

	20	0 SCALE 1 IN	20 40							
SCALE: 1 INCH = 20 FT. SUBDIVISION PLAN FOR TOWN & CAMPUS, INC. OF										
TAX IMAP 2, LOT 14-1 72 MAIN STREET DURHAM, NEW HAMPSHIRE										
	09/06/18	REVISIONS PI	ER TOWN CONDITIONS J.F.K. ESCRIPTION BY							
DRA	DRAWN BY: E.J.S. DATE: JULY 11, 2018									
JOB	NO.:	5549	DRAWING NO.: 1 1 SHEET OF							
DOUCET® DOUCET® SURVEY2 Serving Your Professional Surveying & Mapping Needs 102 Kent Place, Newmarket, NH 03857 (603) 659-6560 2 Commerce Drive (Suite 202) Bedford, NH 03110 (603) 614-4060 10 Storer Street (Riverview Suite) Kennebunk, ME (207) 502-7005 http://www.doucetsurvey.com										









SEWER NOTES

GENERAL

CONSTRUCTION OF ALL COMPONENTS OF THE SANITARY SEWER SYSTEM SHALL CONFORM TO THE MOST CURRENT VERSION OF THE NEW HAMPSHIRE CODE OF ADMINISTRATIVE RULES ENV-WQ 700 AND TECHNICAL SPECIFICATIONS ENTITLED "

TYPES OF SEWERS 2.

A. THERE SHALL BE NO CONNECTION BETWEEN SANITARY SEWERS AND STORM SEWERS. B. RUNOFF FROM ROOFS, STREETS, AND OTHER AREAS AND GROUNDWATER FROM FOUNDATION DRAINS, SUMP PUMPS, OR OTHER SUBSURFACE DRAINS SHALL BE EXCLUDED FROM SANITARY SEWERS.

SEWER SIZE AND COVER

- A. MINIMUM PIPE SIZE FOR GRAVITY SEWER MAINS SHALL BE 8 INCHES.
- B. MINIMUM PIPE SIZE FOR GRAVITY SEWER SERVICES SHALL BE 4 INCHES.
- C. MINIMUM PIPE SIZE FOR FORCE MAIN SEWER SERVICES SHALL BE 2 INCHES.
- D. SANITARY SEWERS SHALL HAVE 6 FEET MINIMUM COVER IN ALL ROADWAY LOCATIONS AND 4 FEET MINIMUM COVER IN ALL CROSS-COUNTRY LOCATIONS.

PIPE AND FITTING MATERIALS

A. DUCTILE IRON PIPE

- DUCTILE IRON PIPE AND FITTINGS SHALL CONFORM TO THE FOLLOWING STANDARDS OF THE AMERICAN WATER WORKS ASSOCIATION:
- (1) AWWA C151 FOR DUCTILE IRON PIPE, CENTRIFUGALLY CAST IN METAL OR SAND LINED MOLDS, FOR WATER OR OTHER LIQUIDS; (2) AWWA C150 FOR THICKNESS DESIGN OF DUCTILE IRON PIPE AND WITH ASTM A 536 IRON
- CASTINGS; AND
- (3) JOINTS SHALL BE MECHANICAL TYPE, PUSH-ON TYPE, OR BALL-AND-SOCKET TYPE;

B. PVC (POLY VINYL CHLORIDE) PIPE

PVC PIPE AND FITTINGS SHALL BE APPROVED FOR SEWAGE SERVICE AND CONFORM TO THE

- FOLLOWING: (1) PVC PIPE USED FOR GRAVITY SEWERS SHALL BE TYPE SDR 35 CONFORMING TO ASTM D3034; (2) PVC PIPE USED FOR FORCE MAINS SHALL BE TYPE SDR 26 CONFORMING TO ASTM D2241 OR ASTM D1785;
- (3) JOINTS SHALL BE PUSH-ON, BELL-AND-SPIGOT TYPE HAVING OIL RESISTANT COMPRESSION RINGS OF ELASTOMERIC MATERIAL CONFORMING TO ASTM D3212.

BEDDING 5.

PIPE BEDDING SHALL BE SCREENED GRAVEL AND/OR CRUSHED STONE FREE FROM ORGANIC MATTER, CLAY, AND/OR LOAM MEETING ASTM C33 STONE SIZE NO. 67. BEDDING SHALL EXTEND FROM THE SPRING LINE OF THE PIPE TO A MINIMUM DEPTH OF 6" BELOW THE BOTTOM OF THE PIPE OUTSIDE SURFACE.

