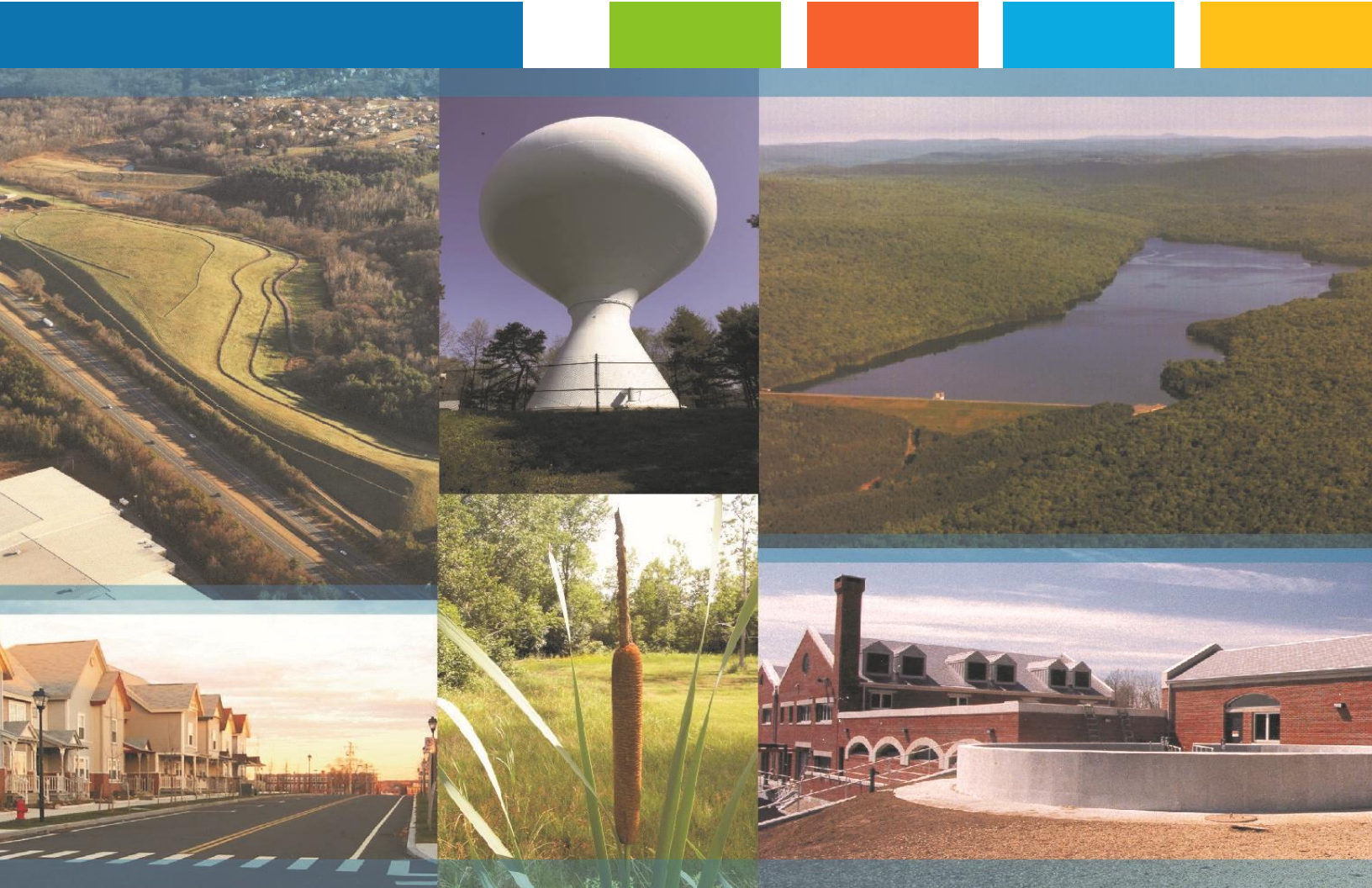




Mill Plaza Redevelopment
Durham, NH

Traffic Impact and Access Study

Colonial Durham Associates

Revised August 14, 2020

Tighe&Bond
Engineers | Environmental Specialists

Executive Summary**Section 1 Introduction**

1.1	Project Description	1-1
1.2	Study Methodology	1-1
1.3	Study Area	1-2

Section 2 Existing Conditions

2.1	Roadway Descriptions	2-1
2.2	Intersection Descriptions	2-2
2.3	Existing Traffic Data	2-2
2.4	Existing Condition Traffic Operations Analysis	2-4

Section 3 No-Build Conditions

3.1	Background Traffic Growth	3-1
3.2	No-Build Traffic Volumes	3-1
3.3	No-Build Condition Traffic Operations Analysis	3-1

Section 4 Build Conditions

4.1	Trip Generation	4-1
4.2	Trip Distribution	4-2
4.3	Build Condition Traffic Volumes	4-2
4.4	Pedestrian Activity	4-3
4.5	Build Condition Traffic Operations Analysis	4-4
4.6	Sight Distance	4-5

Section 5 Conclusion

5.1	Conclusion	5-1
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Section 6 Figures and Tables**Appendix**

- A. Site Plan
- B. Raw Traffic Volume Data
- C. Seasonal Adjustment Factor Worksheets
- D. Capacity Analysis Worksheets
- E. Internal Capture Calculations
- F. Trip Generation Table and Calculations
- G. Intersection Sight Distance and Stopping Sight Distance Tables

Executive Summary

Tighe & Bond completed an evaluation of the transportation impacts for the proposed Mill Plaza Mixed-Use Redevelopment Project (the Project) located at 5 Mill Road, Durham New Hampshire (Site). The following Traffic Impact Study (TIS) was conducted in accordance with New Hampshire Department of Transportation standards and Town of Durham input. This study analyzes existing conditions and forecasted changes generated by the Project for the expected 2021 build year and a ten-year horizon of 2031.

The Site is currently comprised of a shopping center with two buildings totaling approximately 54,300 square feet (sf) that is occupied by a supermarket, a bank, small shops, a cafe and restaurants. The proposed Project will replace the existing building located at the southeast corner of the site with two buildings (one three story and one four story) that will include 5,000 sf of Restaurant space, 2,000 sf of Drive-In Bank, 19,700 sf of Retail space, and Residential Apartments with 258 total bedrooms. The apartments are near the University of New Hampshire (UNH) and leasing will be targeted to students of UNH. Therefore, for the purposes of this study, the residents have been assumed to be students, however there will be no residency restrictions. Following the redevelopment, the Project will result in a net increase of 500± sf of shopping-center use; 2,000± sf Drive-In Bank use; 22,400± sf of general office use; and 258 bedrooms for residential apartments (off campus student housing).

The analysis provided in this TIAS is based on traffic and pedestrian data collected in March 2017, May 2018, and October 2019 at the following intersections:

- Newmarket Road/Dover Street/Main Street
- Main Street/Madbury Road
- Main Street/Mill Road
- Mill Road/Mill Plaza Site Driveway

The traffic data collected included both weekday evening and Saturday midday peak-hour turning movement counts (TMCs) and a 24-hour automatic traffic recorder (ATR) counts. Vehicle classification data were also collected as part of the traffic counts. Four future scenarios were also analyzed; 2021 opening year No-Build condition and Build conditions, and a ten-year horizon 2031 No-Build and Build conditions. The opening year scenarios represent estimated traffic patterns at the projected opening year. Ten-year horizon scenarios represent traffic patterns ten years after the project opening, and accounts for background traffic growth.

The analysis uses a hybrid trip generation methodology that includes projecting existing retail related site trips plus data provided by the Institute of Transportation Engineers (ITE). It is also important to note that the Project will replace some of the retail space, adding a drive-in bank (with a drive-thru), office space, and off-campus student housing, converting this Site to a multi-use complex. Some of the new trips will be captured within the premises of the Site, thus reducing the overall trip generation. Based on this methodology, the Project is expected to generate approximately 65 new vehicular trips during the evening peak hour and 66 trips during the Saturday midday peak hour. It is expected 133 pedestrian trips will be generated during the evening peak hour.

The Site will be subject to an active parking management program which will monitor parking activity and restrict parking associated with the residential use in addition to restricting other non-commercial related parking activities. The parking management program will include monitoring and enforcement activities to ensure compliance with the on-site parking restrictions. This approach to parking management should limit the total trips associated with the residential use, however to provide a conservative assessment of potential traffic impacts, the TIS includes the full trip generation for the residential component of the Project

The site driveway traffic volumes, collected by the traffic counts, were substantially higher than trip generation estimates calculated utilizing the ITE Trip Generation Manual for the same land use. Current parking usage by non-retail customers is inflating site generated traffic at the site driveway. This TIS takes the conservative assumption that the same parking pattern will occur after the Project is constructed and no reduction of observed site traffic was considered in the analysis. Nonetheless, we understand that the Applicant proposes to restrict the Build condition to business-invitees only, and that neither apartment residents nor non-customer third-parties will have on-site parking privileges.

A detailed traffic operations analysis was conducted at the study area intersections for the Existing, Opening Year No-Build and Build, and Ten-Year Horizon No-Build and Build condition scenarios. Based on the traffic operations, and even with the use of the conservative traffic modeling assumptions, there is sufficient capacity to accommodate the Project related trips and the Project will have minimal impact to traffic operations at the study area intersections.

Intersection and stopping sight distance measurements were also conducted at the site driveway. Based on the observations, there is sufficient sight distance in both directions at the site driveway for speeds that exceed the posted speed limit.

Based on the analysis provided in the TIS, the Project can be constructed and occupied without the need for any additional roadway or intersection modifications or mitigation.

Potential pedestrian facility improvements should be reviewed to improve pedestrian crossings due to the additional generated pedestrian trips as a result of the project. One potential option is the installation of a Rectangular Rapid Flash Beacon (RRFB) on Mill Road just north of the site driveway to improve safety for students traveling to and from the University buildings to the west of the proposed development. In addition, the proposed location of the residential buildings on the Site in the eastern extent will generate additional pedestrian traffic at the Main Street at Madbury Road intersection via the existing pedestrian walkway. The intersection of Main Street at Madbury Road currently features existing crosswalks. A review of the intersection indicates that the Town should consider shifting the Main Street crosswalk to the west to shorten the crossing distance.

Section 1

Introduction

1.1 Project Description

Tighe & Bond conducted an evaluation of transportation impacts, on behalf of Colonial Durham Associates, for the proposed redevelopment of a shopping plaza (the Project) located at 5 Mill Road in Durham, New Hampshire (the Site). The Site currently consists of two shopping-center buildings totaling approximately 55,300± square feet (sf). The existing businesses include a supermarket, multiple smaller retail stores, a bank, and small restaurants. The Site can be accessed by motor vehicles through a three-lane driveway located on Mill Road, with one lane for entering vehicles and two lanes for exiting vehicles. The Site also has pedestrian access points connecting to Mill Road between Memorial Park and Commons Way and a pedestrian path that connects to Main Street just to the west of Madbury Road. The Project will demolish the shopping-center building located at the south-east corner of the parcel and replace it with two new buildings consisting of residential apartments (marketed to, and assumed to be off-campus student housing for UNH students), a bank with drive-thru, office, restaurant and retail space. The project will result in a net increase of 500± sf of shopping-center uses, 2,000± sf of bank with drive-thru, 22,400± sf of general office uses, and 258 bedrooms in the residential apartments (student housing). A site layout plan is attached in Appendix A.

1.2 Study Methodology

This Traffic Impact Study (TIS) assesses the potential traffic impacts of the Project by analyzing the following scenarios. The weekday evening and Saturday midday peak periods were analyzed for each scenario. These two time periods were selected due to the fact that peak traffic in this type of development occurs with the evening or Saturday midday when shopping activities are overlapping with dining and commuting peak periods. The weekday morning peak hour was not analyzed due to the low number of existing network trips and low trip generation as a result of the proposed development during the morning peak hour, which would therefore not make this the critical peak period.:

- Existing (2019) Traffic Conditions – existing traffic volumes and roadway/intersection geometry and traffic controls
- Opening Year (2021) No Build Conditions (without the Project) – existing volumes plus background traffic growth with existing geometry and controls
- Opening Year (2021) Build Conditions (with the Project) – existing volumes plus background traffic growth and Project traffic volumes with existing geometry and controls
- Horizon Year (2031) No Build Conditions (without the Project) – existing volumes plus background traffic growth with existing geometry and controls
- Horizon Year (2031) Build Conditions (with the Project) – existing volumes plus background traffic growth and Project traffic volumes with existing geometry and controls

Traffic impact will be assessed by comparing the traffic operations under the No-Build conditions, without the Project, to the Build conditions, with the Project.

1.3 Study Area

The study area for the Project was developed based on our understanding of the roadway traffic flow in this area, site development characteristics, and input from the Town of Durham staff. **Figure 1** presents the study area, which includes the following intersections:

- Mill Road at the Mill Plaza Site Driveway
- Mill Road at Main Street
- Main Street at Madbury Road
- Main Street/Dover Road at Newmarket Road/Dover Road (signalized)

Existing lane configuration and traffic control for the intersections are shown in **Figure 2**.

Section 2

Existing Conditions

This section includes a description of existing study area roadway geometry, intersection geometry, intersection traffic control, and traffic volumes within the study area.

2.1 Roadway Descriptions

Existing roadways in the proximity of the Site and within the study area are described below:

Main Street serves the local population as a business and retail district, with small shops, offices, and residential units, with first-floor storefronts along the sidewalk. Main Street runs east / west in the study area. Main Street transitions between two-way and one-way traffic operations in the study area. Main Street carries two-way traffic west of Pettee Brook Lane, provides two lanes running one-way eastbound from Pettee Brook Lane to Madbury Road, and transitions back to two-way traffic east of Madbury Road, providing a two-lane segment between Madbury Road and Park Court, and a three-lane two-way segment between Park Court and Newmarket Road. Parallel and angled on-street parking is provided along the one-way section of the road. Sidewalks, on both sides of the road, and multiple pedestrian crossings are provided throughout the corridor. Bicycle accommodations, such as shared lanes and dedicated bicycle lanes, are provided in the proximity of the Site. A bus stop curb cutout and shelter are located just west of Mill Road. The speed limit on Main Street is 25 miles per hour in the study area.

Mill Road is a two-lane, two-way street that runs north/south, with turn lane pockets at major intersections and driveways. Sidewalks, on both sides of Mill Road, and pedestrian crossings are provided in the proximity of the Site. Five-foot dedicated bicycle lanes run along both sides of the road in the vicinity of the project. The speed limit on Mill Road is 25 miles per hour in the vicinity of the site. The site driveway is located on Mill Road and features a southbound left turn lane into the site plus a single through lane in each direction.

Madbury Road is a one-lane one-way street that runs south to north between Main Street and the intersection with Pettee Brook Lane. Sidewalks, on both sides of Madbury Road, and multiple pedestrian crossings are provided in the proximity of the Site. A five-foot dedicated bicycle lane runs along the east side of the road, while a ten-foot parking lane runs on the west side of the road. The speed limit on Madbury Road is 25 miles per hour in the vicinity of the site.

2.2 Intersection Descriptions

Existing conditions at the study area intersections are described below:

Main Street at Mill Road

Mill Road intersects Main Street from the south to form a three-legged unsignalized intersection. The Main Street one-way eastbound approach consists of two through lanes and the movement from Main Street to Mill Road operates under free flow right turn. The Mill Road northbound approach consists of two right turn lanes that operate under a stop control. Painted pedestrian crosswalks are provided across each approach.

Main Street at Madbury Road

Madbury Road intersects Main Street from the north to form a three-legged unsignalized intersection. The Main Street eastbound approach consists of a channelized left turn lane and a through lane that both operate under free flow conditions. Main Street westbound approach consists of a right turn only lane that operates under a stop control. Madbury Road provides a single one-way northbound departing lane from the intersection with Main Street. Painted pedestrian crosswalks are provided across each approach.

Main Street/Dover Road at Newmarket Road/Holiday Inn Hotel Driveway

Newmarket Road and the Holiday Inn Express Hotel Driveway intersect Main Street and Dover Road from the north and the south to form a four-legged signalized intersection. The Main Street eastbound approach consists of an exclusive left turn lane, a through lane, and a channelized right turn lane. The Dover Road westbound approach consists of a right/through lane and an exclusive left turn lane. Newmarket Road northbound approach consists of a left/through lane and an exclusive right turn lane. The hotel driveway southbound approach consists of a single lane exiting for all movements. Pedestrian crosswalks are provided across each of the local roadway approaches.

The signal operates with two distinct signal timings. From 12:00 a.m. to 4:00 p.m., it operates with two phases and an exclusive actuated pedestrian phase with an overall cycle length of 120 seconds and most of the time given to the westbound left movement and northbound right movement. From 4:00 p.m. to 12:00 a.m., the signal operates with the same phase set up, but a cycle length of 110 seconds with the majority of the time given to the northbound approach.

2.3 Existing Traffic Data

Turning movement counts (TMCs) including vehicle classification were conducted on March 28, 2017, May 8, 2018 and October 24, 2019 during the weekday evening peak period (4:00 p.m. – 6:00 p.m.) and May 5, 2018 and October 26, 2019 during the Saturday Midday peak period. The traffic classification counts included cars, heavy vehicles, pedestrian, and bicycle movements. Counts conducted in 2017 and 2018 were adjusted by a 0.5% annual growth rate to represent 2019 existing traffic conditions. The raw TMC data is provided in Appendix B for reference.

Due to the nature of travel patterns, counts collected on any specific date may not resemble the peak condition of the roadway usage throughout the year. For this reason, NHDOT requires that collected traffic data be seasonally adjusted to the peak month conditions. The adjustment factors are provided by the NHDOT and resemble the regional travel patterns. To obtain a more representative factor, an average of each factor for the past three years was used. The traffic data collected in March, May and October were adjusted up by 13%, 5% and 5%, respectively. The worksheet with seasonal adjustment factor calculations is included in Appendix C.

Lastly, since the counts were collected on different days, the number of cars leaving one intersection may not match the number of cars arriving at the next intersection within the same peak hour. To account for these discrepancies, traffic volumes were balanced by conservatively increasing counts in areas of discrepancies and excluding outliers where multiple data points were available. The 2019 Existing traffic volumes for the weekday evening peak hour and Saturday midday peak hour are presented in **Figures 3** and **4**, respectively.

