.00	49.82	0.07	51.94	3.07		
47.74	0.00	49.86	0.07	51.98	3.16		
47.78	0.00	49.90	0.07	52.02	3.26		
47.82	0.00	49.94	0.07	52.06	3.35		
47.86	0.00	49.98	0.08	52.10	3.45		
47.90	0.00	50.02	0.08	52.14	3.54		
47.94	0.00	50.06	0.10	52.18	3.64		
47.98	0.00	50.10	0.12	52.22	3.74		
48.02	0.00	50.14	0.14	52.26	3.83		
48.06	0.00	50.18	0.17	52.30	3.93		
48.10	0.00	50.22	0.20	52.34	4.03		
48.14	0.01	50.26	0.23	52.38	4.14		
48.18	0.01	50.30	0.27	52.42	4.24		
48.22	0.01	50.34 50.38	0.31	52.46	4.34		
48.26 48.30	0.01		0.35 0.39	52.50	4.44		
	0.01	50.42 50.46		52.54	4.96		
48.34	0.01		0.43	52.58 52.62	5.81		
48.38 48.42	0.01 0.02	50.50 50.54	0.48 0.53	52.62 52.66	6.89 8.15		
46.42 48.46	0.02	50.54	0.53 0.58	52.00 52.70	9.56		
48.50	0.02	50.56	0.56	52.70 52.74	9.56		
46.50 48.54	0.02	50.62	0.68	52.74 52.78	12.79		
48.58	0.02	50.70	0.00	52.76 52.82	14.58		
- 0.00	0.02] 30.70	5.74	02.02	1-7.00		

FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.08)

Type/Node Name: Bioretention Pond #4 (HydroCAD Node #83P)

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable.

Х		Check if you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.0	8(2)
0.33	ar -	A = Area draining to the practice	o(a).
0.25	-	A ₁ = Impervious area draining to the practice	
	decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
	ac-in	WQV= 1" x Rv x A	
877	-	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
219	-	25% x WQV (check calc for sediment forebay volume)	
657	-	75% x WQV (check calc for surface sand filter volume)	
C	B	Method of Pretreatment? (not required for clean or roof runoff)	
	cf	V _{SED} = Sediment forebay volume, if used for pretreatment	≥ 25%WQV
Calculate ti	me to drair	n if system IS NOT underdrained:	
	sf	A _{SA} = Surface area of the practice	
	iph	Ksat _{DESIGN} = Design infiltration rate ¹	
	_ '	If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
	Yes/No	(Use the calculations below)	
_	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	< 72-hrs
		n if system IS underdrained:	
47.05		E _{WQV} = Elevation of WQV (attach stage-storage table)	
0.03	- cfs	Q_{WOV} = Discharge at the E_{WOV} (attach stage-discharge table)	
16.23	hours	$T_{DRAIN} = Drain time = 2WQV/Q_{WQV}$	<u><</u> 72-hrs
44.00	feet	E _{FC} = Elevation of the bottom of the filter course material ²	
43.00	feet	E_{UD} = Invert elevation of the underdrain (UD), if applicable	
43.75	feet	E_{SHWT} = Elevation of SHWT (if none found, enter the lowest elevation of the test p	it)
37.58	feet	E_{ROCK} = Elevation of bedrock (if none found, enter the lowest elevation of the test	pit)
1.00	feet	$D_{FC \text{ to UD}}$ = Depth to UD from the bottom of the filter course	<u>≥</u> 1'
6.42	feet	$D_{FC \text{ to ROCK}}$ = Depth to bedrock from the bottom of the filter course	<u>≥</u> 1'
0.25	feet	$D_{FC \text{ to SHWT}}$ = Depth to SHWT from the bottom of the filter course	<u>≥</u> 1'
48.63	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
49.00	ft	Elevation of the top of the practice	
YES		50 peak elevation \leq Elevation of the top of the practice	← yes
If a surface	sand filter	or underground sand filter is proposed:	
YES	ac	Drainage Area check.	< 10 ac
833	cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> 75%WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet		Note what sheet in the plan set contains the filter course specification.	
	Yes/No	Access grate provided?	← yes
	,		

If a biorete	ntion area i	is proposed:	
YES	ac	Drainage Area no larger than 5 ac?	← yes
873	cf	V = Volume of storage ³ (attach a stage-storage table)	≥ WQV
18.0	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet	C-15	Note what sheet in the plan set contains the filter course specification	
3.0	_:1	Pond side slopes	<u>> 3</u> :1
Sheet	L1 & L2	Note what sheet in the plan set contains the planting plans and surface cover	
If porous pa	avement is	proposed:	
	acres	Type of pavement proposed (Concrete? Asphalt? Pavers? Etc.) A _{SA} = Surface area of the pervious pavement	
	:1	Ratio of the contributing area to the pervious surface area	≤ 5:1
	inches	D _{FC} = Filter course thickness	12", or 18" if within GPA
Sheet		Note what sheet in the plan set contains the filter course spec.	mod. 304.1 (see spec)

- 1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.
- 2. See lines 34, 40 and 48 for required depths of filter media.
- 3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:		
System is lined, SHWT not relevant		