100% PASSING 90-100% PASSING 20-55% PASSING 0-10% PASSING 0-5% PASSING

1 INCH SCREEN ³/₄ INCH SCREEN ³/₈ INCH SCREEN #4 SIEVE #8 SIEVE

MANHOLES

A. PRECAST CONCRETE BARREL SECTIONS, CONES, AND BASES SHALL CONFORM TO ASTM C478. B. MANHOLES SHALL BE DESIGNED FOR H-20 LOADING.

- C. HORIZONTAL JOINTS BETWEEN BARREL SECTIONS SHALL BE OF AN OVERLAPPING TYPE WHICH SHALL DEPEND UPON A DOUBLE ROW OF ELASTOMERIC OR MASTIC-LIKE SEALANT FOR WATER TIGHTNESS. D. PIPE TO MANHOLE JOINTS SHALL BE AS FOLLOWS:
- (1) ELASTOMERIC, RUBBER SLEEVE WITH WATERTIGHT JOINTS AT THE MANHOLE OPENING AND PIPE SURFACES;
- (2) CAST INTO THE WALL OR SECURED WITH STAINLESS STEEL CLAMPS; 3) ELASTOMERIC SEALING RING CAST IN THE MANHOLE OPENING WITH SEAL FORMED ON THE

SURFACE OF THE PIPE BY COMPRESSION OF THE RING; AND (4) NON-SHRINK GROUTED JOINTS WHERE WATERTIGHT BONDING TO THE MANHOLE AND PIPE CAN BE OBTAINED.

E. MANHOLES SHALL HAVE A BRICK PAVED SHELF AND INVERT CONSTRUCTED TO CONFORM TO THE SIZE OF PIPE AND FLOW. AT CHANGES IN DIRECTION, THE INVERTS SHALL BE LAID OUT IN CURVES OF THE LONGEST RADIUS POSSIBLE TANGENT TO THE CENTER LINE OF THE SEWER PIPES. SHELVES SHALL BE CONSTRUCTED TO THE ELEVATION OF THE HIGHEST PIPE CROWN AND SLOPED TO DRAIN TOWARD THE FLOWING THROUGH CHANNEL. UNDERLAYMENT OF INVERT AND SHELF SHALL CONSIST OF BRICK MASONRY. INVERTS AND SHELVES SHALL BE PLACED AFTER TESTING.

PROTECTION OF WATER SUPPLIES

- A. THERE SHALL BE NO PHYSICAL CONNECTION BETWEEN A PUBLIC OR PRIVATE WATER SUPPLY SYSTEM AND A SEWER OR SEWER APPURTENANCE WHICH WOULD PERMIT THE PASSAGE OF SEWAGE OR POLLUTED WATER INTO THE POTABLE SUPPLY. NO WATER PIPE SHALL PASS THROUGH OR COME IN CONTACT WITH ANY PART OF A SEWER OR SEWER MANHOLE.
- B. NO SEWER SHALL BE LOCATED WITHIN THE WELL PROTECTIVE RADII ESTABLISHED IN ENV-WS 300 FOR ANY PUBLIC WATER SUPPLY WELLS OR WITHIN 100 FEET OF ANY PRIVATE WATER SUPPLY WELL.
- C. SEWERS SHALL BE LOCATED AT LEAST 10 FEET HORIZONTALLY FROM ANY EXISTING OR PROPOSED WATER MAIN.
- D. A DEVIATION FROM THE SEPARATION REQUIREMENTS OF (B) OR (C) ABOVE SHALL BE ALLOWED WHERE NECESSARY TO AVOID CONFLICT WITH SUBSURFACE STRUCTURES, UTILITY CHAMBERS, AND BUILDING FOUNDATIONS, PROVIDED THAT THE SEWER IS CONSTRUCTED IN ACCORDANCE WITH THE FORCE MAIN CONSTRUCTION REQUIREMENTS SPECIFIED IN ENV-WQ 704.06.
- E. WHENEVER SEWERS MUST CROSS WATER MAINS, THE SEWER SHALL BE CONSTRUCTED AS FOLLOWS: (1) VERTICAL SEPARATION OF THE SEWER AND WATER MAIN SHALL BE NOT LESS THAN 18 INCHES, WITH WATER ABOVE SEWER; AND

(2) SEWER PIPE JOINTS SHALL BE LOCATED AT LEASE 6 FEET HORIZONTALLY FROM THE WATER MAIN

STANDARD TRENCH NOTES - SEWER

- 1. ORDERED EXCAVATION OF UNSUITABLE MATERIAL BELOW GRADE SHALL BE REPLACED WITH BEDDING MATERIAL. SEE ALSO NOTE 4.
- BEDDING: SCREENED GRAVEL AND/OR CRUSHED STONE FREE FROM ORGANIC MATTER, CLAY, AND/OR LOAM MEETING ASTM C33 STONE SIZE NO. 67.