Automatic traffic recorder (ATR) counts were also conducted on Mill Road north of the Site Driveway and on the Site Driveway east of Mill Road. The ATRs were set on May 8th and collected 24 hours of traffic volume and classification data. **Table 2-1** summarizes the traffic volume data from the ATR counts. The raw ATR data is included in Appendix B.

TABLE 2-1

Existing Daily Traffic Volume

Location	DT ¹	Evening Peak Hour		
		Volume ²	K ³	Dir Dist ⁴
Mill Road South of Main Street	7,403	417	6%	52% NB
Plaza Driveway East of Mill Road	4,515	260	6%	54% EB

Source: Based on ATR counts conducted in May 2018

1. Daily traffic volume (vehicles per day)
2. Peak hour traffic volumes (vehicles per hour)
3. Percentage of daily traffic that occurs during the peak hour
4. Directional distribution of peak hour traffic volumes

2.4 Existing Condition Traffic Operations Analysis

Capacity and queue analyses were performed for the unsignalized study intersections for the Existing Condition traffic volumes during the weekday evening and Saturday midday peak periods using Trafficware Synchro 10 – Traffic Analysis Software (Synchro). The software conducts the analyses of unsignalized intersections based upon the methodology provided in the *Highway Capacity Manual (HCM), 2010*. The analysis results are categorized in terms of Level of Service (LOS), which describes the qualitative approach operational conditions based on the calculated average delay per vehicle. The queue analysis results are summarized in terms of the 95th percentile queue length. The 95th percentile queue length represents the design queue length under peak traffic conditions. The level of service and queue results are presented in **Table 6-1** and **Table 6-2**, respectively. The detailed analysis sheets are provided in the Appendix D.

Capacity and queue analyses were also performed for the signalized study intersection under Existing Condition traffic volumes during the weekday evening and Saturday midday peak hours using Synchro. The software conducts the analysis of the signalized intersection using the HCM 2000 methodology, which provides the LOS, average delay per vehicle, and V/C ratio for each approach and the whole intersection. The queue analysis also provides 50th percentile queue length, and 95th percentile queue length. The 50th percentile queue length represents the approximate average queue length.

Lastly, due to the geometric constraints and the complex layout of Madbury Road and Main Street intersection, it cannot be analyzed using HCM standard methodology. Therefore, capacity analyses were performed using Synchro SimTraffic 10 microsimulation software.

As shown in **Table 6-1**, all movements at the study area intersections operate at LOS D or better under existing conditions during both peak periods except for the shared northbound left-through movement at the intersection of Main Street and Dover Road at Newmarket Road and the Hotel Driveway, which operates at LOS E during the weekday evening and Saturday midday peak hours. Overall, the signalized intersection operates at a LOS B under the afternoon and Saturday midday peaks. The longest queue occurs on Main Street eastbound through movement, due to the preferential treatment of the Dover Road westbound left turn. There is minimal queuing at the remaining study intersections.

Section 3

No-Build Conditions

The 2021 and 2031 No-Build Condition reflect the future scenario that incorporates anticipated traffic volume changes associated with general background traffic growth and other planned developments that will affect travel patterns throughout the study area. The 2021 and 2031 No-Build Conditions do not include the Project-related impacts.

3.1 Background Traffic Growth

The methodology to account for future background traffic growth, independent of large development projects may be affected by changes in demographics, smaller scale development projects, or other local developments unforeseen at this time. Based on the data obtained from the continuous count station located on Route 4, east of Route 108, annual average daily traffic has been declining from 2009 to 2018. To remain conservative and to account for any unforeseen potential growth in traffic volumes, a half percent annual growth rate was applied to the existing traffic volumes to develop the future volumes.

The second part of the methodology identifies any specific planned developments that are expected to affect traffic patterns throughout the study area within the future analysis time horizon. Based on input from the Town of Durham, there are no major planned development projects that will impact the study area.

3.2 No-Build Traffic Volumes

The growth rate of a half percent was applied to the 2019 Existing Condition traffic volumes to develop the resulting 2021 No-Build and 2031 No-Build traffic volumes during the weekday evening and Saturday midday peak hours. The volumes under No-Build conditions are shown in **Figures 5** through **8**.

3.3 No-Build Condition Traffic Operations Analysis

The 2021 and 2031 No-Build conditions scenarios analyses use the same methodology as the 2019 Existing condition scenario analysis. **Table 6-1** summarizes the operational analyses for the weekday evening and Saturday midday peak hours. The detailed analyses reports are provided in the Appendix D.

As shown in **Table 6-1**, the study area intersections operate at the same overall LOS under 2021 No-Build conditions when compared to 2019 Existing Conditions except at the intersection of Main Street/Dover Road and Newmarket Road/Hotel Driveway intersection which experiences a one category reduction in overall LOS due to an increase in delay of less than one second during the weekday evening peak hour. The 2031 Horizon No-Build Conditions does not experience significant degradation in LOS as compared to the 2021 No-Build conditions with an increase in average delay of less than two seconds resulting in the reduction to LOS C. Based on the results of the analyses, it is expected that there will be sufficient capacity throughout the study area to accommodate traffic volume growth over the next ten years.

Section 4

Build Conditions

As previously summarized, the Project is located at 5 Mill Road in Durham, New Hampshire. The project will have a net increase of 500± sf of shopping-center uses, 2,000± sf of bank with drive-thru, 22,400± sf of general office uses, and 258 bedrooms in the residential apartments (off-campus student housing).

4.1 Trip Generation

Determining the future trip generation of the Project is a multi-step process. First, the existing traffic generated by the current land uses on the Site was determined based on traffic data collected at the site driveway. Volumes observed at the driveway were higher than anticipated when compared to the trip generation based on rates in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*. The higher volumes are likely a result of non-site related parking on the Site generated by other uses on other sites. This practice will be restricted and monitored as part of the overall site parking management plan under the proposed Project. This approach to parking management should limit the total trips associated with the residential use, however to provide a conservative assessment of potential traffic impacts, the TIS includes the full trip generation for the residential component of the Project

To estimate the number of trips expected to be generated by the Project, two methods were used. Since the total square footage and the uses of the Shopping Plaza will remain similar to the existing conditions, adding approximately 500± sf or 1% in square footage, retail trip generation was calculated by proportionally projecting the current volumes at the driveway. For the Drive-In Bank, General Office, and Student Housing, the ITE Trip Generation Manual was used to estimate the site generated traffic based on the following Land Use Codes (LUCs).

Land Use Code 225 – Off-Campus Student Housing. This land use code refers to student apartment complex that serves nearby colleges or universities. These properties are usually located within walking distance from the institution as is the case with this Site. Most apartments include student-related amenities and typically have washers and dryers in each unit. Trip generation estimates are based on number of bedrooms.

Land Use Code 710 – General Office Building. This land use code refers to an office building that houses multiple tenants. This is a location where typical businesses, commercial, or industrial organizations conduct their affairs. Trip generation estimates are based on average trip rate per 1,000 sf.

Land Use Code 912 – Drive-In Bank. This land use code refers to banking facilities that provide services to allow drivers to conduct financial transactions from their vehicles as well as serve patrons who walk into the building. Trip generation estimates are based on average trip rate per 1,000 sf.

It is also expected that not all trips between the mix of land uses on the Site will be generated outside of the Project in the future condition as the development will now be mixed use. Trips that originate and end within the Site, without entering public roadways, are classified as “Internal Capture” trips. Internal Capture was estimated in accordance

with the methodology described in the National Cooperative Highway Research Program (NCHRP) 684 Report. Internal capture calculation worksheets are included in Appendix E.

Based on the methodology described, the trip generation estimates for the Project are summarized in **Table 4-1** and **4-2** for the weekday evening and Saturday midday peak hours. Detailed calculations are provided in Appendix F. As shown, the Project is expected to generate approximately 558 trips during an average weekday evening peak hour and 404 trips during the Saturday midday peak hour. This represents a total of 65 additional trips during the weekday evening peak hour and 66 additional trips during the Saturday midday peak hour when compared to the existing use on the site.

TABLE 4-1

Weekday Evening Peak Hour Trip Generation Estimates

	Existing Trips	Project Generated Trips	Net New Trips
Enter	254	276	22
Exit	239	282	43
Total	493	558	65

TABLE 4-2

Saturday Midday Peak Hour Trip Generation Estimates

	Existing Trips	Project Generated Trips	Net New Trips
Enter	169	202	33
Exit	169	202	33
Total	338	404	66

4.2 Trip Distribution

The trip distribution identifies the various travel paths for vehicles arriving and leaving the site. Trip distribution patterns for the Project were based on existing traffic patterns throughout the study area. The distribution pattern calculated with the existing traffic volume data is shown in **Figure 9**. The trip distribution pattern used for the Project-generated trips are illustrated in **Figures 10** through **12** for the commercial, residential, and office uses, respectively.

4.3 Build Condition Traffic Volumes

The Project-generated evening peak hour and Saturday midday peak hour vehicle trips were assigned to the study area roadway network based on the trip distribution detailed in the previous section with the resulting Project-Generated trips presented in **Figures 13** and **14**, respectively. The 2021 and 2031 Build Conditions evening and Saturday midday peak hour traffic volumes were then calculated by adding the Project-Generated trips/redistributions to the 2021 and 2031 No-Build volumes. The 2021 Build Condition weekday evening and Saturday midday peak hour traffic volumes are shown in **Figure 15** and **Figure 16**, respectively. The 2031 Build Condition weekday evening and Saturday midday peak hour traffic volumes are shown in **Figure 17** and **Figure 18**, respectively.

4.4 Pedestrian Activity

The Site is located in the downtown area of Durham with a robust sidewalk network connecting local uses. To assess the pedestrian activity, pedestrian volumes at intersections and crosswalks were collected concurrently with the vehicular traffic data. Existing pedestrian volumes are shown in **Table 4-3** and **Figure 19**. Future pedestrian volumes were estimated using the proportionality of the existing retail to the future retail (1% expansion), and by assuming that half of students living in the off-campus housing will be active pedestrians during the evening peak hour. These pedestrians were distributed proportionally to the existing pedestrian volumes. The Project-generated pedestrian trips and the future pedestrian volumes are summarized in **Table 4-3** and shown in **Figures 13** and **14**, respectively.

It was also noted that not all pedestrians cross Main Street at intersections or designated crosswalks. Counts collected in May 2018 showed that in the segment of Main Street between Mill Road and Madbury Road, approximately 70 pedestrians crossed the street at non-designated crossing locations.

TABLE 4-3

Weekday Afternoon Pedestrian Trip Generation Estimates – Evening Peak Hour

	Existing	Commercial Trips Generated	Residential Trips Generated	Future Trips
Mill Road at Driveway	44	1	15	60
Mill Road Midblock Intersection	169	1	59	229
Main Street across Mill Road	91	1	31	123
Main Street at Madbury Road	70	1	24	95
Total	374	4	129	507

As shown in **Table 4-3**, the Project is expected to generate approximately 133 pedestrian trips during an average weekday evening peak hour. These assumptions were incorporated into the traffic capacity analyses to assess the impact of the additional pedestrian crossings on traffic operations.

It is anticipated that students traveling to and from the University buildings to the west of the proposed development will cross Mill Road at the existing crosswalk, just north of the site driveway. One potential option that could be considered to improve pedestrian safety and mobility includes the installation of a Rectangular Rapid Flash Beacon (RRFB) at this location as part of off-site improvements for the development. In addition, the proposed location of the residential buildings on the Site in the eastern extent will generate additional pedestrian traffic at the Main Street at Madbury Road intersection via the existing pedestrian walkway adjacent to the Grange. The intersection of Main Street at Madbury Road currently features existing crosswalks. A review of the intersection indicates that the Town should consider shifting the Main Street crosswalk to the west to shorten the crossing distance.

4.5 Build Condition Traffic Operations Analysis

The 2021 and 2031 Build Condition analyses use the same methodology described in Section 2.4. The level of service and queueing results of the intersection analyses are presented in **Tables 6-1** and **6-2**, respectively.

As shown in **Table 6-1**, most movements at the study area intersections will continue to operate with similar LOS in the 2021 Build as the No-Build conditions. The northbound right movement at the intersection of Main Street at Mill Road experiences a degradation in LOS from C to D during the weekday evening peak period and from B to C during the Saturday midday peak period due to minor increases in delay. The westbound approach of Main Street at Madbury Road experiences a degradation in LOS from B to C during the weekday evening peak hour and from C to D during the Saturday midday peak hour as well. A review of the queue analysis for the 2021 Build Conditions show minimal increases in queueing of less than one vehicle length (25 feet) except at the Main Street northbound right movement approaching Mill Road, which experiences an increase of approximately one vehicles.

Under 2031 Build Conditions, the majority of study area intersections operate with similar LOS and experience minimal increases in delay relative to the 2031 No Build condition. The northbound right movement at the intersection of Main Street at Mill Road experiences a degradation in LOS from C to D and B to C during the weekday evening and Saturday midday peak hours, respectively. A review of the queuing analysis shows minor increases of less than two vehicle lengths at study area intersections as compared to the 2031 No Build condition.

Overall, the analysis shows that the Project will have minimal impact to the queuing and delays throughout the study area and that there is sufficient capacity to accommodate the Project-generated traffic volumes without any additional roadway or intersection mitigation.

4.6 Sight Distance

Sight distance measurements were conducted along Mill Road and Main Street at the location of each driveway. Posted Speed Limit along Mill Road and Main Street is 25 miles per hour (MPH). Stopping sight distance (SSD) measures the distance required for drivers to perceive a two-foot tall object within the roadway and come to a complete stop. Intersection sight distance (ISD) measures the required sight distance available to the driver at the minor approach looking in each direction allowing them to assess and select an acceptable gap to enter the main road. **Tables 4-4** and **4-5** present the ISD and SSD, respectively, based on what is available and the minimum required in the American Association of State Highway and Transportation Officials (AASHTO) publication titled "A Policy on Geometric Design of Highway and Streets". AASHTO tables used for the study are included in Appendix F. The measurements indicate that the available ISD is sufficient for roadway speeds of 40 or 55 miles per hour, while the SSD is sufficient for roadway speeds of 30 or 45 miles per hour, well above the posted speed limits of 25 miles per hour.

TABLE 4-4

Intersection Sight Distance

Driveway	<u>Left</u>		<u>Right</u>	
	Calculated SSD	Available SSD	Calculated SSD	Available SSD
Mill Road Driveway	495 ²	500+	305 ¹	305+
Main Street Driveway	495 ²	500+	495 ²	500+

Notes:

1. SSD sufficient for 40 MPH.
2. SSD sufficient for 55 MPH.

TABLE 4-5

Stopping Sight Distance

Driveway	<u>Left</u>		<u>Right</u>	
	Calculated SSD	Available SSD	Calculated SSD	Available SSD
Mill Road Driveway	500 ²	500+	290 ¹	305+
Main Street Driveway	500 ²	500+	500 ²	500+

Notes:

1. SSD sufficient for 30 MPH.
2. SSD sufficient for 45 MPH.

Section 5 Conclusion

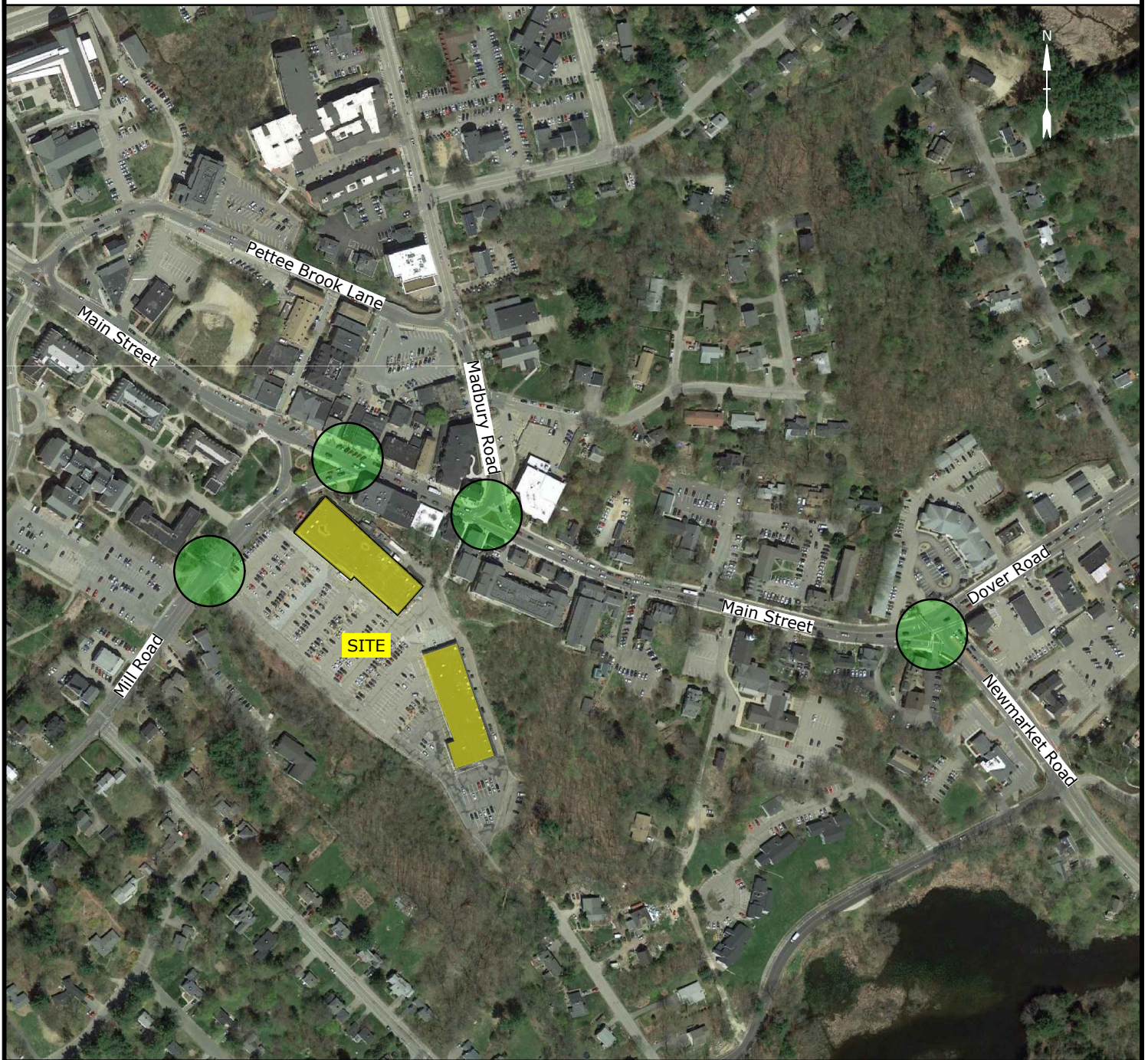
5.1 Conclusion

This TIS evaluation includes a review of traffic operations at the study area intersections under five different analysis scenarios for both the weekday evening and Saturday midday peak periods: 2019 Existing, 2021 No-Build, 2021 Build, 2031 No-Build, and 2031 Build Analysis. The analyses show that the study area intersections have sufficient capacity to accommodate the Project-generated traffic volumes without any additional roadway or intersection mitigation. Road conditions are not expected to be hindered by the Project.