Stage-Area-Storage for Pond 83P: Bio #4 / OS #22

Elevation	Surface	Storage	Elevation	Surface	Storage	
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)	
44.00	315	0	46.65	672	582	
44.05	315	1	46.70	689	616	
44.10	315	2	46.75	706	651	
44.15	315	2	46.80	722	687	
44.20	315	3	46.85	739	724	
44.25	315	4	46.90	756	761	
44.30	315	5	46.95	772	799	
44.35	315	6	47.00	789	838	WQV = 877 cf
44.40	315	6	47.05	810		Ewqv = 47.05
44.45	315	7	47.10	830		Lowest Outlet = 47.05
44.50	315	8	47.15	851	961	Available WQV = 878 cf
44.55	315	9	47.20	872	1,004	rttallable tract
44.60	315	9	47.25	893	1,048	
44.65	315	10	47.30	913	1,093	
44.70	315	11	47.35	934	1,140	
44.75	315	12	47.40	955	1,187	
44.80	315	13	47.45	975	1,235	
44.85	315	13	47.50	996	1,284	
44.90	315	14	47.55	1,017	1,335	
44.95	315	15	47.60	1,037	1,386	
45.00	315	16	47.65	1,058	1,438	
45.05	315	17	47.70	1,079	1,492	
45.10	315	17	47.75	1,100	1,546	
45.15	315	18	47.80	1,120	1,602	
45.20	315	19	47.85	1,141	1,658	
45.25	315	20	47.90	1,162	1,716	
45.30	315	20	47.95	1,182	1,774	
45.35	315	21	48.00	1,203	1,834	
45.40	315	22	48.05	1,228	1,895	
45.45	315	23	48.10	1,252	1,957	
45.50	315	24	48.15	1,277	2,020	
45.55	329	40	48.20	1,301	2,085	
45.60	343	57	48.25	1,326	2,150	
45.65	357	74	48.30	1,351	2,217	
45.70	371	92	48.35	1,375	2,285	
45.75	385	111	48.40	1,400	2,355	
45.80	399	131	48.45	1,424	2,425	
45.85	413	151	48.50	1,449	2,497	
45.90	427	172	48.55	1,474	2,570	
45.95	441	194	48.60	1,498	2,644	
46.00	455	216	48.65	1,523	2,720	
46.05	472	239	48.70	1,547	2,797	
46.10	488	263	48.75	1,572	2,875	
46.15	505	288	48.80	1,597	2,954	
46.20	522	314	48.85	1,621	3,034	
46.25	539	340	48.90	1,646	3,116	
46.30	555	368	48.95	1,670	3,199	
46.35	572	396	49.00	1,695	3,283	
46.40	589	425				
46.45	605	455				
46.50	622	485				
46.55	639	517				
46.60	655	549				

Stage-Discharge for Pond 83P: Bio #4 / OS #22

Elevation	Primary	Elevation	Primary	
(feet) 44.00	(cfs)	(feet) 46.65	(cfs) 0.02	
44.00 44.05	0.00 0.00	46.70	0.02	
44.03	0.00	46.75	0.02	
44.10 44.15	0.00	46.80	0.02	
44.20	0.00	46.85	0.02	
44.25	0.00	46.90	0.02	
44.30	0.00	46.95	0.03	
44.35	0.00	47.00	0.03	
44.40	0.00	47.05	0.03	Qwqv = 0.03 cfs
44.45	0.00	47.10	0.04	= QWqV = 0.00 cls
44.50	0.00	47.15	0.05	
44.55	0.00	47.20	0.08	
44.60	0.00	47.25	0.12	
44.65	0.00	47.30	0.15	
44.70	0.00	47.35	0.19	
44.75	0.00	47.40	0.22	
44.80	0.00	47.45	0.24	
44.85	0.00	47.50	0.26	
44.90	0.00	47.55	0.28	
44.95	0.00	47.60	0.30	
45.00	0.00	47.65	0.32	
45.05 45.10	0.00	47.70 47.75	0.34	
45.10 45.15	0.00 0.00	47.75 47.80	0.35 0.37	
45.13 45.20	0.00	47.85	0.37	
45.25	0.00	47.90	0.40	
45.30	0.00	47.95	0.40	
45.35	0.00	48.00	0.42	
45.40	0.00	48.05	0.44	
45.45	0.00	48.10	0.45	
45.50	0.00	48.15	0.46	
45.55	0.00	48.20	0.47	
45.60	0.00	48.25	0.49	
45.65	0.00	48.30	0.50	
45.70	0.00	48.35	0.51	
45.75	0.00	48.40	0.52	
45.80	0.00	48.45	0.53	
45.85	0.01	48.50	0.54	
45.90	0.01	48.55	0.84	
45.95 46.00	0.01 0.01	48.60 48.65	1.39 2.09	
46.05	0.01	48.70	2.09	
46.10	0.01	48.75	3.86	
46.15	0.01	48.80	4.90	
46.20	0.01	48.85	6.03	
46.25	0.01	48.90	7.24	
46.30	0.01	48.95	8.53	
46.35	0.01	49.00	9.89	
46.40	0.02			
46.45	0.02			
46.50	0.02			
46.55	0.02			
46.60	0.02			