100% PASSING	1 INCH SCREEN
90-100% PASSING	³ / ₄ INCH SCREEN
20-55% PASSING	³ / ₈ INCH SCREEN
0-10% PASSING	#4 SIEVE
0-5% PASSING	#8 SIEVE

- 3. SAND BLANKET: CLEAN SAND FREE FROM ORGANIC MATTER, SO GRADED THAT 100% PASSES A $\frac{1}{2}$ INCH SIEVE AND NOT MORE THAN 15% PASSES A #200 SIEVE.
- 4. SUITABLE MATERIAL: IN ROADS, ROAD SHOULDERS, WALKWAYS, AND TRAVELED WAYS, SUITABLE DURING THE COURSE OF CONSTRUCTION, AFTER EXCLUDING DEBRIS, PIECES OF PAVEMENT, ORGANIC MATTER, TOP SOIL, WET OR SOFT MUCK, PEAT OR CLAY, EXCAVATED LEDGE MATERIAL AND ALL ROCKS OVER SIX INCHES IN LARGEST DIMENSION, OR ANY MATERIAL NOT APPROVED BY THE ENGINEER.

TRENCH BACKFILL IN CROSS-COUNTRY LOCATIONS SHALL BE SUITABLE MATERIAL AS DESCRIBED ABOVE, EXCEPT THAT TOP SOIL, LOAM, MUCK, OR PEAT MAY BE USED PROVIDED THAT THE COMPLETED CONSTRUCTION WILL BE STABLE AND ACCESS TO THE PIPE FOR MAINTENANCE AND RECONSTRUCTION IS PRESERVED. BACKFILL SHALL BE MOUNDED TO A HEIGHT OF SIX INCHES ABOVE THE ORIGINAL GROUND SURFACE

- 5. BASE COURSE FOR TRENCH REPAIR SHALL MEET THE REQUIREMENTS OF SECTION 300 OF THE LATEST EDITION OF THE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION OF THE STATE OF NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION.
- 6. SHEETING: ALL TRENCH SUPPORTS SHALL CONFORM TO OSHA STANDARDS. CONTRACTOR IS RESPONSIBLE FOR OSHA COMPLIANCE AND WORKER SAFETY THROUGHOUT CONSTRUCTION.
- 7. TRENCH DIMENSIONS: W = MAXIMUM ALLOWABLE TRENCH WIDTH MEASURED 12 INCHES ABOVE THE PIPE. FOR PIPES 15 INCHES NOMINAL DIAMETER (D) OR LESS, W SHALL BE NO MORE THAN 36 INCHES; FOR PIPES GREATER THAN 15 INCHES NOMINAL DIAMETER, W SHALL BE 24 INCHES PLUS THE PIPE OUTSIDE DIAMETER. W SHALL ALSO BE THE PAYMENT WIDTH FOR LEDGE EXCAVATION AND FOR ORDERED EXCAVATION BELOW GRADE. THE MAXIMUM ALLOWABLE TRENCH PAVEMENT PAYMENT WIDTH SHALL BE 8 FEET CENTERED OVER PIPE.
- 8. PIPE INSULATION AT STORM DRAIN CROSSING: INSTALL 2" THICK RIGID FOAM INSULATION OVER SEWER AT STORM DRAIN CROSSINGS, EXTEND INSULATION 4 FEET EITHER SIDE OF STORM DRAIN ALONG SEWER.