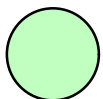
In recognition of the increase in pedestrian traffic in the study area likely to occur as a result in the new residential use on the site, safety improvements to the existing midblock crosswalk on Mill Road in the form of Rectangular Rapid Flash Beacons is a potential option to improve the existing crossing. Additionally, additional pedestrian traffic may be directed to the Main Street at Madbury Road area and relocation of the Main Street crosswalk to the west to shorten the crossing should be reviewed by the Town.

Section 6

Figures and Tables



Legend:



Study Intersection

Mill Road Plaza
Durham, NH

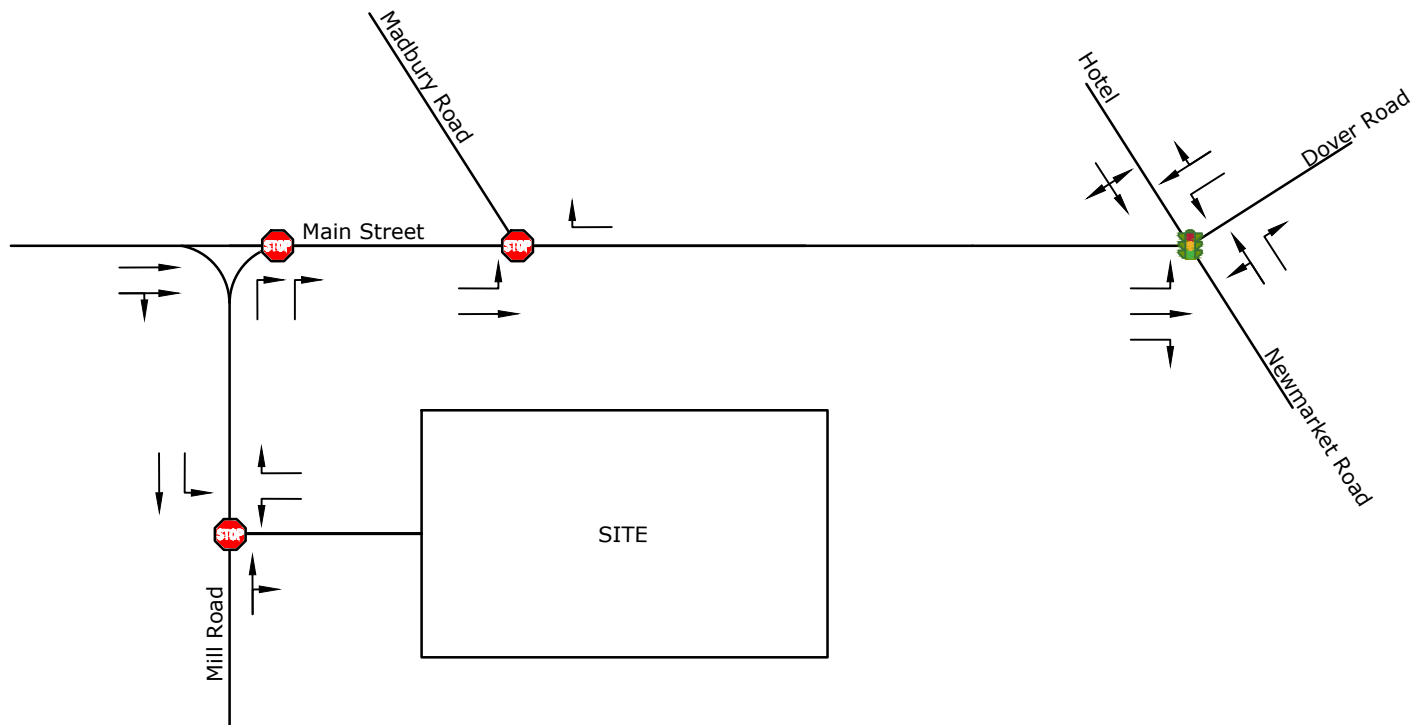
Study Area

DATE: 08/14/2020

SCALE: No Scale

FIGURE 1

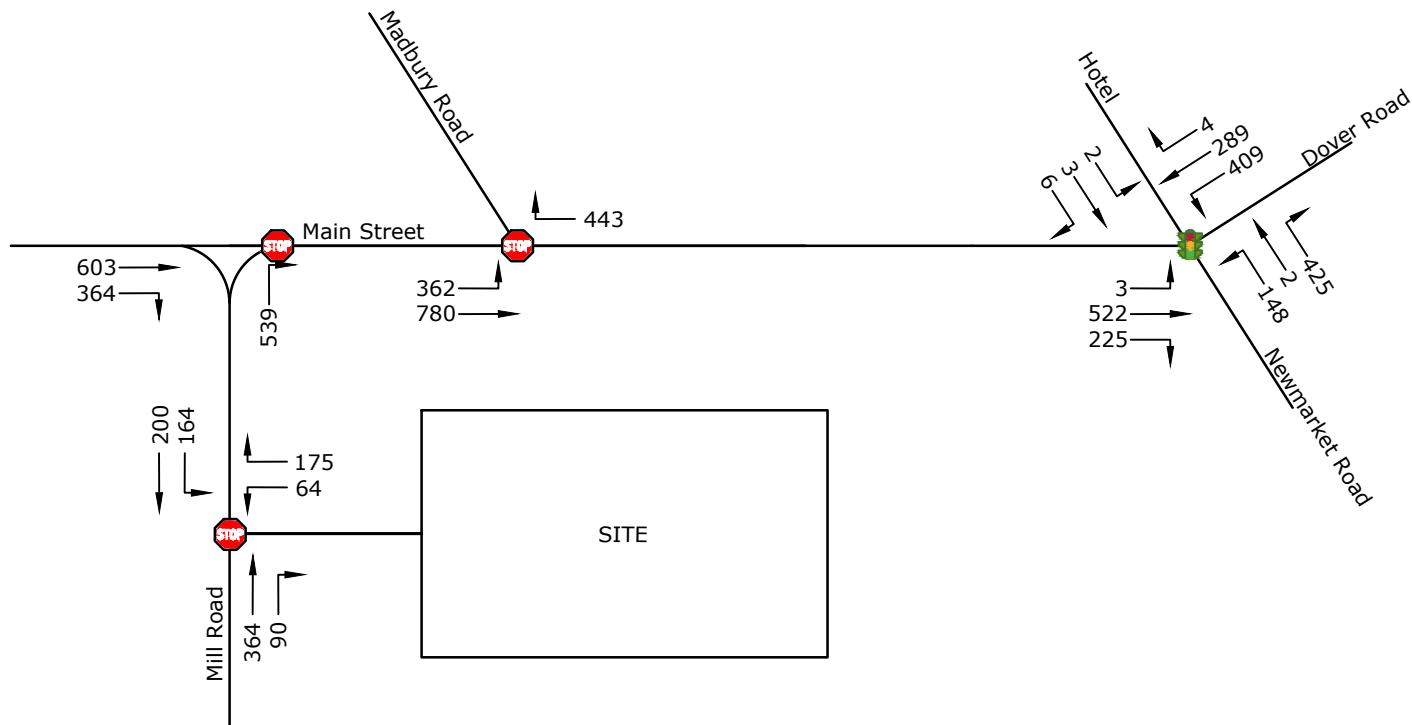
Tighe & Bond
Engineers | Environmental Specialists



Legend

-  Signal
-  Stop Sign

Mill Road Plaza Durham, NH	
Existing Lane Configuration and Traffic Control	
DATE: 08/14/2020	 Engineers Environmental Specialists
SCALE: No Scale	
FIGURE 2	



Legend



Signal



Stop Sign

Mill Road Plaza
Durham, NH

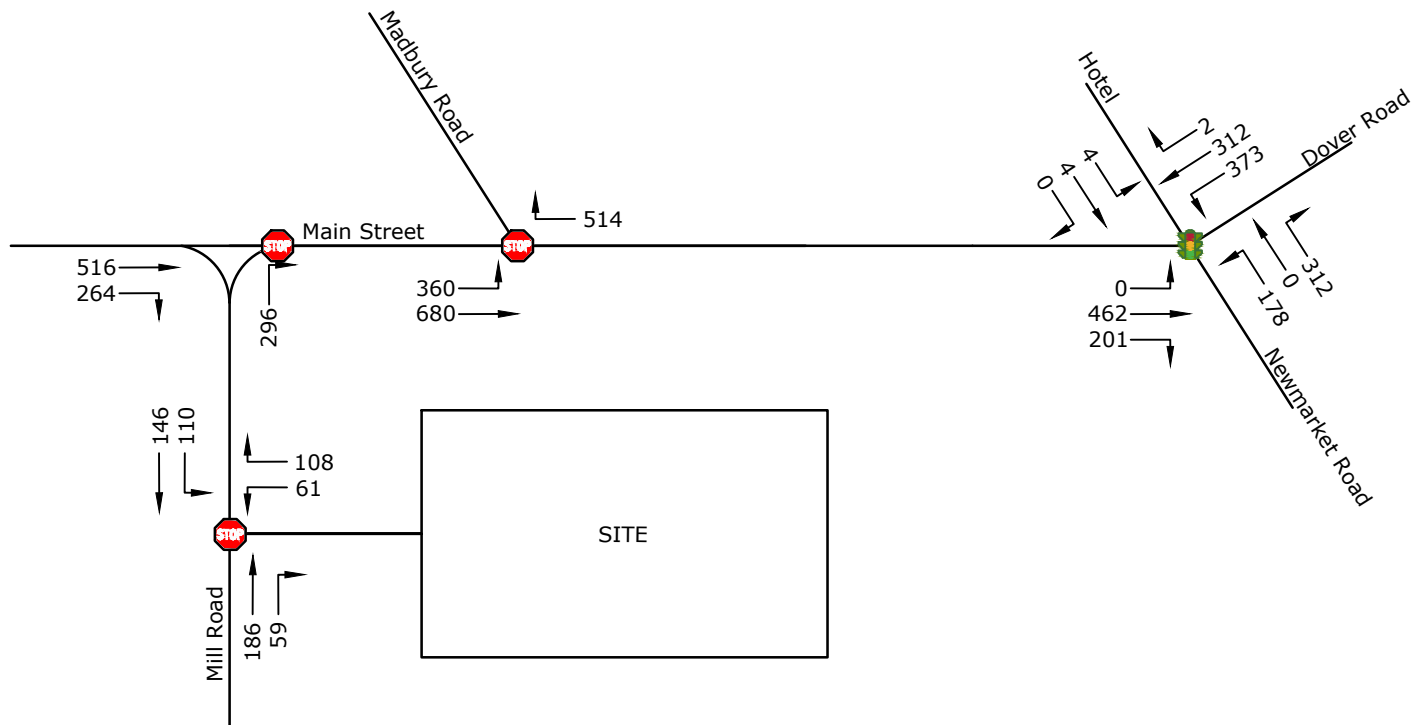
2019 Evening Peak Hour
Balanced Existing Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 3

Tighe&Bond
Engineers | Environmental Specialists



Legend

-  Signal
-  Stop Sign

Mill Road Plaza
Durham, NH

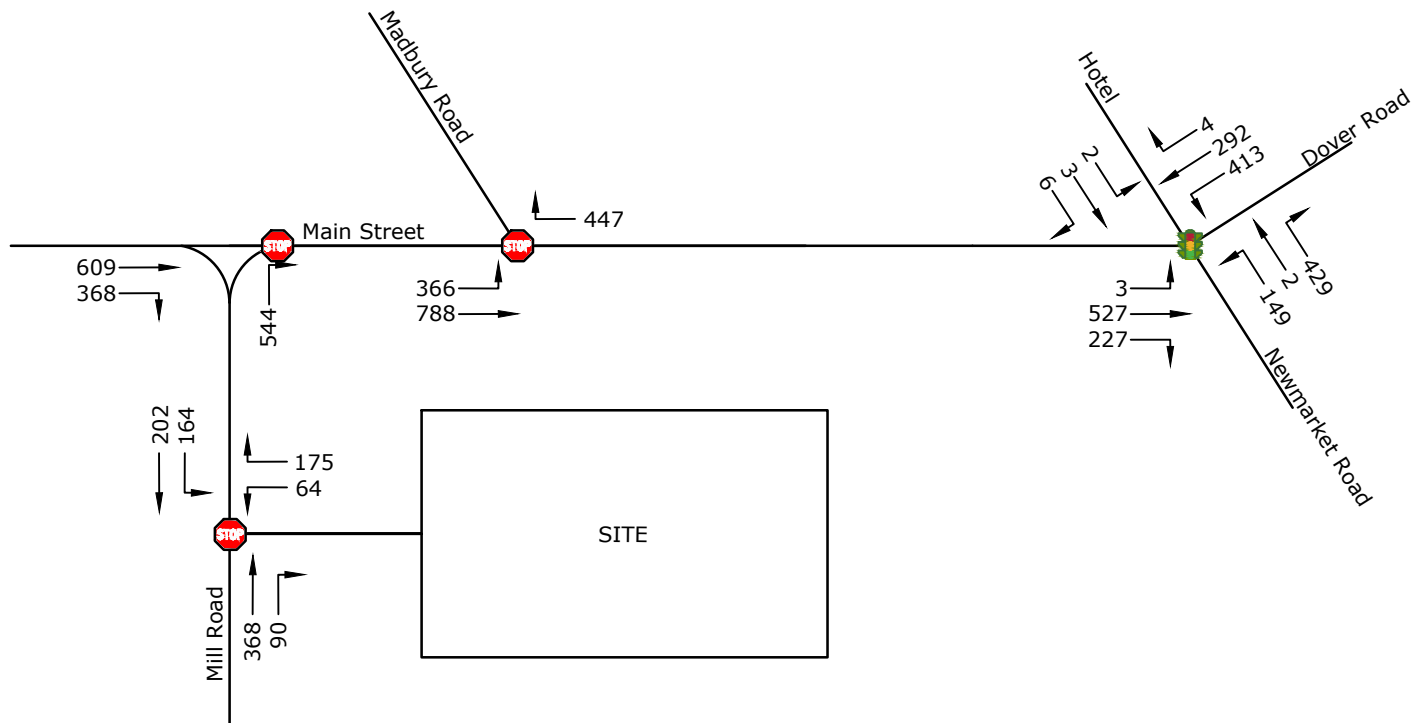
2019 Saturday Midday Peak Hour
Balanced Existing Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 4

Tighe&Bond
Engineers | Environmental Specialists



Legend



Signal



Stop Sign

Mill Road Plaza
Durham, NH

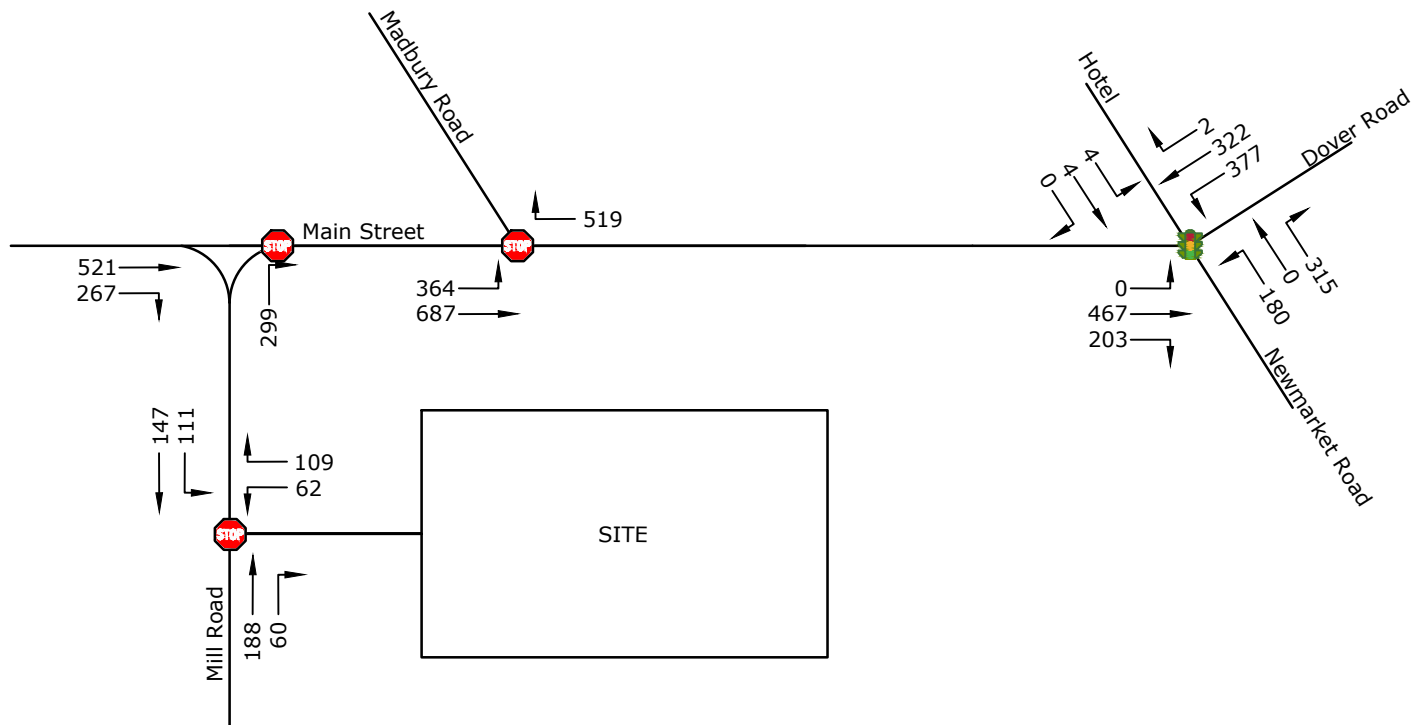
2021 Evening Peak Hour
No Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 5