Stage-Area-Storage for Pond 10P: Porous Pavement

- 1 ('	0 (\A/ ((01
Elevation	Surface	Wetted	Storage
(feet)	(sq-ft)	(sq-ft)	(cubic-feet)
0.00	2,761	2,761	0
0.10	2,761	2,780	110
0.20	2,761	2,798	221
0.30	2,761	2,817	331
0.40	2,761	2,836	442
0.50	2,761	2,854	552
0.60	2,761	2,873	663
0.70	2,761	2,891	773
0.80	2,761	2,910	884
0.90	2,761	2,929	994
1.00	2,761	2,947	1,104
1.10	2,761	2,966	1,215
1.20	2,761	2,985	1,325
1.30	2,761	3,003	1,436
1.40	2,761	3,022	1,546
1.50	2,761	3,040	1,657
1.60	2,761	3,059	1,670
1.70	2,761	3,078	1,684
1.80	2,761	3,096	1,698
1.90	2,761	3,115	1,712
2.00	2,761	3,134	1,726
2.10	2,761	3,152	1,739
2.20	2,761	3,171	1,753
2.30	2,761	3,189	1,767
2.40	2,761	3,208	1,781
2.50	2,761	3,227	1,795
2.60	2,761	3,245	1,905
2.70	2,761	3,264	2,016
2.80	2,761	3,283	2,126
2.90	2,761	3,301	2,236
3.00	2,761	3,320	2,347
3.10	2,761	3,338	2,457
3.20	2,761	3,357	2,559
3.30	2,761	3,376	2,642
3.40	2,761	3,394	2,725
3.50	2,761	3,413	2,808
		*	•

FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.08)

Type/Node Name: Porous Pavement (HydroCAD Node #10P)

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable.

			20/)
X 0.22		Check if you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.0	18(a).
0.23	-	A = Area draining to the practice	
0.23	_	A _I = Impervious area draining to the practice	
	decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
	ac-in	WQV= 1" x Rv x A	
800	_	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
200	_	25% x WQV (check calc for sediment forebay volume)	
600		75% x WQV (check calc for surface sand filter volume)	
	СВ	_ Method of Pretreatment? (not required for clean or roof runoff)	> 250/14/01/
	cf	V _{SED} = Sediment forebay volume, if used for pretreatment	<u>></u> 25%WQV
		n if system IS NOT underdrained:	
2,761	_sf _	A _{SA} = Surface area of the practice	
0.50	iph	Ksat _{DESIGN} = Design infiltration rate ¹	
	_	If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
	Yes/No	(Use the calculations below)	
7.0	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	< 72-hrs
Calculate ti	me to drain	n if system IS underdrained:	
	ft	E _{WQV} = Elevation of WQV (attach stage-storage table)	
	cfs	Q_{WQV} = Discharge at the E_{WQV} (attach stage-discharge table)	
-	hours	$T_{DRAIN} = Drain time = 2WQV/Q_{WQV}$	<u><</u> 72-hrs
	feet	E _{FC} = Elevation of the bottom of the filter course material ²	
	feet	E _{UD} = Invert elevation of the underdrain (UD), if applicable	
	feet	E_{SHWT} = Elevation of SHWT (if none found, enter the lowest elevation of the test p	oit)
	feet	E_{ROCK} = Elevation of bedrock (if none found, enter the lowest elevation of the test	pit)
-	feet	$D_{FC \text{ to UD}}$ = Depth to UD from the bottom of the filter course	<u>≥</u> 1'
-	feet	$D_{FC \text{ to ROCK}}$ = Depth to bedrock from the bottom of the filter course	<u>≥</u> 1'
-	feet	$D_{FC \text{ to SHWT}}$ = Depth to SHWT from the bottom of the filter course	<u>≥</u> 1'
	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
	ft	Elevation of the top of the practice	
-		50 peak elevation \leq Elevation of the top of the practice	← yes
If a surface	sand filter	or underground sand filter is proposed:	
YES	ac	Drainage Area check.	< 10 ac
	_cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> 75%WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet	- · ·	Note what sheet in the plan set contains the filter course specification.	
	Yes/No	Access grate provided?	← yes
•			

If a biorete	ention area	is proposed:	
YES	ac	Drainage Area no larger than 5 ac?	← yes
	cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Shee	t	Note what sheet in the plan set contains the filter course specification	
	:1	Pond side slopes	<u>> 3</u> :1
Shee	t	Note what sheet in the plan set contains the planting plans and surface cover	
If porous p	avement is	proposed:	
Ası	ohalt	Type of pavement proposed (Concrete? Asphalt? Pavers? Etc.)	
0.1	acres	A _{SA} = Surface area of the pervious pavement	
3.9	:1	Ratio of the contributing area to the pervious surface area	≤ 5:1
12.0	inches –	D _{FC} = Filter course thickness	12", or 18" if within GPA
Shee	t C-15	Note what sheet in the plan set contains the filter course spec.	mod. 304.1 (see spec)

- 1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.
- 2. See lines 34, 40 and 48 for required depths of filter media.
- 3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:		
2,808 cf storage provided		

GROUNDWATER RECHARGE VOLUME (GRV) (Env-Wq 1504.12)

	ac	Area of HSG A soil that was replaced by impervious cover	0.40"
0.12	ac	Area of HSG B soil that was replaced by impervious cover	0.25"
1.70	ac	Area of HSG C soil that was replaced by impervious cover	0.10"
0.01	ac	Area of HSG D soil or impervious cover that was replaced by impervious cover	0.0"
0.11	inches	Rd = Weighted groundwater recharge depth	
0.2 ac-in		GRV = AI * Rd	
726	cf	GRV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
	N	Is a stage-storage table attached showing that the GRV can be stored below the low	est
		invert of the outlet structure (if applicable)? Multiple stormwater control measures	may be
		used to meet the GRV requirements.	