MATERIAL FOR TRENCH BACKFILL SHALL BE THE NATURAL MATERIAL EXCAVATED FROM THE TRENCH

INSIDE FACE-

OF MANHOLE

FINISH GRADE

SHEETING -

SEE NOTE 6

SEE NOTE 4

TAPE

STANDARD TRENCH SECTIONS

NOT TO SCALE

NOT TO SCALE

EXISTING GROUND OR FINISH GRADE

BRICK AS REQUIRED TO ADJUST FRAME AND COVER TO GRADE, FIVE COURSES MAXIMUM

— ECCENTRIC CONE SECTION (REINFORCED CONCRETE SLAB RATED FOR H-20 LOADING MAY BE USED WHERE MANHOLE DEPTH IS LESS THAN 6')

> - OVERLAPPING TYPE JOINT SEALED WITH DOUBLE ROW OF BITUMASTIC (TYPICAL)

- PRECAST CONCRETE BARREL SECTIONS AS REQUIRED

- PRECAST CONCRETE BASE

5 FT. MAX. DISTANCE TO FLEXIBLE JOINT PIPE INTO MANHOLE \leftarrow FLOW SECTION A-A' MIN. 0.1' DROP BETWEEN INCOMING AND OUTGOING SEWERS \leftarrow FLOW

MANHOLE INVERT DETAILS NOT TO SCALE

KOR-N-SEAL JOINT SLEEVE

LOCK-JOINT FLEXIBLE MANHOLE SLEEVE

PIPE-TO-MANHOLE CONNECTION DETAIL NOT TO SCALE horizons Engineering SEE NOTE 4 **Civil and Structural Engineering** Land Surveying and Environmental Consulting MAINE • NEW HAMPSHIRE • VERMONT www.horizonsengineering.com COMPACT IN 12" LAYERS SUITABLE MATERIAL CONSTRUCTION DETAILS DETECTABLE WARNING PREPARED FOR SEE NOTE 7 **CLARK PROPERTIES LLC** 12" MIN. SAND BLANKET SEE NOTE 3 74 MAIN STREET TAX MAP 106, LOT 59 BEDDING Ър SEE NOTE 2 DURHAM, NEW HAMPSHIRE 6" MIN. 202020202020202020202 NO. DATE REVISION DESCRIPTION ENG DW SEE NOTE 1 01 2021-07-19 REVISED DRAINAGE DESIGN MCS MCS DATE: PROJECT : EARTH CONSTRUCTION 2021-07-19 NM18054 WITH OR WITHOUT SHEETING DESIGNER: DRAWN BY MJS MCS DATE OF PRINT ARCHIVE # HECKER:

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28 SEPTEMBER 2022

HORIZONS ENGINEERING

GENERAL NOTES:

- 1. Support of Excavation plan used project drawings Clark Properties, LLC C103 Utility Plan, Topographic Plan for Town & Campus, Inc. by Doucet Survy, Inc., and Structural Drawings prepared by Odeh Engineers dated 7/26/2022, 8/21/2018, and 7/23/2021, respectively. For additional information, please reference the architectural and structural drawings for proposed dimensions and locations of proposed structures.
- 2. "Geotechnical Investigation Report, Proposed 74 Main Street Durham, New Hampshire", prepared by John Turner Consulting, Inc. dated April 22, 2022 was used for the subsurface information when designing the Support of Excavation.
- 3. Limits of the Support of Excavation system shown on the drawings are based on the project plans and discussions between HELICAL and the General Contractor. This design has assumed installation by HELICAL Drilling and is not for use by other contractors/installers.
- 4. The excavation support system has been designed for a maximum vertical construction surcharge or 250 psf applied no closer than 2 feet behind the system. If additional surcharge loads are required, HELICAL will need to be made aware.
- 5. All slopes and excavations to be conducted onsite and around the proposed Support of Excavation shall be stable and safe within the applicable OSHA requirements. Any overhead wires shall be deenergized or removed to provide OSHA 10-ft minimum clearance at all times.
- 6. Soldier piles shall be installed at the locations shown and to the minimum depths provided. If any variations are encountered, notify the engineer immediately.
- 7. All existing and proposed below grade utilities shall be marked and identified prior to the start of work. DIG SAFE will be notified by HELICAL but any additional utilities not picked up by DIG SAFE is the responsibility of the General Contractor. If any conflicts are discovered they should be address immediately and prior to mobilization to start the work.
- 8. To prevent the loss of retained earth between lagging board joints, surface drainage shall be graded to not allow it to collect behind the Support of Excavation. The Support of Excavation system is also not designed to retain water and will allow groundwater to seep between lagging boards.
- 9. No lagging excavations shall be left open overnight. If lagging boards cannot be installed prior to end of day, any excavated areas will need to be temporarily backfilled (by Others) until lagging boards can be installed. Speed and frequency of excavation for lagging work shall be at the sole discretion of HELICAL.
- 10. Any temporary work stoppages will need to allow time to allow excavations to be made safe for the period of the shutdown.
- 11. The Lateral movement of the system shall be monitored (by Others) during construction and during use of the system. If the movement exceeds the threshold value of 1 inch HELICAL shall be notified immediately to review the conditions.
- 12. The type of rock removal shall be reviewed with Helical Drilling. Methods used that may reduce the quality of the rock at the soldier pile locations should not be allowed.