Tighe&Bond
Engineers | Environmental Specialists



Legend

-  Signal
-  Stop Sign

Mill Road Plaza Durham, NH

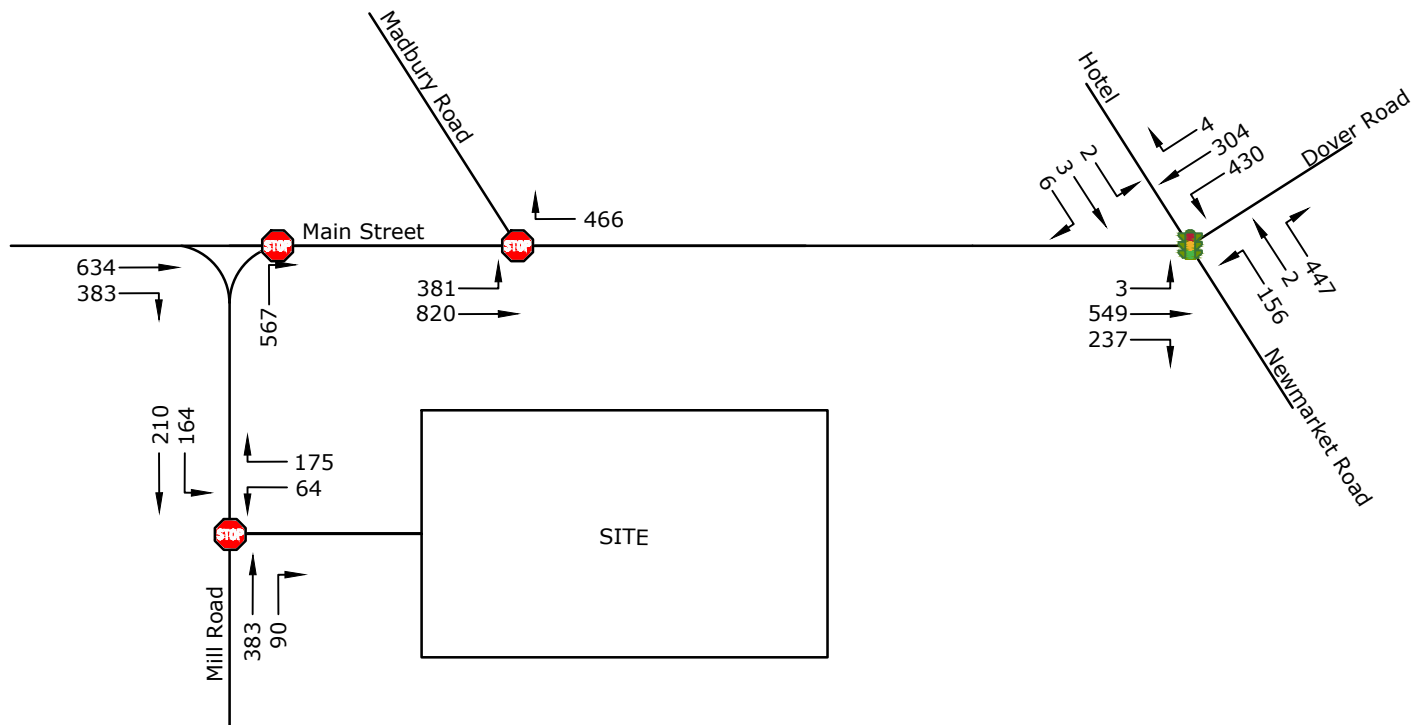
2021 Saturday Midday Peak Hour
No Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 6

Tighe&Bond
Engineers | Environmental Specialists



Legend

-  Signal
-  Stop Sign

Mill Road Plaza
Durham, NH

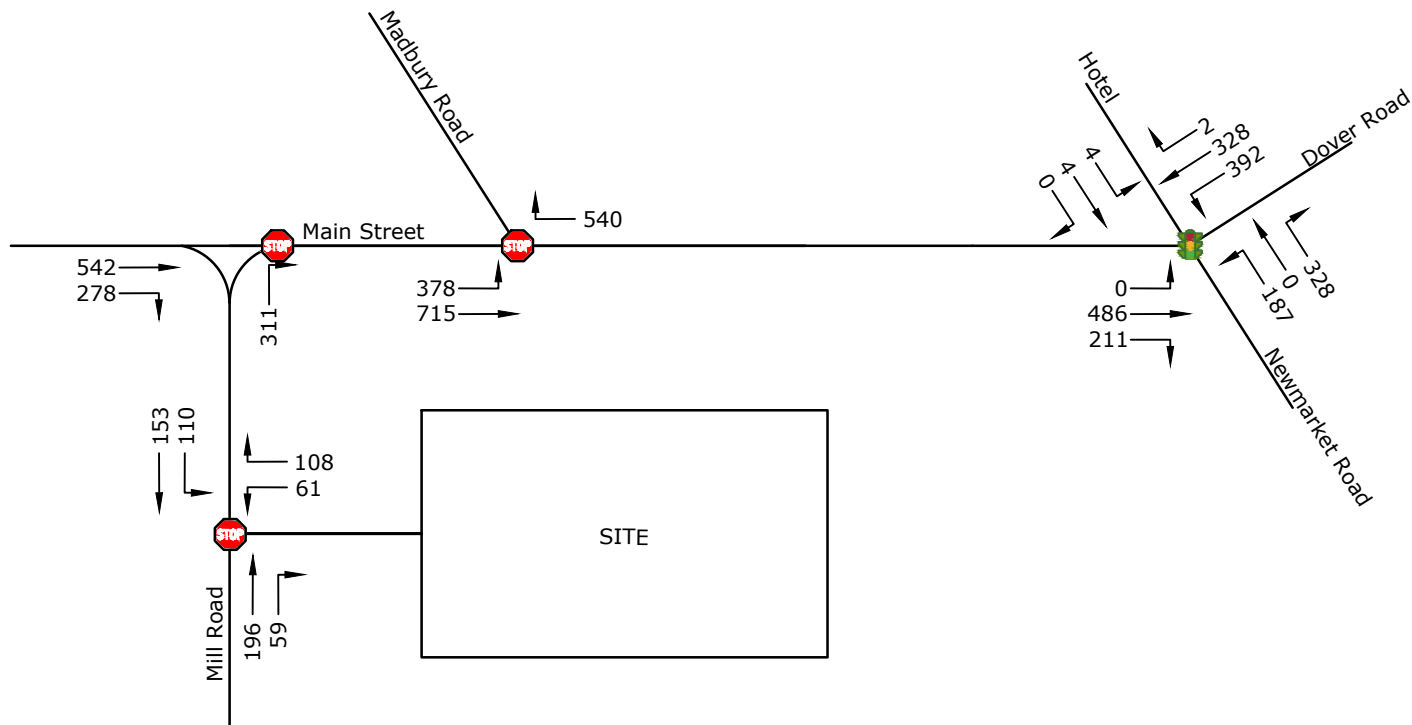
2031 Evening Peak Hour
No-Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 7

Tighe&Bond
Engineers | Environmental Specialists



Legend



Signal



Stop Sign

Mill Road Plaza Durham, NH

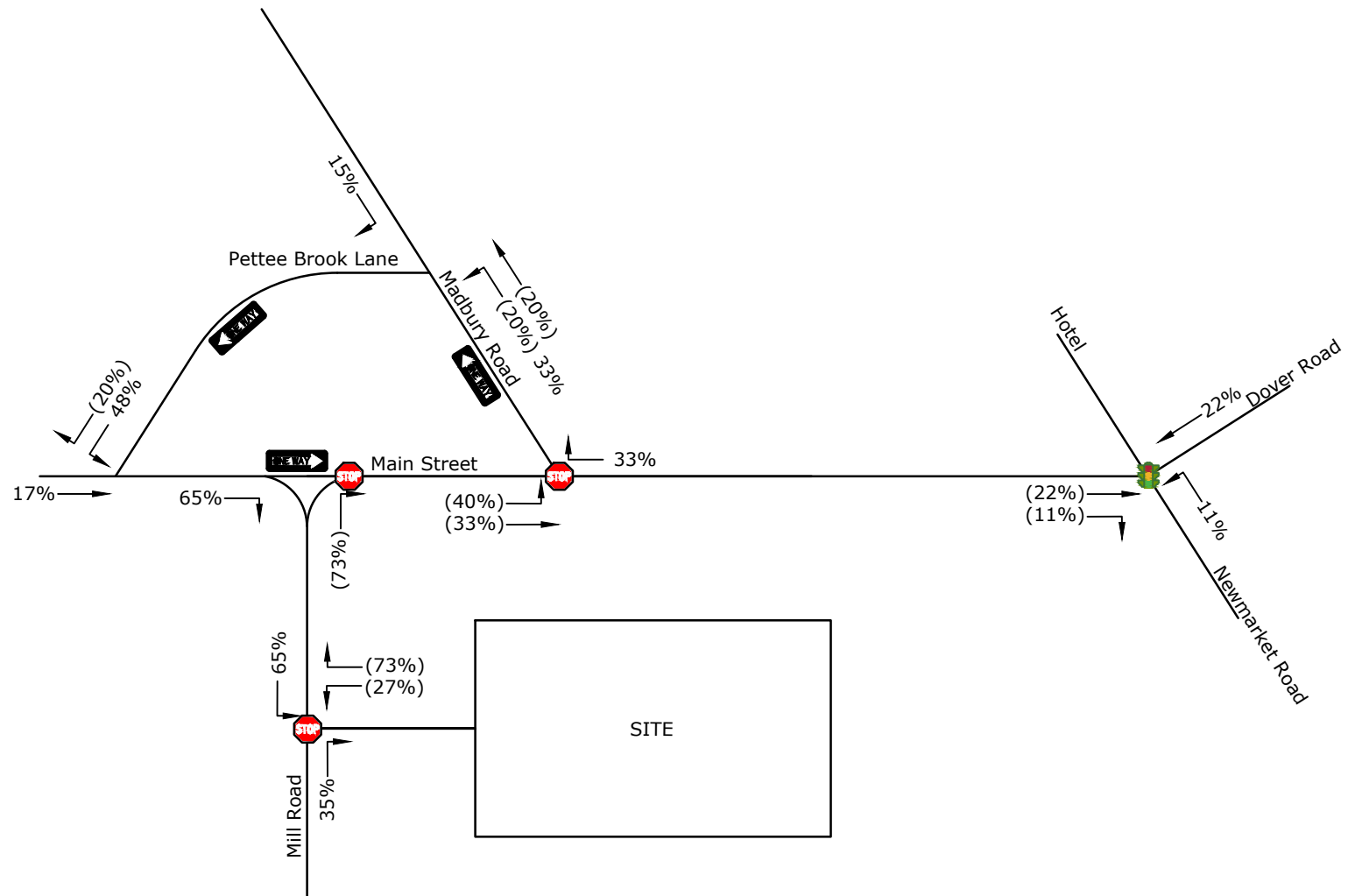
2031 Saturday Midday Peak Hour
No-Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 8

Tighe&Bond
Engineers | Environmental Specialists



Legend:

XX Entering Vehicles
 (XX) Exiting Vehicles



Signal
 Stop Sign

Mill Road Plaza
 Durham, NH

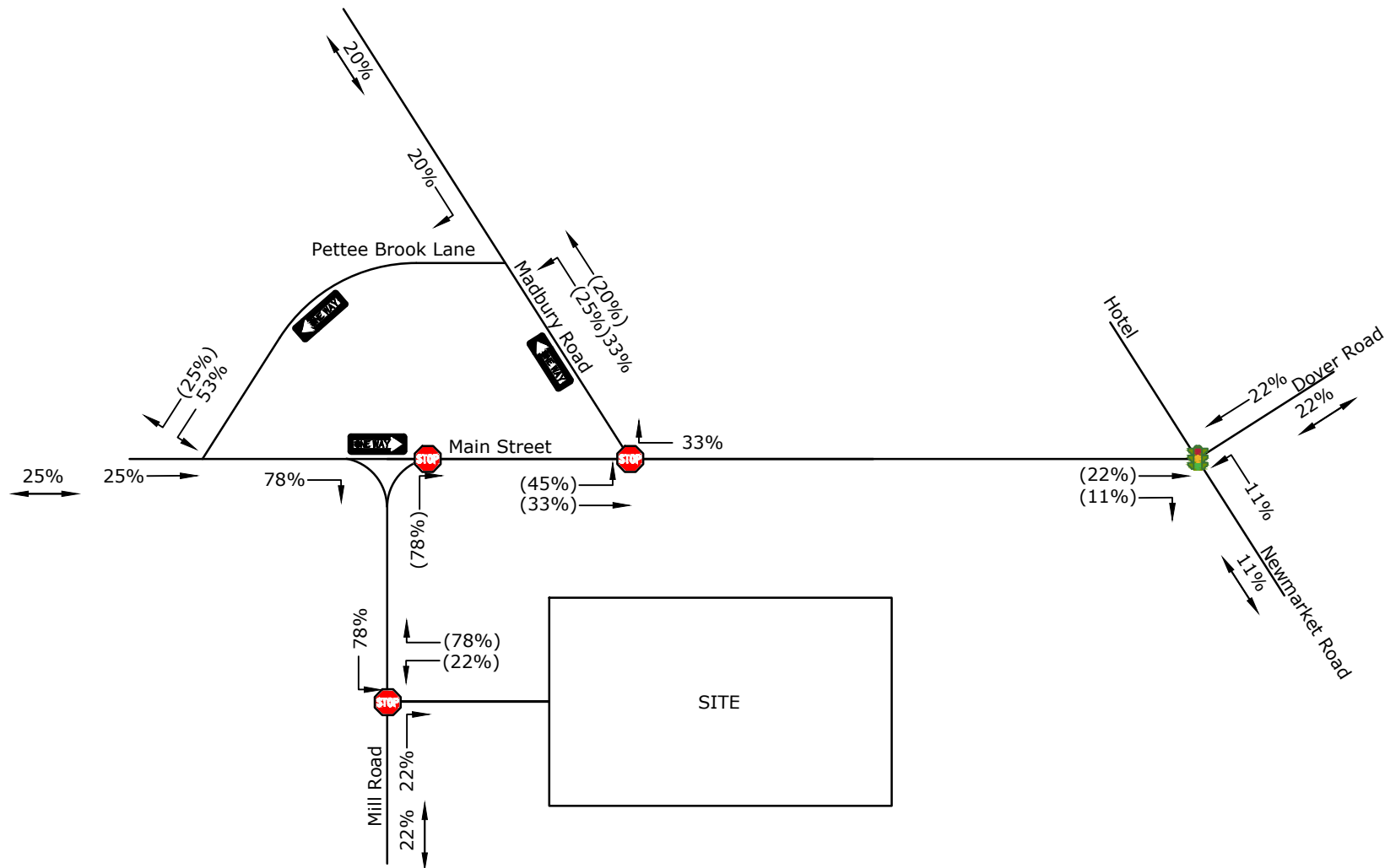
Existing Trip Distribution

DATE: 08/14/2020

SCALE: No Scale

FIGURE 9

Tighe&Bond
 Engineers | Environmental Specialists



Legend:

XX Entering Vehicles
 (XX) Exiting Vehicles



Signal
 Stop Sign

Mill Road Plaza
 Durham, NH

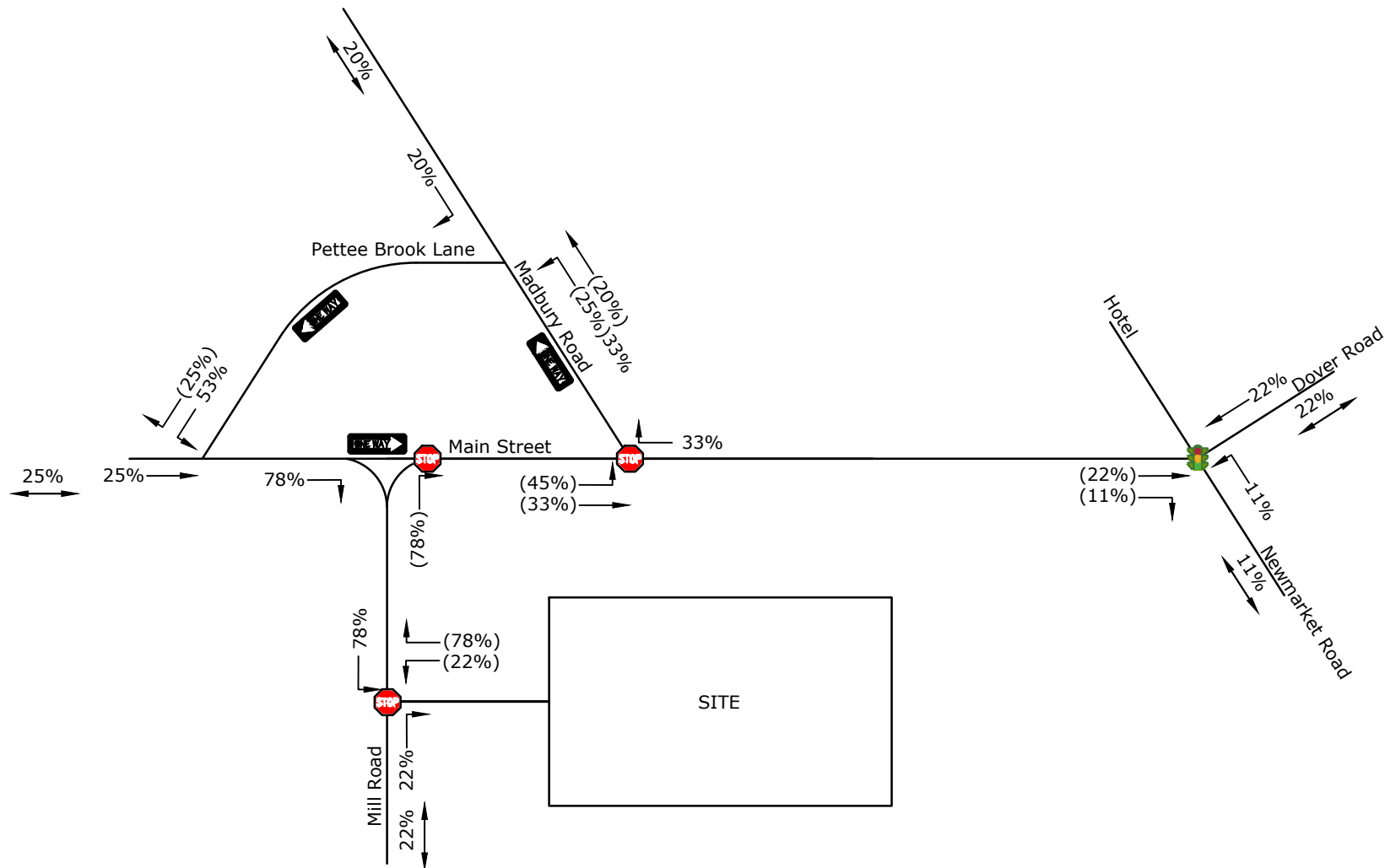
Commercial Trip Distribution

DATE: 08/14/2020

SCALE: No Scale

FIGURE 10

Tighe&Bond
 Engineers | Environmental Specialists



Legend:

XX Entering Vehicles
 (XX) Exiting Vehicles



Signal
 Stop Sign

Mill Road Plaza
 Durham, NH

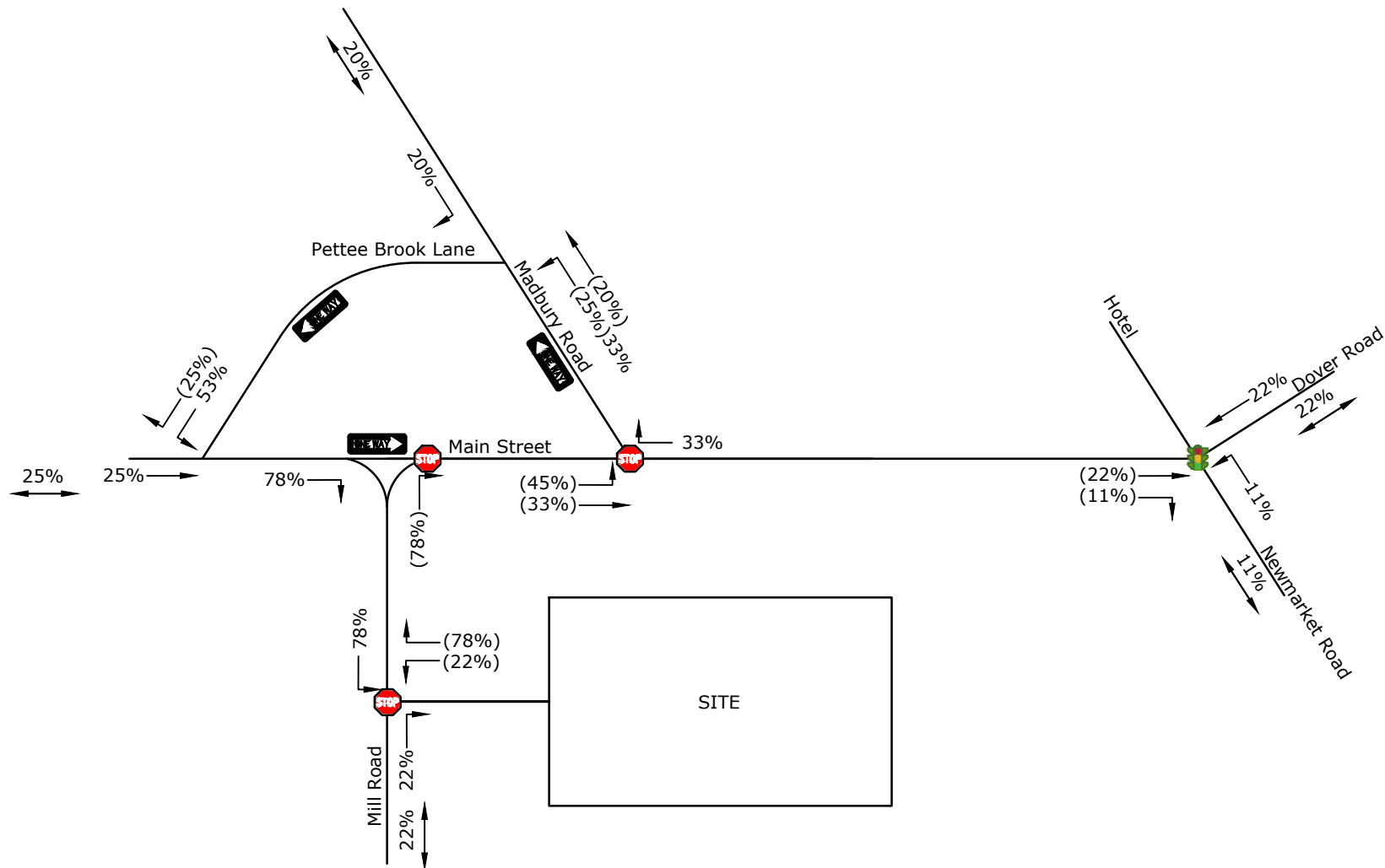
Residential Trip Distribution

DATE: 08/14/2020

SCALE: No Scale

FIGURE 11

Tighe & Bond
 Engineers | Environmental Specialists



Legend:

XX Entering Vehicles
 (XX) Exiting Vehicles



Signal
 Stop Sign

Mill Road Plaza
 Durham, NH

Office Trip Distribution

DATE: 08/14/2020

SCALE: No Scale

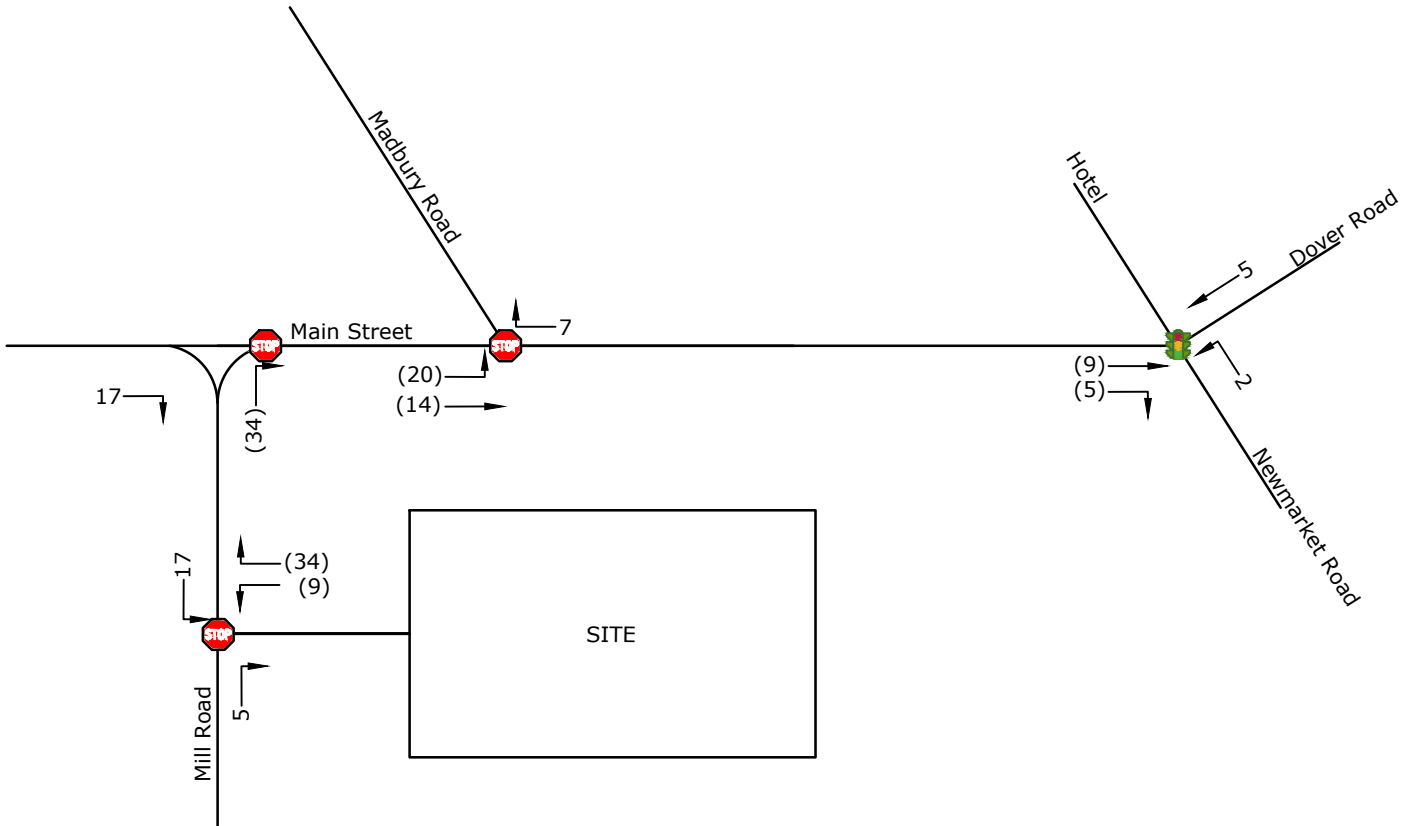
FIGURE 12

Tighe & Bond
 Engineers | Environmental Specialists

Aug 12, 2020-12:34pm Plotted By: MStoutz
Tighe & Bond, Inc. J:\M1529 McCauley Realty Advisors, LLC\002-Mill Road Plaza\Drawings_Figures\AutoCAD\Figures\Traffic Volumes.dwg

Generated Trips:

	Entering	Exiting	Total
Residential	17	18	33
Commercial	3	7	10
Office	<u>2</u>	<u>18</u>	<u>20</u>
Total	22	43	63



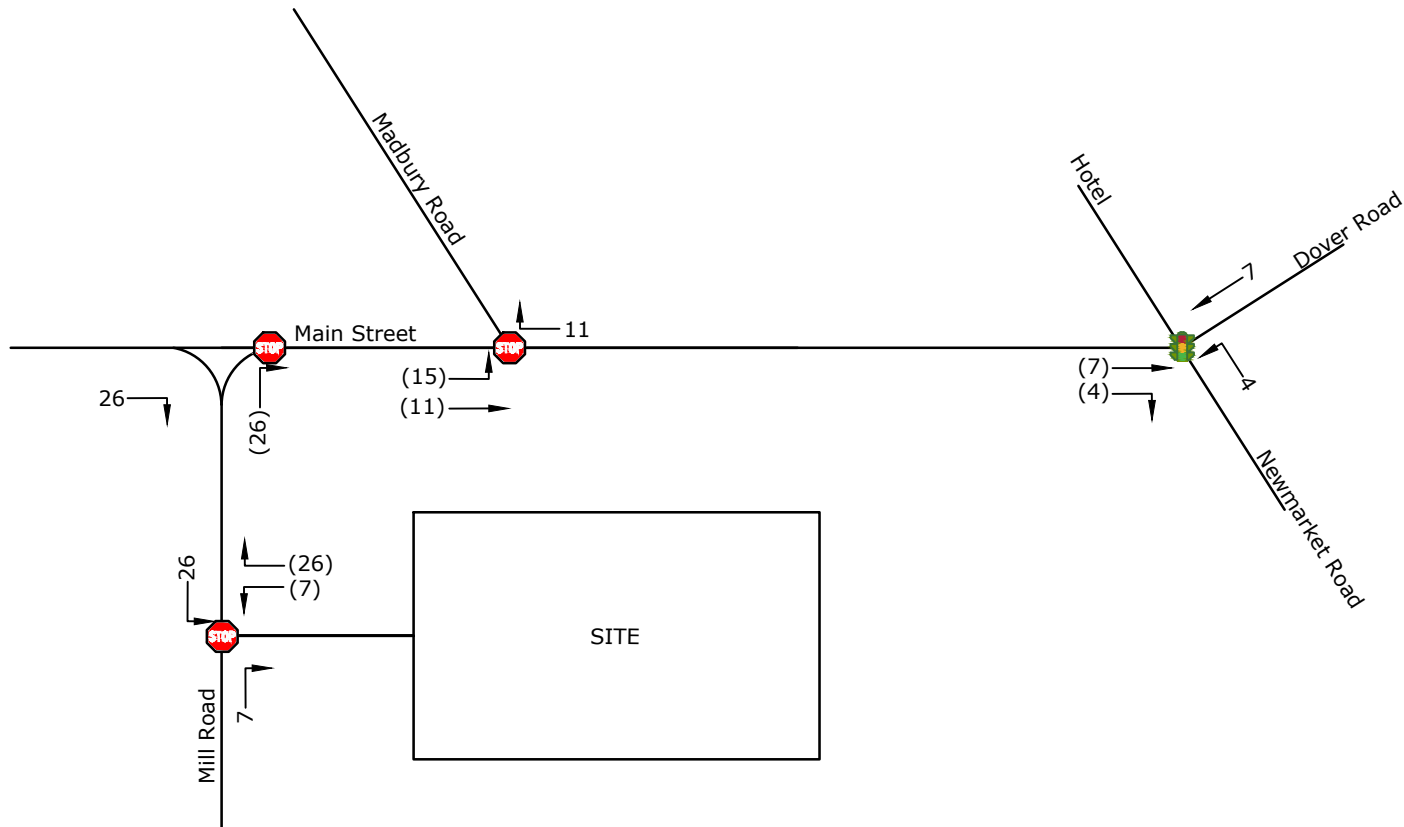
Legend:

- XX Entering Vehicles
- (XX) Exiting Vehicles
-  Signal
-  Stop Sign

Mill Road Plaza Durham, NH	
Site Generated Trips for Evening Peak Hour	
DATE: 08/14/2020	Tighe&Bond Engineers Environmental Specialists
SCALE: No Scale	
FIGURE 13	

Generated Trips:

	Entering	Exiting	Total
Residential	17	18	35
Commercial	13	10	23
Office	<u>3</u>	<u>5</u>	<u>8</u>
Total	33	33	66



Legend:

XX Entering Vehicles
(XX) Exiting Vehicles



Signal
Stop Sign

Mill Road Plaza
Durham, NH

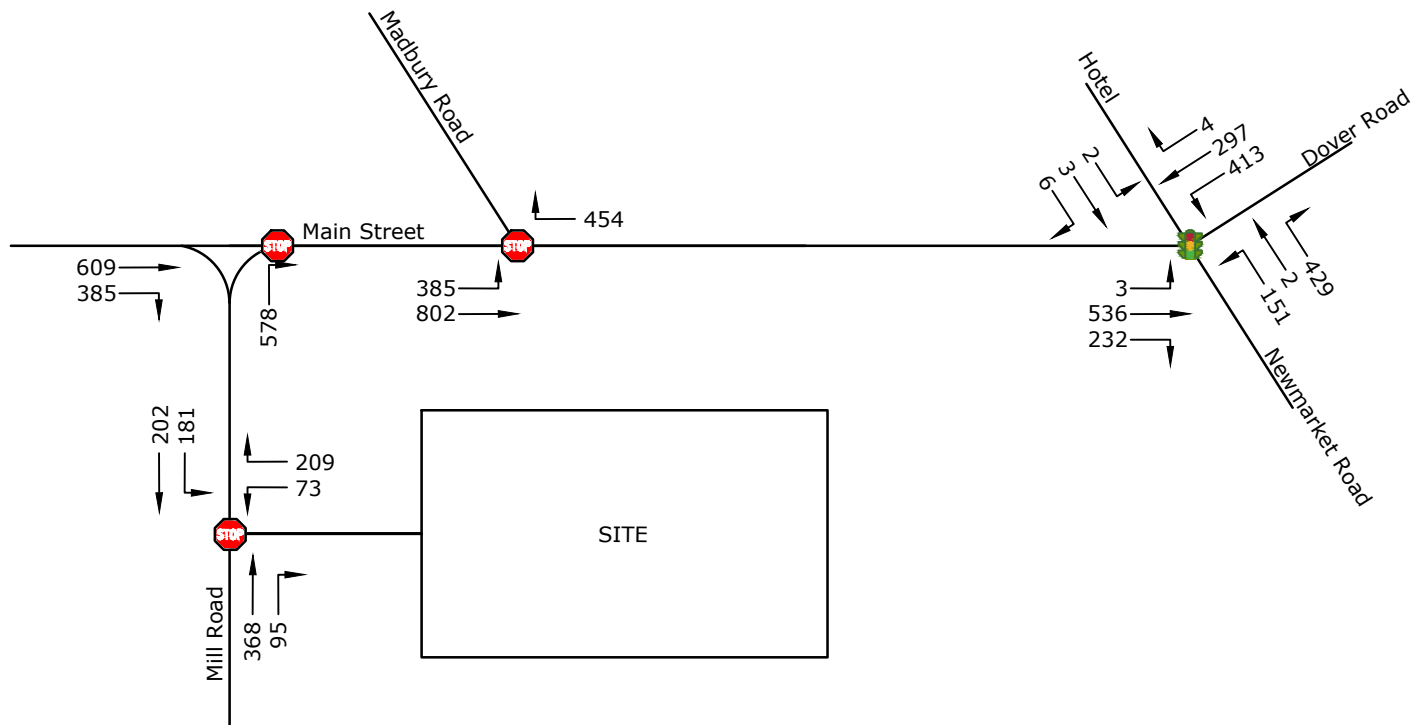
Site Generated Trips for
Saturday Midday Peak Hour