Provide calculations/discussion below showing that the project meets the groundwater recharge requirements (Env-Wq 1507.04):

2,808 cf provided in porous pavement				
Stage strorage table NA (no outlet structure)				

Water Quaity Volume Calculations

Town of Durham, New Hampshire

Project #: 5540 Date: 10/6/2025

HSG	Area (ac)	Ratio	WQV		
Α	0.00	1	0.000	in	GRV
В	0.12	0.75	0.090	in	GRV
С	1.70	0.4	0.680	in	GRV
D	0.01	0.16	0.002	in	GRV
			0.772	ac-in	Total GRV = Al*Rd
			2803.12	cf	GRV Conversion (ac-in x 43560sf/ac x 1'/12")
			0.064	af	

2,808 cf (0.064 cf) storage provided in porous pavement



Location: Bio #1, 15" Culvert (HydroCAD Pond #17P)

Project: 5440 Date: 10/6/2025 By: EBS

La	Apron Length, Ft.	Calculated
Tw	Tailwater, Ft.	0.1
Q	Flow, 10 Yr Storm, CFS	0.3
D50	Median Stone Dia., Ft.	Calculated
D	Depth of Stone, In	Calculated
Do	Pipe Diameter, Ft	1.25
W1	Width @ Start, Ft.	Calculated
W2	Width @ End, Ft	Calculated
W	Width of Channel	4

W1:

3(Do)= 3.75 Ft.

Width @ Start: 4 Ft.

D50: $0.02(Q)^{4/3}$ D50= 0.03 Ft.

or 0.4 In.

Median Stone Size: 6 In.

D: 2.25*D50 **Depth of Riprap:** 14 In.

La: If $Tw \le Do/2$: Do/2 = 0.625 Ft.

 $La=1.8Q/Do^{3/2}+7Do \qquad \qquad Tw= \qquad \qquad 0.1 \ \ Ft.$ and $W2=width \ of \ channel$

or

If Tw>Do/2:

 $La=3Q/Do^{3/2}+7Do$

W2=3Do+La

and W2=width of channel

1112 AD 10 AT

Length of Apron:	10 Ft.
Width @ End:	4 Ft.



Location: Bio #2, 15" Culvert (HydroCAD Pond #22P)

Project: 5440 Date: 10/6/2025 By: EBS

La	Apron Length, Ft.	Calculated
Tw	Tailwater, Ft.	0.1
Q	Flow, 10 Yr Storm, CFS	0.44
D50	Median Stone Dia., Ft.	Calculated
D	Depth of Stone, In	Calculated
Do	Pipe Diameter, Ft	1.25
W1	Width @ Start, Ft.	Calculated
W2	Width @ End, Ft	Calculated
W	Width of Channel	4

W1:

3(Do)= 3.75 Ft.

Width @ Start: 4 Ft.

D50: $0.02(Q)^{4/3}$ D50= 0.11 Ft.

or 1.3 In.

Median Stone Size: 6 In.

D: 2.25*D50 **Depth of Riprap: 14 In.**

La: If $Tw \le Do/2$: Do/2 = 0.625 Ft.

 $La=1.8Q/Do^{3/2} + 7Do$ Tw= 0.05 Ft.

and W2=width of channel or

W2=3Do+La

If Tw>Do/2:

 $La=3Q/Do^{3/2}+7Do$ and $W2=width\ of\ channel$

C

Length of Apron:	10 Ft.
Width @ End:	4 Ft.



Location: DMH #12, 15" Culvert (HydroCAD Pond #50P)

Project: 5440 Date: 10/6/2025 By: EBS

La	Apron Length, Ft.	Calculated
Tw	Tailwater, Ft.	3.2
Q	Flow, 10 Yr Storm, CFS	3.23
D50	Median Stone Dia., Ft.	Calculated
D	Depth of Stone, In	Calculated
Do	Pipe Diameter, Ft	1.25
W1	Width @ Start, Ft.	Calculated
W2	Width @ End, Ft	Calculated
W	Width of Channel	4

W1:

3(Do)= 3.75 Ft.

Width @ Start: 4 Ft.

D50: $0.02(Q)^{4/3}$ D50= 0.02 Ft.

or 0.3 In.

Median Stone Size: 6 In.

D: 2.25*D50 **Depth of Riprap: 14 In.**

La: If $Tw \le Do/2$: Do/2 = 0.625 Ft.

 $La=1.8Q/Do^{3/2} + 7Do$ Tw= 3.15 Ft.

and W2=width of channel or

W2=3Do+La

If Tw>Do/2:

 $La=3Q/Do^{3/2}+7Do$ and $W2=width\ of\ channel$

or

Length of Apron:	16 Ft.
Width @ End:	4 Ft.