MATERIAL NOTES:

- 1. Drilled soldier piles API N80 Drill Casing.
- 2. Grout shall be a either a neat cement grout with a water/cement ratio not to exceed 0.45 or a ready-mix sand grout.
- 3. Either grout used shall have a minimum 28 day design strength of 4,000psi.
- 4. All plates, angles or miscellaneous metals shall ASTM A36 Grade 36 unless noted otherwise. Member sizes of equal or greater capacity than shown may be substituted.
- 5. All welding shall be done by a qualified welder at the sole judgement of HELICAL. Welding electrodes shall be E70xx unless noted otherwise.
- 6. Timber lagging shall be rough sawn nominal 3" thick and 8" wide boards. Lagging boards shall be free from significant deflects and can be either new or used at the sole discretion of HELICAL.

INSTALLATION PROCEDURE NOTES:

- 1. The area along the alignment shall be cleared of all existing utilities and other obstructions prior to pile installation. The soldier piles shall then be installed at the locations shown in the plan. Piles shall be drilled. If utilities are within 2 feet of a pile location, the pile location shall be sleeved by Others to the invert of the utility.
- 2. Working grade is assumed to be the existing grade along Pettee Brook Lane, ranging from approximately El. 56.5 53.
- 3. Piles will then be drilled to their minimum design tip elevations.
- 4. Next grout will be placed in the piles. Grout can be end dumped from the surface as long as there is no standing water or sediment within the soldier pile. If water or sediment is within the soldier pile prior to grouting, grout shall be tremie grouted in place displacing all water and sediment.
- 5. Placement of the inner sleeve will occur after the soldier pile has been fully grouted. Sleeve will be wet set into the fresh grout without delay.
- 6. Core pipe sleeve shall be placed to the pile tip and will be 15-ft long minimum.
- 7. Cable handrail will be installed.
- 8. Installation of lagging boards will be completed in a top down manner. Excavation depth shall be no deeper than 4-ft prior to placement of lagging boards at any time. The number of bays excavated and prepared for lagging placement shall be no more than what can be lagged within a few hour period and at the sole discretion of HELICAL.
- 9. Piles shall be cut below frost depth after building is backfilled and temporary shoring wall is no longer needed.

SCALE	DATE
N.T.S.	9/20/20
DRAWN BY KAD	CHKD : JMC

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APPROXIM ELEVATION PILE WALL	IATE EXISTING GROUND N AT START OF SOLDIER , El. 56.5	START OF PILE				3" x 8" TIMBER LA	GGING BOARDS,	
El.60						TYPICAL.		
El.58		1				5		
El.56	<u></u>						<u> </u>	
El.54								
El.52								
El.50		\square				v		
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El.42			<u>N</u>					×
El.40					:			
El.38	BOTTOM OF EVEN							
El.36	AT A 2H:1V SLOPE	AT THE ENDS OF	THE WALL		2 2			
El.34						BOTTOM OF EXCA	√ATION	
El.32								
El.30								

SCALE	DATE					
3/16" = 1'	9/20/2022					
DRAMN BY KAD	CHKD BY JMC					

NOTE: PILES 10 - 26 WILL BE SLEEVED BY HELICAL

---- POSSIBLE ROCK EXCAVATION FOR SOLDIER PILES 1 - 14. HELICAL WILL DRILL 6 FOOT ROCK SOCKET FROM BOTTOM OF FOOTING OR TOP OF BEDROCK DEPENDING ON PILE LOCATION.

- APPROXIMATE ELEVATION WHERE

PILES WILL BE CUT OFF AFTER FOUNDATION INSTALLATION AND BACKFILLING ·≠≈≠≈≈≈≈≈≈∞<u>₽</u>____ _______ ┥╾┾╼╼╼ _____ _____ _____ - ESTIMATED TOP OF SOUND BEDROCK

PROPOSED BUILDING FFE = El. 56.67 BOTTOM OF DEEPEST FOOTING IS -14' 10" CORRELATING TO El. 41.84

MIXED-USE BUILDING 74 MAIN STREET DURHAM, NH 03824

SUPPORT OF EXCAVATION PLAN

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