DATE: 08/14/2020

SCALE: No Scale

FIGURE 14

Tighe&Bond
Engineers | Environmental Specialists



Legend



Signal



Stop Sign

Mill Road Plaza
Durham, NH

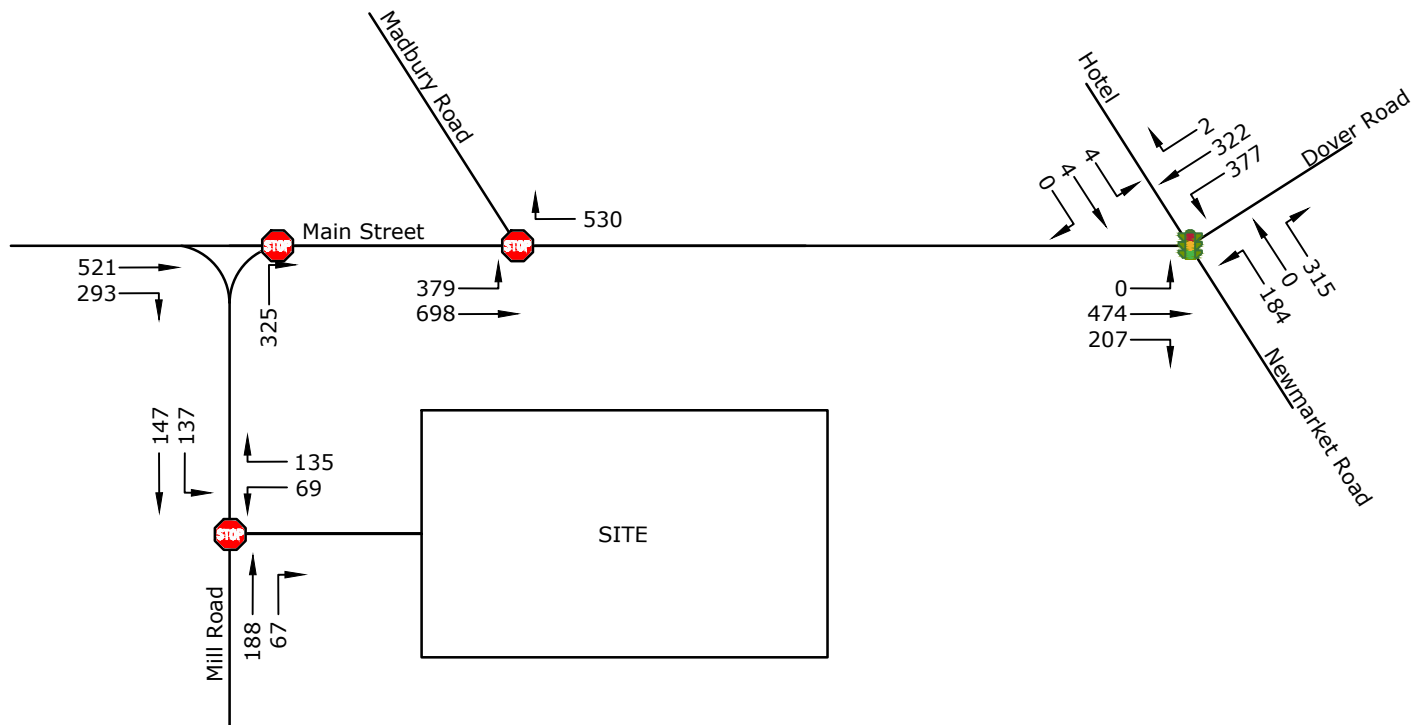
2021 Evening Peak Hour
Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 15

Tighe&Bond
Engineers | Environmental Specialists



Legend



Signal



Stop Sign

Mill Road Plaza Durham, NH

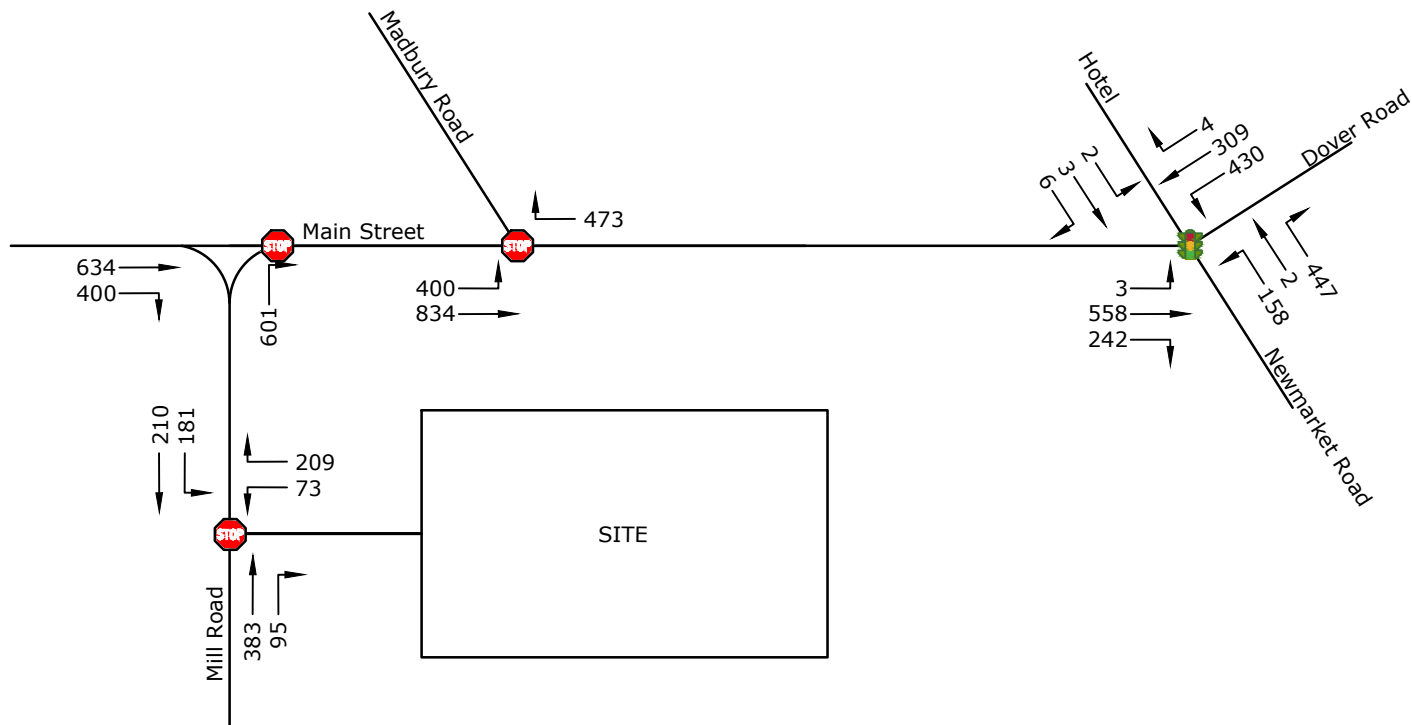
2021 Saturday Midday Peak Hour
Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 16

Tighe&Bond
Engineers | Environmental Specialists



Legend



Signal



Stop Sign

Mill Road Plaza
Durham, NH

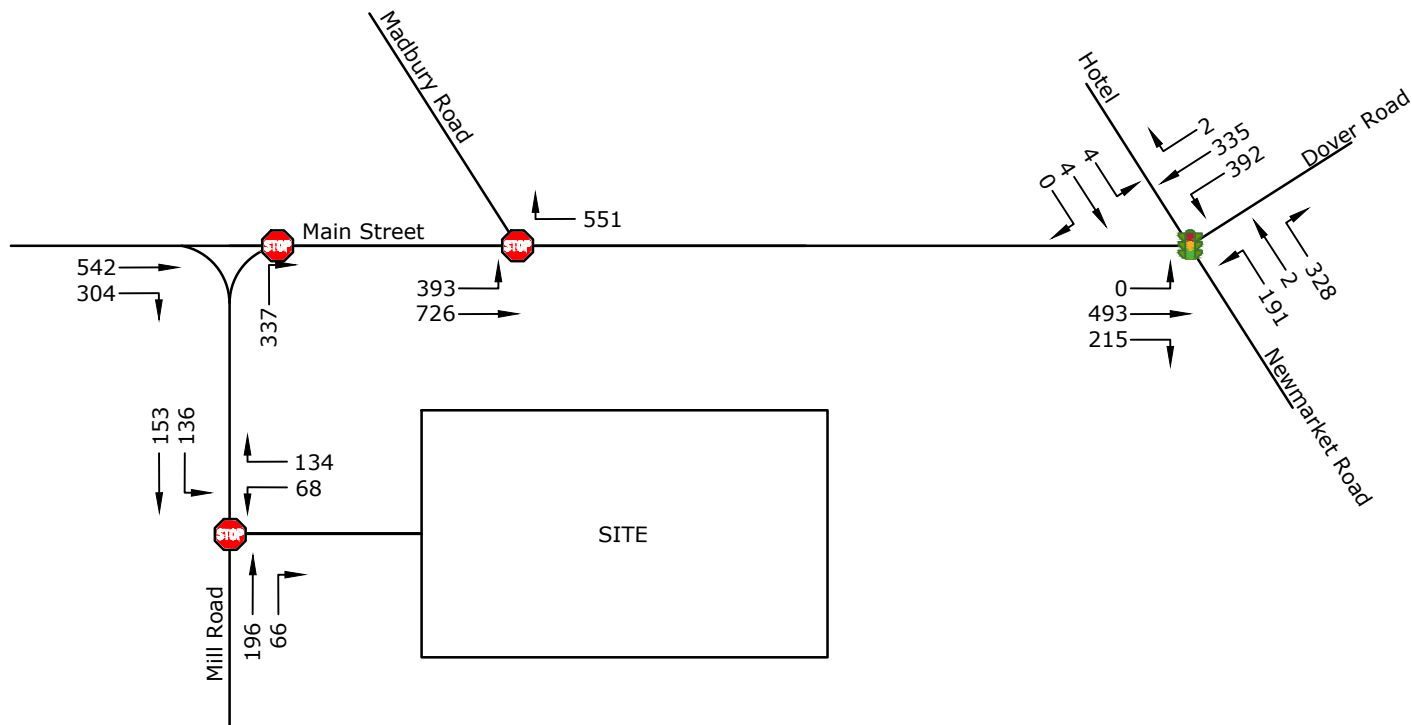
2031 Evening Peak Hour
Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 17

Tighe&Bond
Engineers | Environmental Specialists



Legend

-  Signal
-  Stop Sign

Mill Road Plaza Durham, NH

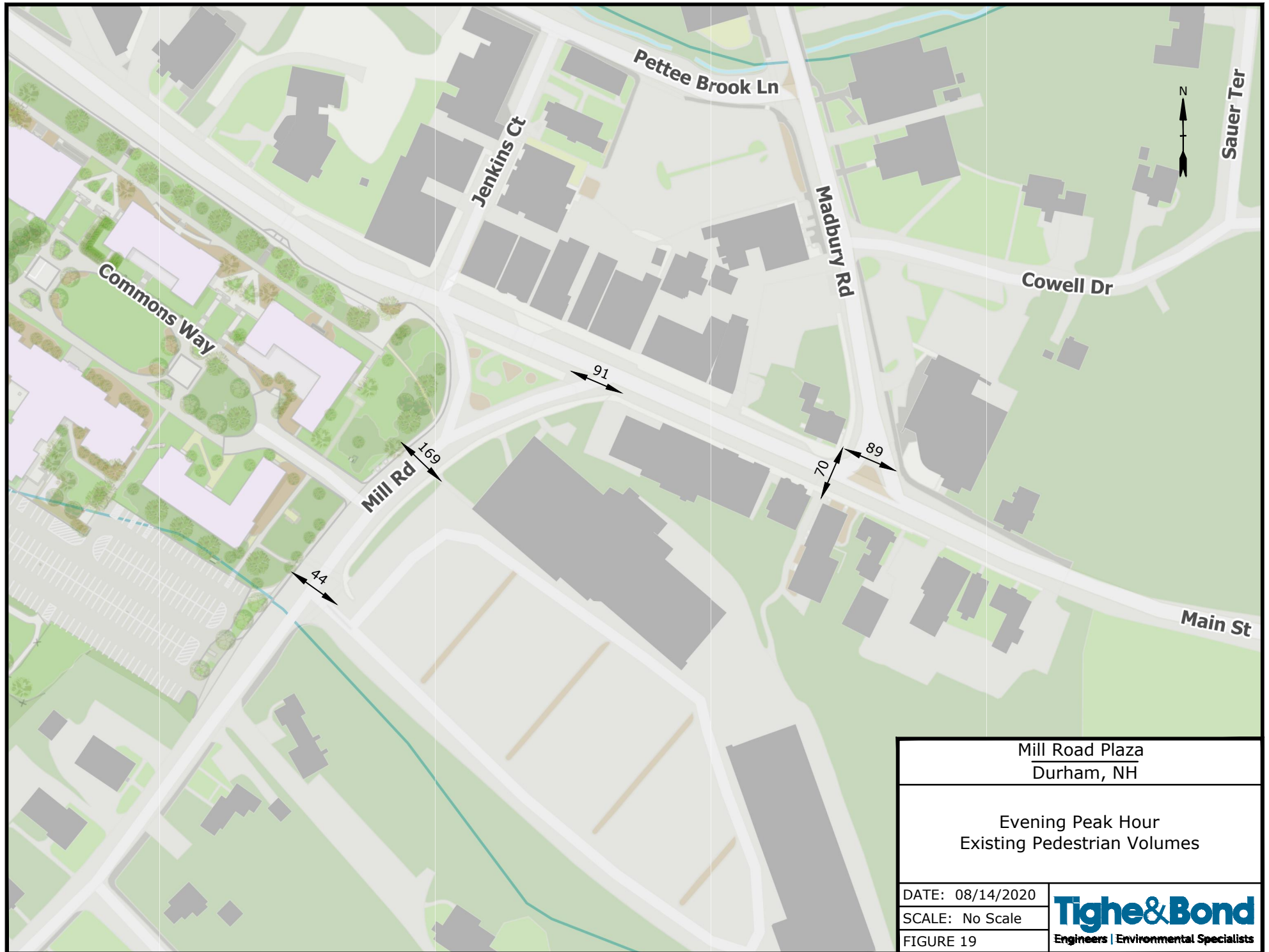
2031 Saturday Midday Peak Hour
Build Traffic Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 18

Tighe&Bond
Engineers | Environmental Specialists



Mill Road Plaza
Durham, NH

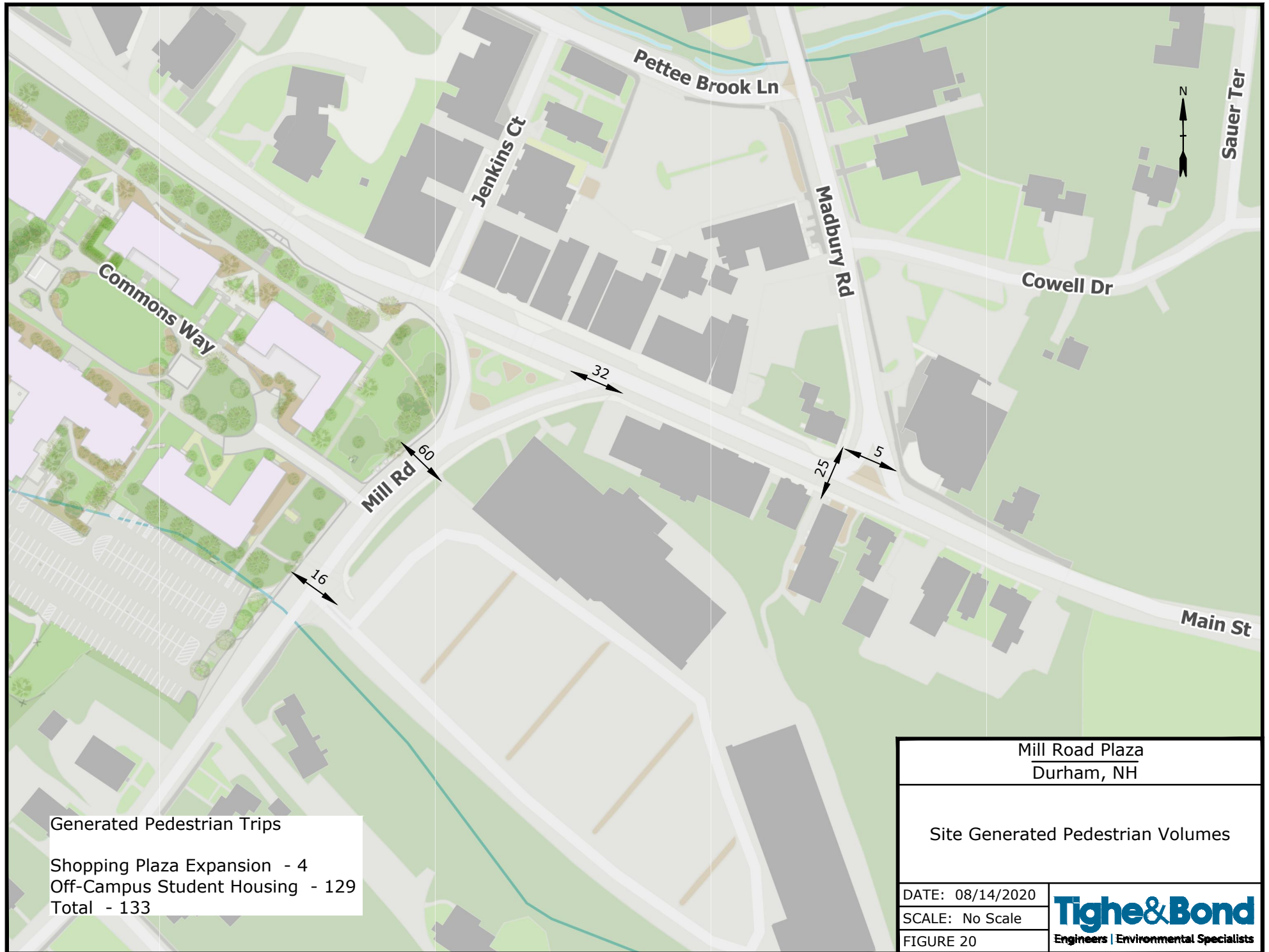
Evening Peak Hour
Existing Pedestrian Volumes