Location: Bio #3, 18" Culvert (HydroCAD Pond #69P)

Project: 5440 Date: 10/6/2025 By: EBS

La	Apron Length, Ft.	Calculated
Tw	Tailwater, Ft.	0.4
Q	Flow, 10 Yr Storm, CFS	2.52
D50	Median Stone Dia., Ft.	Calculated
D	Depth of Stone, In	Calculated
Do	Pipe Diameter, Ft	1.50
W1	Width @ Start, Ft.	Calculated
W2	Width @ End, Ft	Calculated
W	Width of Channel	4

W1:

3(Do)= 4.5 Ft.

Width @ Start: 5 Ft.

D50: $0.02(Q)^{4/3}$

Tw(Do)

D50=0.11 Ft.

Do/2=

Tw=

or 1.4 In.

Median Stone Size:

0.75 Ft.

0.4 Ft.

6 In.

D: 2.25*D50

Depth of Riprap: 14 In.

La: If $Tw \le Do/2$:

 $La=1.8Q/Do^{3/2} + 7Do$

and W2=width of channel

or

W2=3Do+La

If Tw>Do/2:

 $La=3Q/Do^{3/2} + 7Do$

and W2=width of channel

or

Length of Apron:	13 Ft.
Width @ End:	4 Ft



Location: Bio #4, 15" Culvert (HydroCAD Pond #83P)

Project: 5440 Date: 10/6/2025 By: EBS

La	Apron Length, Ft.	Calculated
Tw	Tailwater, Ft.	0.1
Q	Flow, 10 Yr Storm, CFS	0.5
D50	Median Stone Dia., Ft.	Calculated
D	Depth of Stone, In	Calculated
Do	Pipe Diameter, Ft	1.25
W1	Width @ Start, Ft.	Calculated
W2	Width @ End, Ft	Calculated
W	Width of Channel	4

W1:

3(Do)= 3.75 Ft.

Width @ Start: 4 Ft.

D50: $0.02(Q)^{4/3}$ D50= 0.06 Ft.

or 0.8 In.

Median Stone Size: 6 In.

D: 2.25*D50 **Depth of Riprap: 14 In.**

La: If $Tw \le Do/2$: Do/2 = 0.625 Ft.

 $La=1.8Q/Do^{3/2}+7Do \qquad \qquad Tw= \qquad \qquad 0.1 \ \ Ft.$ and $W2=width \ of \ channel$

d W2=width of channel or

If Tw>Do/2:

 $La=3Q/Do^{3/2}+7Do$

W2=3Do+La

and W2=width of channel

or Length of Apron: 10 Ft.

W2=3Do+0.4La Width @ End: 4 Ft.



LEVEL SPREADER CALCULATIONS

Location	Location: Bio #3 Outfall (Reach #70R)						
Project:	5440	Date: 10/6/2025	By: EBS				
Based on 1	Based on Maine Volume III BMP's Technical Design Manual						
Level Spre	eader Length	based on 10-Year 24-Hour storn	rm at 0.25 cfs per foot.				
10-Year P	eak Rate:	2.52 cfs					
Required	Length:	10.08 feet					



LEVEL SPREADER CALCULATIONS

Location	Location: Bio #4 Outfall (Reach #83R)					
Project:	5440	Date: 10/6/2025	By: EBS			
Based on I	Maine Volume III	BMP's Technical Desig	gn Manual			
Level Spre	eader Length based	on 10-Year 24-Hour s	torm at 0.25 cfs per foo	ot.		
10-Year P	eak Rate:	0.49 cfs				
Required	Length:	1.96 feet	t			



Section 8

Stormwater Operations & Maintenance Plan Inspection Form Stormwater Management Plan



STORMWATER INSPECTION AND MAINTENANCE MANUAL

RiverWoods Phase II Assessor's Map 209, Lot 33

OWNER:

RiverWoods Company at Durham 14 Stone Quarry Drive Durham, NH 03824

Proper inspection, maintenance, and repair are key elements in maintaining a successful stormwater management program on a developed property. Routine inspections ensure permit compliance and reduce the potential for deterioration of infrastructure or reduced water quality. The following responsible parties shall be in charge of managing the stormwater facilities:

RESPONSIBLE PARTIES:

Owner:	RiverWoods Durham		(603) 590-4001
	Name	Company	Phone
Inspection:	RiverWoods Durham		(603) 590-4001
•	Name	Company	Phone
Maintenance: RiverWoods Durham			(603) 590-4001
	Name	Company	Phone

NOTES:

Written inspection forms and maintenance logs shall be completed yearly by a qualified inspector retained the owner or assigns on or before January 31st of each year and a copy retained at the site's business office.

Photographs of each stormwater BMP are to be taken at each inspection and submitted with the annual inspection reports.

Inspection and maintenance responsibilities shall transfer to any future property owner(s).

This manual shall be updated as needed to reflect any changes related to any transfer of ownership and/or any delegation of inspection and maintenance responsibilities to another entity



BIORETENTION PONDS (AKA RAINGARDENS)

Function – Bioretention ponds provide treatment to runoff prior to directing it to stormwater systems by filtering sediment and suspended solids, trapping them in the bottom of the garden and in the filter media itself. Additional treatment is provided by the native water-tolerant vegetation which removes nutrients and other pollutants through bio-uptake. Stormwater detention and infiltration can also be provided as the filtering process slows runoff, decreases the peak rate of discharge and promotes groundwater recharge.