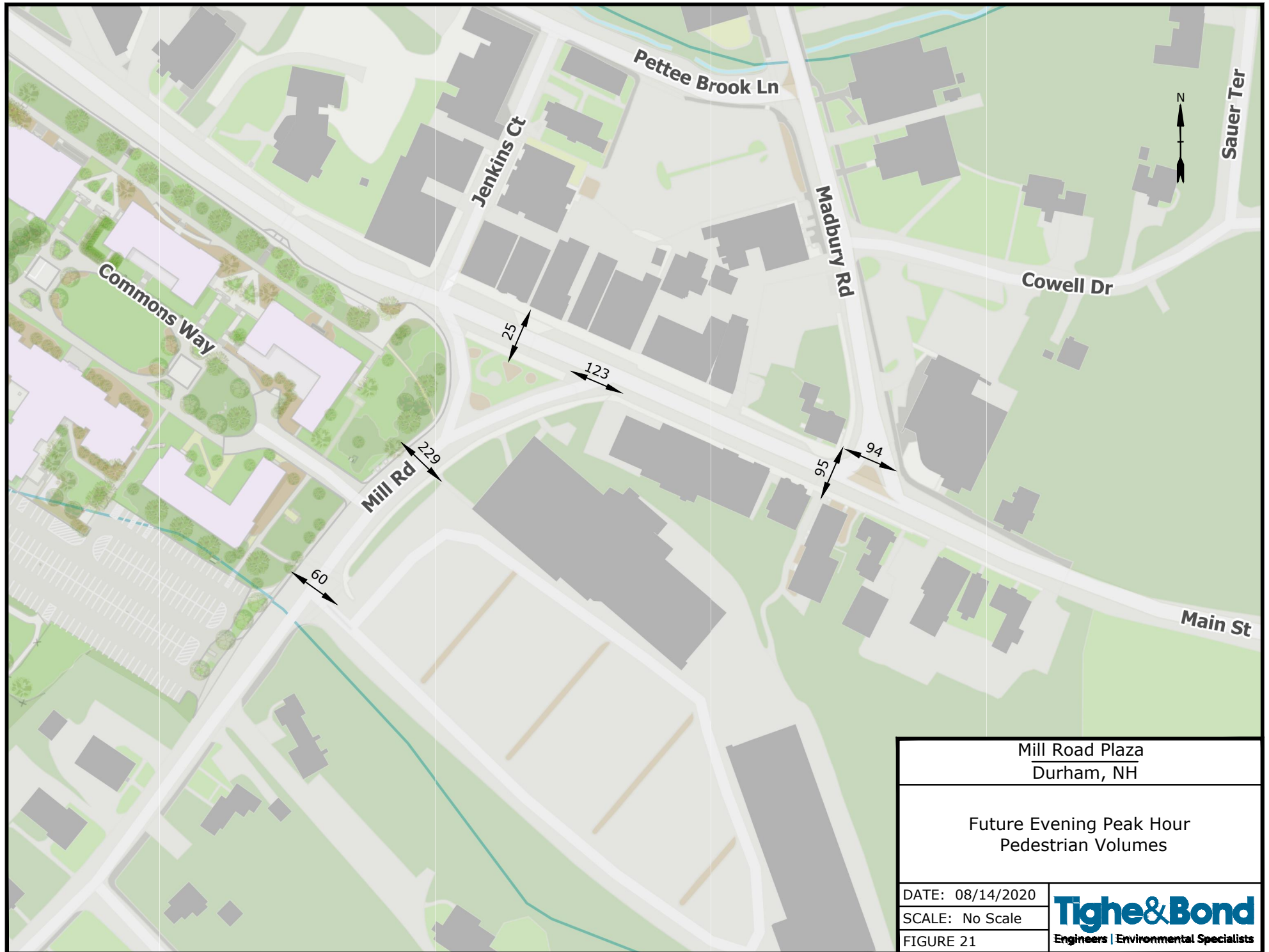
DATE: 08/14/2020

SCALE: No Scale

FIGURE 19

Tighe&Bond
Engineers | Environmental Specialists





Mill Road Plaza
Durham, NH

Future Evening Peak Hour
Pedestrian Volumes

DATE: 08/14/2020

SCALE: No Scale

FIGURE 21

Tighe&Bond
Engineers | Environmental Specialists

Table 6-1

Intersection Operation Summary - Vehicular Levels of Service / Average Delay (sec/veh)

		Weekday Afternoon Peak Hour					Saturday Midday Peak Hour				
	Lane Use	2019 Existing	2021 No Build	2021 Build	2031 No Build	2031 Build	2019 Existing	2021 No Build	2021 Build	2031 No Build	2031 Build
Traffic Signal - Main Street at Newmarket Road/ Hotel Driveway/ Dover Road											
Overall		B / 19.7	C / 20.2	C / 20.6	C / 22.7	C / 23.3	B / 17.8	B / 18.3	B / 18.9	C / 20.1	C / 21.5
Main Street	EBL	A / 8.0	A / 8.0	A / 8.0	A / 8.3	A / 8.3	0 / 0.0	0 / 0.0	0 / 0.0	0 / 0.0	0 / 0.0
	EBT	C / 34.3	D / 35.2	D / 36.2	D / 40.6	D / 42.0	C / 27.4	C / 28.2	C / 29.1	C / 31.2	C / 31.7
	EBR	A / 4.5	A / 4.5	A / 4.5	A / 4.7	A / 4.7	A / 4.3	A / 4.4	A / 4.4	A / 4.5	A / 4.5
Dover Road	WBL	B / 15.0	B / 15.7	B / 16.5	B / 19.7	C / 20.8	B / 11.6	B / 12.0	B / 12.6	B / 14.3	B / 15.1
	WBTR	A / 6.7	A / 6.7	A / 6.8	A / 6.8	A / 6.8	A / 5.4	A / 5.5	A / 5.5	A / 5.5	A / 5.5
Newmarket Road	NBLT	E / 55.3	E / 55.9	E / 56.6	E / 59.4	E / 60.1	E / 57.8	E / 59.0	E / 61.3	E / 64.2	E / 75.1
Hotel Driveway	NBR	B / 10.8	B / 10.9	B / 10.8	B / 11.0	B / 11.0	A / 9.4	A / 9.4	A / 9.4	A / 9.6	A / 9.6
	SB	C / 25.7	C / 25.7	C / 25.7	C / 26.0	C / 26.0	C / 31.9	C / 32.2	C / 32.4	C / 33.1	C / 33.1
Unsignalized TWSC - Mill Road at Site Driveway											
Overall		-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --
Site Driveway	WBL	D / 28.1	D / 28.4	D / 32.0	D / 29.9	D / 33.7	C / 15.4	C / 15.5	C / 17.1	C / 15.7	C / 17.3
	WBR	C / 15.0	C / 15.1	C / 16.8	C / 15.5	C / 17.4	B / 10.9	B / 11.0	B / 11.5	B / 11.0	B / 11.6
Mill Road	SBL	A / 9.0	A / 9.1	A / 9.2	A / 9.1	A / 9.2	A / 8.1	A / 8.1	A / 8.2	A / 8.1	A / 8.2
Unsignalized - Main Street at Mill Road											
Overall		-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --
Mill Road	NBR	C / 20.9	C / 21.2	D / 25.5	C / 23.0	D / 28.3	B / 14.0	B / 14.1	C / 15.4	B / 14.5	C / 15.9
Unsignalized - Main Street at Madbury Road											
Overall		-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --	-- / --
Main Street	WBR	B / 11.8	B / 13.2	C / 15.1	C / 15.4	C / 17.2	C / 19.0	C / 20.9	D / 25.1	D / 32.3	D / 31.4

Table 6-2Intersection Operation Summary - Vehicular 50th / 95th Percentile Queue (In Feet)

		Weekday Afternoon Peak Hour						Saturday Midday Peak Hour				
	Lane Use	Available Storage	2019 Existing	2021 No Build	2021 Build	2031 No Build	2031 Build	2019 Existing	2021 No Build	2021 Build	2031 No Build	2031 Build
Traffic Signal - Main Street at Newmarket Road/ Hotel Driveway/ Dover Road												
Main Street	EBL	100	1 / 3	1 / 3	1 / 3	1 / 3	1 / 3	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0
	EBT	>1000	232 / 490	236 / 498	242 / 510	256 / 542	262 / 554	184 / 390	188 / 403	195 / 415	206 / 443	210 / 453
	EBR	460	0 / 50	0 / 50	0 / 51	0 / 53	0 / 52	0 / 46	0 / 47	0 / 48	0 / 49	0 / 49
Dover Road	WBL	230	81 / 175	86 / 181	90 / 186	112 / 211	117 / 216	66 / 125	67 / 132	67 / 138	71 / 159	76 / 164
	WBTR	>1000	48 / 123	49 / 124	50 / 126	51 / 130	52 / 131	52 / 83	54 / 86	54 / 85	56 / 87	57 / 89
Newmarket Road	NBLT	>1000	70 / 186	70 / 187	71 / 190	75 / 198	76 / 202	84 / 208	86 / 214	88 / 222	90 / 232	94 / 245
Hotel Driveway	NBR	150	0 / 90	0 / 90	0 / 90	0 / 94	0 / 94	0 / 71	0 / 72	0 / 73	0 / 75	0 / 75
	SB	75	2 / 17	2 / 17	2 / 17	2 / 17	2 / 17	4 / 16	4 / 16	4 / 16	4 / 16	4 / 16
Unsignalized TWSC - Mill Road at Site Driveway												
Site Driveway	WBL	125	33	33	40	35	43	15	15	18	15	18
	WBR	125	38	40	53	40	55	15	15	20	15	20
Mill Road	SBL	65	15	15	15	15	15	8	8	10	8	10
Unsignalized TWSC - Main Street at Mill Road												
Mill Road	NBR	375	91	93	119	89	136	31	31	38	34	41
Unsignalized - Main Street at Madbury Road												
Main Street	WBR	>1000	197	243	245	262	302	334	360	412	417	457

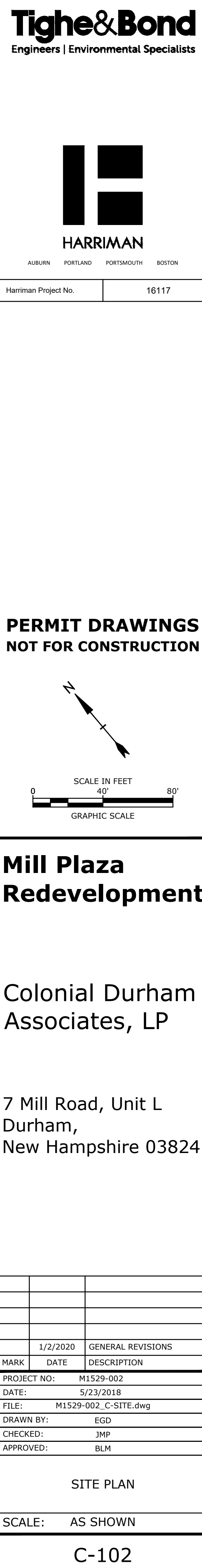
PARKING NOTES:

(1) EXISTING SITE CONTAINS 345 PARKING SPACES. THE PROPOSED 581 SPACES CONSIST OF 497 SURFACE PARKING (90 COMPACT, 407 STANDARD) PLUS 84 GARAGE SPACES. 157 OF THE SURFACE PARKING SPACES ARE PROPOSED TO BE LEASED FROM THE ADJACENT PARCELS (TAX MAP 5, LOTS 1-15 AND 1-16) UNDER A SEPARATE SITE PLAN APPLICATION.

(2) PER AGREEMENT WITH TOWN OF DURHAM, DATED DECEMBER 14, 2015, PARKING ON SITE SHALL BE INCREASED BEYOND THE 345 SPACES THAT CURRENTLY EXIST.

(3) SECTION 175-112.A., OF THE DURHAM ZONING ORDINANCE ALLOWS AN EXEMPTION FROM THE PARKING REQUIREMENTS IN THE CENTRAL BUSINESS DISTRICT. THIS PLAN REQUESTS AN EXEMPTION OF 15 PARKING SPACES.

Year	Men (%)	Women (%)
1980	10	12
1985	11	13
1990	12	14
1995	14	16
2000	16	18



PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	67	0	0	0	67	0	117	0	0	117	184
11:45 AM	0	0	0	0	0	0	0	0	0	0	73	0	0	0	73	0	107	0	0	107	180
Total	0	0	0	0	0	0	0	0	0	0	140	0	0	0	140	0	224	0	0	224	364
12:00 PM	0	0	1	0	1	0	0	0	0	0	56	0	0	0	56	0	117	0	0	117	174
12:15 PM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	66	0	128	1	0	129	195
12:30 PM	0	0	0	0	0	0	0	0	0	0	87	0	0	0	87	0	143	0	0	143	230
12:45 PM	0	0	0	0	0	0	0	0	0	0	60	0	0	0	60	0	105	0	0	105	165
Total	0	0	1	0	1	0	0	0	0	0	269	0	0	0	269	0	493	1	0	494	764
1:00 PM	0	0	0	0	0	0	0	0	0	0	53	1	0	0	54	0	119	0	0	119	173
1:15 PM	0	0	0	0	0	0	0	0	0	0	58	0	0	0	58	0	112	0	0	112	170
Total	0	0	0	0	0	0	0	0	0	0	111	1	0	0	112	0	231	0	0	231	343
Grand Total	0	0	1	0	1	0	0	0	0	0	520	1	0	0	521	0	948	1	0	949	1471
Approach %	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0		99.8	0.2	0.0	0.0		0.0	99.9	0.1	0.0		
Total %	0.0	0.0	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	35.4	0.1	0.0	0.0	35.4	0.0	64.4	0.1	0.0	64.5	
Exiting Leg Total	2					1469					0					0					1471
Cars	0	0	1	0	1	0	0	0	0	0	513	1	0	0	514	0	935	1	0	936	1451
% Cars	0.0	0.0	100.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	98.7	100.0	0.0	0.0	98.7	0.0	98.6	100.0	0.0	98.6	98.6
Exiting Leg Total	2					1449					0					0					1451
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	0	13	0	0	13	20
% Heavy Vehicles	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.3	0.0	0.0	0.0	1.3	0.0	1.4	0.0	0.0	1.4	1.4
Exiting Leg Total	0					20					0					0					20

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:45 AM	0	0	0	0	0	0	0	0	0	0	73	0	0	0	73	0	107	0	0	107	180
12:00 PM	0	0	1	0	1	0	0	0	0	0	56	0	0	0	56	0	117	0	0	117	174
12:15 PM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	66	0	128	1	0	129	195
12:30 PM	0	0	0	0	0	0	0	0	0	0	87	0	0	0	87	0	143	0	0	143	230
Total Volume	0	0	1	0	1	0	0	0	0	0	282	0	0	0	282	0	495	1	0	496	779
% Approach Total	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	99.8	0.2	0.0		
PHF	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.810	0.000	0.000	0.000	0.810	0.000	0.865	0.250	0.000	0.867	0.847
Cars	0	0	1	0	1	0	0	0	0	0	278	0	0	0	278	0	485	1	0	486	765
Cars %	0.0	0.0	100.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	98.6	0.0	0.0	0.0	98.6	0.0	98.0	100.0	0.0	98.0	98.2
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	10	0	0	10	14
Heavy Vehicles %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0	0.0	1.4	0.0	2.0	0.0	0.0	2.0	1.8
Cars Enter Leg	0	0	1	0	1	0	0	0	0	0	278	0	0	0	278	0	485	1	0	486	765
Heavy Enter Leg	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	10	0	0	10	14
Total Entering Leg	0	0	1	0	1	0	0	0	0	0	282	0	0	0	282	0	495	1	0	496	779
Cars Exiting Leg	1					764					0					0					765
Heavy Exiting Leg	0					14					0					0					14
Total Exiting Leg	1					778					0					0					779

PDI File #: **186243 A**
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 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars-Combined (Motorcycles, Cars, Light Goods)

	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
11:30 AM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	0	66	0	117	0	0	117	183
11:45 AM	0	0	0	0	0	0	0	0	0	0	72	0	0	0	0	72	0	104	0	0	104	176
Total	0	0	0	0	0	0	0	0	0	0	138	0	0	0	0	138	0	221	0	0	221	359
12:00 PM	0	0	1	0	1	0	0	0	0	0	53	0	0	0	0	53	0	114	0	0	114	168
12:15 PM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	0	66	0	126	1	0	127	193
12:30 PM	0	0	0	0	0	0	0	0	0	0	87	0	0	0	0	87	0	141	0	0	141	228
12:45 PM	0	0	0	0	0	0	0	0	0	0	59	0	0	0	0	59	0	104	0	0	104	163
Total	0	0	1	0	1	0	0	0	0	0	265	0	0	0	0	265	0	485	1	0	486	752
1:00 PM	0	0	0	0	0	0	0	0	0	0	52	1	0	0	0	53	0	117	0	0	117	170
1:15 PM	0	0	0	0	0	0	0	0	0	0	58	0	0	0	0	58	0	112	0	0	112	170
Total	0	0	0	0	0	0	0	0	0	0	110	1	0	0	0	111	0	229	0	0	229	340
Grand Total	0	0	1	0	1	0	0	0	0	0	513	1	0	0	0	514	0	935	1	0	936	1451
Approach %	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0		99.8	0.2	0.0	0.0			0.0	99.9	0.1	0.0		
Total %	0.0	0.0	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	35.4	0.1	0.0	0.0	35.4		0.0	64.4	0.1	0.0	64.5	
Exiting Leg Total	2					1449					0					0					1451	

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Driveway					Main Street					Mill Road					Main Street					
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Total
11:45 AM	0	0	0	0	0	0	0	0	0	0	72	0	0	0	72	0	104	0	0	104	176
12:00 PM	0	0	1	0	1	0	0	0	0	0	53	0	0	0	53	0	114	0	0	114	168
12:15 PM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	66	0	126	1	0	127	193
12:30 PM	0	0	0	0	0	0	0	0	0	0	87	0	0	0	87	0	141	0	0	141	228
Total Volume	0	0	1	0	1	0	0	0	0	0	278	0	0	0	278	0	485	1	0	486	765
% Approach Total	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	99.8	0.2	0.0		
PHF	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.799	0.000	0.000	0.000	0.799	0.000	0.860	0.250	0.000	0.862	0.839
Entering Leg	0	0	1	0	1	0	0	0	0	0	278	0	0	0	278	0	485	1	0	486	765
Exiting Leg	1					764					0					0					765
Total	2					764					278					486					1530