Bioretention ponds shall be managed (Per AGR 3800 and RSA 430:53) to: prevent and control the spread of invasive plant, insect, and fungal species; minimize the adverse environmental and economic effects invasive species cause to agriculture, forests, wetlands, wildlife, and other natural resources of the state; and protect the public from potential health problems attributed to certain invasive species.

Maintenance

- Inspect annually and after significant rainfall events.
- If a raingarden does not completely drain within 72-hours following a rainfall event, then a qualified professional shall be retained to assess the condition of the facility to determine measures required to restore its filtration and/or infiltration function(s), including but not limited to removal of accumulated sediments and/or replacement or reconstruction of the filter media. Filter media shall be replaced with material matching the specification on the design drawings or the NHDES Stormwater Manual.
- Replace any riprap dislodged from spillways, inlets and outlets.
- Remove any obstructions, litter and accumulated sediment or debris as warranted but no less than once a year.
- Mowing of any grassed area in or adjacent to a raingarden, including its berm, shall be performed at least twice per year (when areas are not inundated) to keep the vegetation in vigorous condition. The cut grass shall be removed to prevent the decaying organic litter from clogging the filter media or choking other vegetation.
- Select vegetation should be maintained in healthy condition. This may include pruning, removal and replacement of dead or diseased vegetation.
- Remove any invasive species, Per AGR 3800 and RSA 430:53.
- Remove any hard wood growth from raingardens.

CULVERTS AND DRAINAGE PIPES

Function – Culverts and drainage pipes convey stormwater away from buildings, walkways, and parking areas and to surface waters or closed drainage systems.

Maintenance

- Culverts and drainage pipes shall be inspected semi-annually, or more often as needed, for accumulation of debris and structural integrity. Leaves and other debris shall be removed from the inlet and outlet to insure the functionality of drainage structures. Debris shall be disposed of on site where it will not concentrate back at the drainage structures or at a solid waste disposal facility.
- Riprap Areas Culvert outlets and inlets shall be inspected during annual maintenance and operations for erosion and scour. If scour or creek erosion is identified, the outlet owner shall take appropriate means to prevent further erosion. Increased lengths of riprap may require a NHDES Permit and/or local permit.

DEEP SUMP CATCH BASINS

Function – Catch basins collect stormwater, primarily from paved surfaces and roofs. Stormwater from paved areas often contains sediment and contaminants. Catch basin sumps serve to trap sediment, trace metals, nutrients and debris. Hooded catch basins trap hydrocarbons and floating debris.

Maintenance

- Remove leaves and debris from structure grates on an as-needed basis.
- Sumps shall be inspected and cleaned annually and any removed sediment and debris shall be disposed of at a solid waste disposal facility.

RIP RAP OUTLETS, SWALES, LEVEL SPREADERS AND BUFFERS

Function – Rip rap outlets slow the velocity of runoff, minimizing erosion and maximizing the treatment capabilities of associated buffers. Vegetated buffers, either forested or meadow, slow runoff which promotes and reduces peak rates of runoff. The reduced velocities and the presence of vegetation encourage the filtration of sediment and the limited bio-uptake of nutrients.

Maintenance

- Inspect riprap, level spreaders and buffers at least annually for signs of erosion, sediment buildup, or vegetation loss.
- Inspect level for signs of condensed flows. Level spreader and rip rap shall be maintained to disperse flows evenly over level spreader.
- If a meadow buffer, provide periodic mowing as needed to maintain a healthy stand of herbaceous vegetation.
- If a forested buffer, then the buffer should be maintained in an undisturbed condition, unless erosion occurs.
- If erosion of the buffer (forested or meadow) occurs, eroded areas should be repaired and replanted with vegetation similar to the remaining buffer. Corrective action should include eliminating the source of the erosion problem and may require retrofit or reconstruction of the level spreader.
- Remove debris and accumulated sediment and dispose of properly.

LANDSCAPED AREAS - FERTILIZER MANAGEMENT

Function – Fertilizer management involves controlling the rate, timing and method of fertilizer application so that the nutrients are taken up by the plants thereby reducing the chance of polluting the surface and ground waters. Fertilizer management can be effective in reducing the amounts of phosphorus and nitrogen in runoff from landscaped areas, particularly lawns.

Maintenance

- Have the soil tested by your landscaper or local Soil Conservation Service for nutrient requirements and follow the recommendations.
- Do not apply fertilizer to frozen ground.
- Clean up any fertilizer spills.
- Do not allow fertilizer to be broadcast into water bodies.
- When fertilizing a lawn, water thoroughly, but do not create a situation where water runs off the surface of the lawn.

LANDSCAPED AREAS - LITTER CONTROL

Function – Landscaped areas tend to filter debris and contaminates that may block drainage systems and pollute the surface and ground waters.

Maintenance

- Litter Control and lawn maintenance involves removing litter such as trash, leaves, lawn clippings, pet wastes, oil and chemicals from streets, parking lots, and lawns before materials are transported into surface waters.
- Litter control shall be implemented as part of the grounds maintenance program.

VEGETATIVE SWALES

Function – Vegetative swales filter sediment from stormwater, promote infiltration, and the uptake of contaminates. They are designed to treat runoff and dispose of it safely into the natural drainage system.

Maintenance

- Timely maintenance is important to keep a swale in good working condition. Mowing of grassed swales shall be monthly to keep the vegetation in vigorous condition. The cut vegetation shall be removed to prevent the decaying organic litter from adding pollutants to the discharge from the swale.
- Fertilizing shall be bi-annual or as recommended from soil testing.
- Inspect swales following significant rainfall events.
- Woody vegetation shall not be allowed to become established in the swales or rock riprap outlet protection and if present shall be removed.
- Accumulated debris disrupts flow and leads to clogging and erosion. Remove debris and litter as necessary.
- Inspect for eroded areas. Determine cause of erosion and correct deficiency as required. Monitor repaired areas.

POROUS PAVEMENT

Function – Porous pavement is designed to capture rainwater runoff containing suspended solids, nutrients and pollutants. Proper maintenance of porous pavement is crucial for ensuring its longevity and functionality to infiltrate runoff.

Maintenance

- Signs shall be installed indicating the location of porous pavement and the special maintenance required.
- New porous pavement shall be inspected several times in the first month after construction and at least annually thereafter. Inspections shall be conducted after major storms to check for surface ponding that might indicate possible clogging.
- Inspect annually for pavement deterioration or spalling.
- Vacuum sweeping shall be performed 2-4 times a year. Power washing may be required prior to vacuum sweeping to dislodge trapped particles.
- Sand and abrasives shall not be used for winter maintenance, as they will clog the pores; deicing materials shall be used instead.
- Never reseal or repave with impermeable materials. If the porous pavement is damaged, it can be repaired using conventional, non-porous patching mixes as long as the cumulative area repaired does not exceed 10 percent of the paved area.

CONTROL OF INVASIVE PLANTS

Function – Invasive plants are introduced, alien, or non-native plants, which have been moved by people from their native habitat to a new area. Some exotic plants are imported for human use such as landscaping, erosion control, or food crops. They also can arrive as "hitchhikers" among shipments of other plants, seeds, packing materials, or fresh produce. Some exotic plants become invasive and cause harm by:

- becoming weedy and overgrown;
- killing established shade trees;
- obstructing pipes and drainage systems;
- forming dense beds in water;
- lowering water levels in lakes, streams, and wetlands;
- destroying natural communities;
- promoting erosion on stream banks and hillsides; and
- resisting control except by hazardous chemical.

Maintenance

During maintenance activities, check for the presence of invasive plants and remove in a safe manner as described in the attached "Methods for Disposing Non-Native Invasive Plants" prepared by the UNH Cooperative Extension.

STREET/PARKING LOT SWEEPING (NON-POROUS PAVEMENT)

Function – Parking lots accumulate sand and debris. Street sweeping removes the sand and debris, which lowers transport of sediment and pollutants the stormwater systems and into the environment.

Maintenance

• A regular periodic cleaning schedule is recommended. The more frequent, the greater the sediment and pollutant removal. Regular cleaning of paved areas reduces the frequency of cleaning catch basins and drainage systems. It is recommended that the parking lots and access ways shall be swept at least once a month during winter months.

GENERAL CLEAN UP

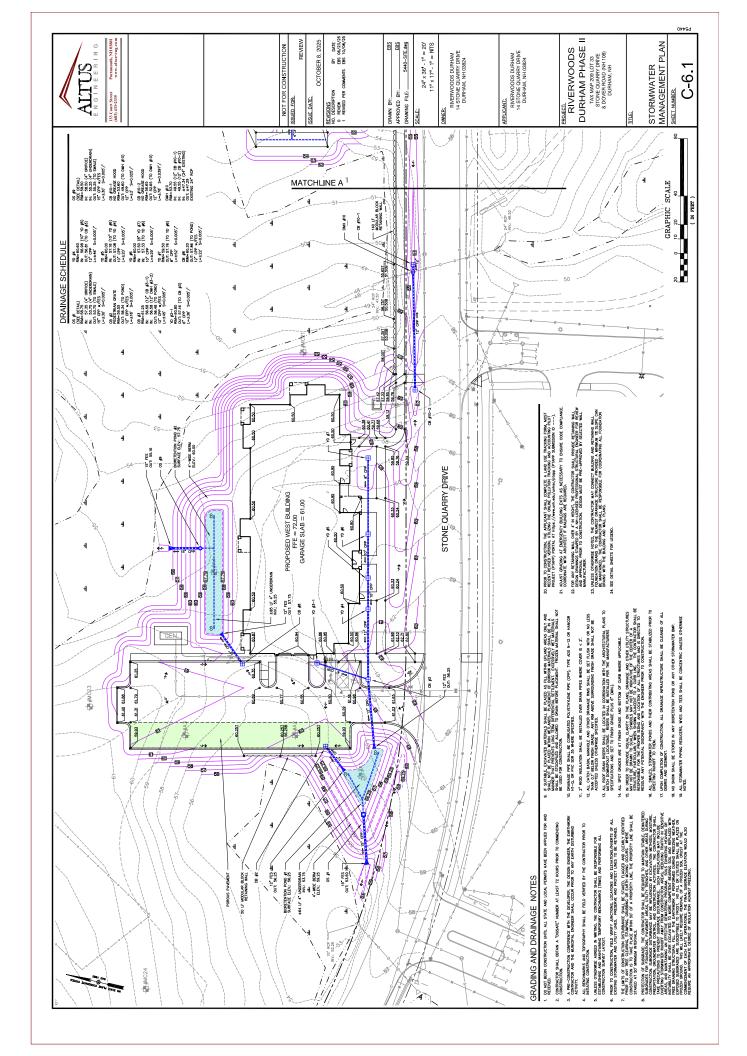
- Upon completion of the project, the contractor shall remove all temporary stormwater structures (i.e., temporary stone check dams, silt fence, temporary diversion swales, catch basin inlet filter, etc.). Any sediment deposits remaining in place after the silt fence or filter barrier is no longer required shall be dressed to conform to the existing grade, prepared, and seeded. Remove any sediment in catch basins and clean drain pipes that may have accumulated during construction.
- Once in operation, all paved areas of the site should be swept at least once annually at the end of winter/early spring prior to significant spring rains.

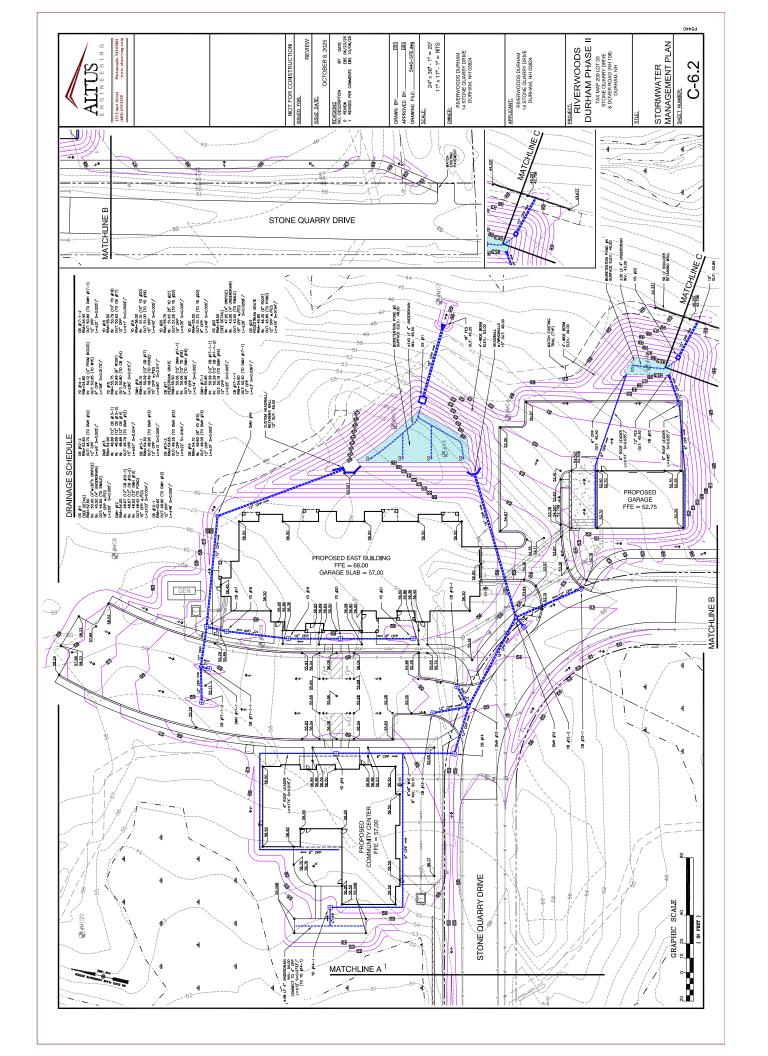
APPPENDIX

- A. Stormwater System Operations and Maintenance ReportB. Site Grading and Drainage Plan

STORM WATER SYSTEM OPERATION AND MAINTENANCE REPORT

General Information							
Project Name							
Owner							
Inspector's Name(s)							
Inspector's Contact Information							
Date of Inspection			Start Time:	End Time:			
Type of Inspection: Annual Report Due to a discharge of significant amounts of sediment							
Notes:							
	General Site Qu	estions and Dis	charges of Significant Amounts of Sedin	nent			
Sub	ject	Status	Notes				
			ndicated by (but is not limited to) observations	of the following.			
Note	e whether any are observed during	g this inspection:					
			Notes/ Action taken:				
1	Do the current site conditions ref the attached site plan?	□No					
2	Is the site permanently stabilized						
	temporary erosion and sediment	□No					
	controls are removed, and stormy						
	discharges from construction acti are eliminated?						
3 Is there evidence of the discharge of							
	significant amounts of sediment						
	surface waters, or conveyance sy	stems					
	leading to surface waters?						
Permit Coverage and Plans							
#	BMP/Facility	Inspected	Corrective Action Needed and Notes	Date Corrected			
	Catch Basins and Yard Drains	□Yes					
		□No					
	Drainage Pipes	□Yes					
		□No					
	Porous Pavement	□Yes □No					
	Riprap Aprons	□Yes					
	Tuptup TipTolls	□No					
Site Vegetation		□Yes					
	Bioretention Ponds	□No □Yes					
		□No					
		□Yes					
		□No					
		□Yes □No					





Section 9

Watershed Plans

Pre-Development Drainage Plan
Pre-Development Soils Plan
Post-Development Drainage Plan
Post-Development Soils Plan