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)

	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
11:30 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	3	0	0	3	4
Total	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	3	0	0	3	5
12:00 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	3	0	3	0	0	3	6
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
12:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	1	0	0	1	2
Total	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	4	0	8	0	0	8	12
1:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2	0	0	2	3
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2	0	0	2	3
Grand Total	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	7	0	13	0	0	13	20
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0			0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	35.0	0.0	0.0	0.0	35.0		0.0	65.0	0.0	0.0	65.0	
Exiting Leg Total	0					20					0					0					20	
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	5
% Buses	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	38.5	0.0	0.0	38.5	25.0
Exiting Leg Total	0					5					0					0					5	
Single-Unit Trucks	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	7	0	8	0	0	8	15
% Single-Unit	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	0.0	100.0		0.0	61.5	0.0	0.0	61.5	75.0
Exiting Leg Total	0					15					0					0					15	
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Exiting Leg Total	0					0					0					0					0	

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	3	0	0	3	4
12:00 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	0	3	0	0	3	6
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
Total Volume	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	10	0	0	10	14
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.333	0.000	0.000	0.000	0.333	0.000	0.833	0.000	0.000	0.833	0.583
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	4
Buses %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	40.0	0.0	0.0	40.0	28.6
Single-Unit Trucks	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	6	0	0	6	10
Single-Unit %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	0.0	100.0	0.0	60.0	0.0	0.0	60.0	71.4
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	4
Single-Unit Trucks	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	6	0	0	6	10
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Entering Leg	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	10	0	0	10	14
Buses	0					4					0					0					4
Single-Unit Trucks	0					10					0					0					10
Articulated Trucks	0					0					0					0					0
Total Exiting Leg	0					14					0					0					14

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars

	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
11:30 AM	0	0	0	0	0	0	0	0	0	0	64	0	0	0	0	64	0	100	0	0	100	164
11:45 AM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	0	66	0	94	0	0	94	160
Total	0	0	0	0	0	0	0	0	0	0	130	0	0	0	0	130	0	194	0	0	194	324
12:00 PM	0	0	1	0	1	0	0	0	0	0	44	0	0	0	0	44	0	96	0	0	96	141
12:15 PM	0	0	0	0	0	0	0	0	0	0	63	0	0	0	0	63	0	110	1	0	111	174
12:30 PM	0	0	0	0	0	0	0	0	0	0	78	0	0	0	0	78	0	118	0	0	118	196
12:45 PM	0	0	0	0	0	0	0	0	0	0	53	0	0	0	0	53	0	94	0	0	94	147
Total	0	0	1	0	1	0	0	0	0	0	238	0	0	0	0	238	0	418	1	0	419	658
1:00 PM	0	0	0	0	0	0	0	0	0	0	46	1	0	0	0	47	0	100	0	0	100	147
1:15 PM	0	0	0	0	0	0	0	0	0	0	52	0	0	0	0	52	0	91	0	0	91	143
Total	0	0	0	0	0	0	0	0	0	0	98	1	0	0	0	99	0	191	0	0	191	290
Grand Total	0	0	1	0	1	0	0	0	0	0	466	1	0	0	0	467	0	803	1	0	804	1272
Approach %	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0		99.8	0.2	0.0	0.0		0.0	99.9	0.1	0.0			
Total %	0.0	0.0	0.1	0.0	0.1	0.0	0.0	0.0	0.0	0.0	36.6	0.1	0.0	0.0	36.7	0.0	63.1	0.1	0.0	63.2		
Exiting Leg Total	2					1270					0					0					1272	

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Driveway					Main Street					Mill Road					Main Street						
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Total	
11:45 AM	0	0	0	0	0	0	0	0	0	0	66	0	0	0	0	66	0	94	0	0	94	160
12:00 PM	0	0	1	0	1	0	0	0	0	0	44	0	0	0	0	44	0	96	0	0	96	141
12:15 PM	0	0	0	0	0	0	0	0	0	0	63	0	0	0	0	63	0	110	1	0	111	174
12:30 PM	0	0	0	0	0	0	0	0	0	0	78	0	0	0	0	78	0	118	0	0	118	196
Total Volume	0	0	1	0	1	0	0	0	0	0	251	0	0	0	0	251	0	418	1	0	419	671
% Approach Total	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0			0.0	99.8	0.2	0.0		
PHF	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.804	0.000	0.000	0.000	0.804		0.000	0.886	0.250	0.000	0.888	0.856
Entering Leg	0	0	1	0	1	0	0	0	0	0	251	0	0	0	0	251	0	418	1	0	419	671
Exiting Leg	1					670					0					0					671	
Total	2					670					251					419					1342	

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Light Goods Vehicle

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	10	0	0	10	12
11:45 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	8	0	0	8	12
Total	0	0	0	0	0	0	0	0	0	0	6	0	0	0	6	0	18	0	0	18	24
12:00 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	0	13	0	0	13	20
12:15 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	11	0	0	11	13
12:30 PM	0	0	0	0	0	0	0	0	0	0	9	0	0	0	9	0	18	0	0	18	27
12:45 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	7	0	0	7	11
Total	0	0	0	0	0	0	0	0	0	0	22	0	0	0	22	0	49	0	0	49	71
1:00 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	9	0	0	9	13
1:15 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	18	0	0	18	22
Total	0	0	0	0	0	0	0	0	0	0	8	0	0	0	8	0	27	0	0	27	35
Grand Total	0	0	0	0	0	0	0	0	0	0	36	0	0	0	36	0	94	0	0	94	130
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	27.7	0.0	0.0	0.0	27.7	0.0	72.3	0.0	0.0	72.3	
Exiting Leg Total	0					130					0					0					130

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
12:30 PM	0	0	0	0	0	0	0	0	0	0	9	0	0	0	9	0	18	0	0	18	27	
12:45 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	7	0	0	7	11	
1:00 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	9	0	0	9	13	
1:15 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	18	0	0	18	22	
Total Volume	0	0	0	0	0	0	0	0	0	0	21	0	0	0	21	0	52	0	0	52	73	
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0			
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.583	0.000	0.000	0.000	0.583	0.000	0.722	0.000	0.000	0.722	0.676	
Entering Leg	0	0	0	0	0	0	0	0	0	0	21	0	0	0	21	0	52	0	0	52	73	
Exiting Leg	0					73					0					0					73	
Total	0					73					21					52					146	

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Buses

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	0	5
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	100.0	
Exiting Leg Total	0					5					0					0					5

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.333	0.000	0.000	0.333	0.333
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4
Exiting Leg	0					4					0					0					4
Total	0					4					0					4					8

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Single-Unit Trucks

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	2	0	0	2	3
Total	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	2	0	0	2	4
12:00 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	0	0	0	0	0	3
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
12:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2
Total	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	5	0	0	5	9
1:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2
Grand Total	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	0	8	0	0	8	15
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	46.7	0.0	0.0	0.0	46.7	0.0	53.3	0.0	0.0	53.3	
Exiting Leg Total	0					15					0					0					15

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	2	0	0	2	3
12:00 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	0	0	0	0	0	3
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
Total Volume	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	6	0	0	6	10
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.333	0.000	0.000	0.000	0.333	0.000	0.750	0.000	0.000	0.750	0.833
Entering Leg	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	6	0	0	6	10
Exiting Leg	0					10					0					0					10
Total	0					10					4					6					20

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Articulated Trucks

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Exiting Leg Total	0					0					0					0					0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exiting Leg	0					0					0					0					0
Total	0					0					0					0					0

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Bicycles (on Roadway and Crosswalks)

	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	0	1	0	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	0	1	0	0	0	0	1
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	1	0	0	1	1	3
1:00 PM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	1	1	0	0	0	0	0	0	4	0	0	0	0	0	0	4	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	1	1	0	0	0	0	0	0	6	1	0	0	0	0	0	7	0	2	0	0	1	1	4
Approach %	0.0	0.0	0.0	0.0	0.0	100.0			0.0	0.0	0.0	0.0	0.0	0.0	85.7	14.3	0.0	0.0	0.0	0.0			0.0	50.0	0.0	0.0	25.0	25.0	
Total %	0.0	0.0	0.0	0.0	0.0	8.3	8.3		0.0	0.0	0.0	0.0	0.0	0.0	50.0	8.3	0.0	0.0	0.0	0.0	58.3		0.0	16.7	0.0	0.0	8.3	8.3	33.3
Exiting Leg Total	2							8							0							2							12

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	
1:00 PM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	2
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3	0	0	0	0	0	0	3
Total Volume	0	0	0	0	0	0	1	1	0	0	0	0	0	0	5	0	0	0	0	0	0	5	0	1	0	0	0	0	7
% Approach Total	0.0	0.0	0.0	0.0	0.0	0.0	100.0		0.0	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0	0.0	0.0	
PHF	0.000	0.000	0.000	0.000	0.000	0.250	0.250		0.000	0.000	0.000	0.000	0.000	0.000	0.417	0.000	0.000	0.000	0.000	0.000	0.417		0.000	0.250	0.000	0.000	0.000	0.250	0.583
Entering Leg	0	0	0	0	0	0	1	1	0	0	0	0	0	0	5	0	0	0	0	0	0	5	0	1	0	0	0	0	7
Exiting Leg	1							6							0							0							7
Total	2							6							5							1							14

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Pedestrians

	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	0	61	51	112	0	0	0	0	4	11	15	0	0	0	0	7	20	27	0	0	0	0	0	0	0	154
11:45 AM	0	0	0	0	68	50	118	0	0	0	0	4	20	24	0	0	0	0	4	8	12	0	0	0	0	0	0	0	154
Total	0	0	0	0	129	101	230	0	0	0	0	8	31	39	0	0	0	0	11	28	39	0	0	0	0	0	0	0	308
12:00 PM	0	0	0	0	61	38	99	0	0	0	0	9	11	20	0	0	0	0	5	10	15	0	0	0	0	0	0	0	134
12:15 PM	0	0	0	0	57	18	75	0	0	0	0	8	24	32	0	0	0	0	10	14	24	0	0	0	0	0	0	0	131
12:30 PM	0	0	0	0	102	76	178	0	0	0	0	9	8	17	0	0	0	0	13	3	16	0	0	0	0	0	0	0	211
12:45 PM	0	0	0	0	43	50	93	0	0	0	0	8	4	12	0	0	0	0	5	22	27	0	0	0	0	0	0	0	132
Total	0	0	0	0	263	182	445	0	0	0	0	34	47	81	0	0	0	0	33	49	82	0	0	0	0	0	0	0	608
1:00 PM	0	0	0	0	86	56	142	0	0	0	0	7	21	28	0	0	0	0	20	31	51	0	0	0	0	0	0	0	221
1:15 PM	0	0	0	0	101	73	174	0	0	0	0	54	12	66	0	0	0	0	15	21	36	0	0	0	0	0	0	0	276
Total	0	0	0	0	187	129	316	0	0	0	0	61	33	94	0	0	0	0	35	52	87	0	0	0	0	0	0	0	497
Grand Total	0	0	0	0	579	412	991	0	0	0	0	103	111	214	0	0	0	0	79	129	208	0	0	0	0	0	0	0	1413
Approach %	0	0	0	0	58.4	41.6		0	0	0	0	48.1	51.9		0	0	0	0	38	62		0	0	0	0	0	0		
Total %	0	0	0	0	41	29.2	70.1	0	0	0	0	7.29	7.86	15.1	0	0	0	0	5.59	9.13	14.7	0	0	0	0	0	0	0	
Exiting Leg Total	991							214							208							0							1413

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
12:30 PM	0	0	0	0	102	76	178	0	0	0	0	9	8	17	0	0	0	0	13	3	16	0	0	0	0	0	0	0	211
12:45 PM	0	0	0	0	43	50	93	0	0	0	0	8	4	12	0	0	0	0	5	22	27	0	0	0	0	0	0	0	132
1:00 PM	0	0	0	0	86	56	142	0	0	0	0	7	21	28	0	0	0	0	20	31	51	0	0	0	0	0	0	0	221
1:15 PM	0	0	0	0	101	73	174	0	0	0	0	54	12	66	0	0	0	0	15	21	36	0	0	0	0	0	0	0	276
Total Volume	0	0	0	0	332	255	587	0	0	0	0	78	45	123	0	0	0	0	53	77	130	0	0	0	0	0	0	0	840
% Approach Total	0.0	0.0	0.0	0.0	56.6	43.4		0.0	0.0	0.0	0.0	63.4	36.6		0.0	0.0	0.0	0.0	40.8	59.2		0.0	0.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.814	0.839	0.824	0.000	0.000	0.000	0.000	0.361	0.536	0.466	0.000	0.000	0.000	0.000	0.663	0.621	0.637	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.761
Entering Leg	0	0	0	0	332	255	587	0	0	0	0	78	45	123	0	0	0	0	53	77	130	0	0	0	0	0	0	0	840
Exiting Leg	587							123							130							0							840
Total	1174							246							260							0							1680

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	97	0	1	0	98	0	135	1	0	136	234
4:15 PM	0	0	0	0	0	0	0	0	0	0	87	0	4	0	91	0	129	1	0	130	221
4:30 PM	0	0	0	0	0	0	0	0	0	0	115	0	0	0	115	0	149	0	0	149	264
4:45 PM	0	0	0	0	0	0	0	0	0	0	135	0	1	0	136	0	160	1	0	161	297
Total	0	0	0	0	0	0	0	0	0	0	434	0	6	0	440	0	573	3	0	576	1016
5:00 PM	0	0	0	0	0	0	0	0	0	0	121	0	0	0	121	0	163	1	0	164	285
5:15 PM	0	0	0	0	0	0	0	0	0	0	111	0	0	0	111	0	113	0	0	113	224
5:30 PM	0	0	0	0	0	0	0	0	0	0	92	0	0	0	92	0	139	0	0	139	231
5:45 PM	0	0	0	0	0	0	0	0	0	0	114	1	0	0	115	0	110	1	0	111	226
Total	0	0	0	0	0	0	0	0	0	0	438	1	0	0	439	0	525	2	0	527	966
Grand Total	0	0	0	0	0	0	0	0	0	0	872	1	6	0	879	0	1098	5	0	1103	1982
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		99.2	0.1	0.7	0.0		0.0	99.5	0.5	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	44.0	0.1	0.3	0.0	44.3	0.0	55.4	0.3	0.0	55.7	
Exiting Leg Total	6					1970					0					6					1982
Cars	0	0	0	0	0	0	0	0	0	0	870	1	6	0	877	0	1067	5	0	1072	1949
% Cars	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	99.8	100.0	100.0	0.0	99.8	0.0	97.2	100.0	0.0	97.2	98.3
Exiting Leg Total	6					1937					0					6					1949
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	31	0	0	31	33
% Heavy Vehicles	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.2	0.0	2.8	0.0	0.0	2.8	1.7
Exiting Leg Total	0					33					0					0					33

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Driveway					Main Street					Mill Road					Main Street					Total		
	from North					from East					from South					from West							
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total			
4:30 PM	0	0	0	0	0	0	0	0	0	0	115	0	0	0	115	0	149	0	0	149	264		
4:45 PM	0	0	0	0	0	0	0	0	0	0	135	0	1	0	136	0	160	1	0	161	297		
5:00 PM	0	0	0	0	0	0	0	0	0	0	121	0	0	0	121	0	163	1	0	164	285		
5:15 PM	0	0	0	0	0	0	0	0	0	0	111	0	0	0	111	0	113	0	0	113	224		
Total Volume	0	0	0	0	0	0	0	0	0	0	482	0	1	0	483	0	585	2	0	587	1070		
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		99.8	0.0	0.2	0.0		0.0	99.7	0.3	0.0				
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.893	0.000	0.250	0.000	0.888	0.000	0.897	0.500	0.000	0.895	0.901		
Cars	0	0	0	0	0	0	0	0	0	0	481	0	1	0	482	0	570	2	0	572	1054		
Cars %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	99.8	0.0	100.0	0.0	99.8	0.0	97.4	100.0	0.0	97.4	98.5		
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	15	0	0	15	16		
Heavy Vehicles %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.2	0.0	2.6	0.0	0.0	2.6	1.5		
Cars Enter Leg	0	0	0	0	0	0	0	0	0	0	481	0	1	0	482	0	570	2	0	572	1054		
Heavy Enter Leg	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	15	0	0	15	16		
Total Entering Leg	0	0	0	0	0	0	0	0	0	0	482	0	1	0	483	0	585	2	0	587	1070		
Cars Exiting Leg	2															0					1		1054
Heavy Exiting Leg	0															0					0		16
Total Exiting Leg	2															0					1		1070

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars-Combined (Motorcycles, Cars, Light Goods)

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	97	0	1	0	98	0	129	1	0	130	228
4:15 PM	0	0	0	0	0	0	0	0	0	0	87	0	4	0	91	0	125	1	0	126	217
4:30 PM	0	0	0	0	0	0	0	0	0	0	115	0	0	0	115	0	145	0	0	145	260
4:45 PM	0	0	0	0	0	0	0	0	0	0	134	0	1	0	135	0	156	1	0	157	292
Total	0	0	0	0	0	0	0	0	0	0	433	0	6	0	439	0	555	3	0	558	997
5:00 PM	0	0	0	0	0	0	0	0	0	0	121	0	0	0	121	0	159	1	0	160	281
5:15 PM	0	0	0	0	0	0	0	0	0	0	111	0	0	0	111	0	110	0	0	110	221
5:30 PM	0	0	0	0	0	0	0	0	0	0	92	0	0	0	92	0	135	0	0	135	227
5:45 PM	0	0	0	0	0	0	0	0	0	0	113	1	0	0	114	0	108	1	0	109	223
Total	0	0	0	0	0	0	0	0	0	0	437	1	0	0	438	0	512	2	0	514	952
Grand Total	0	0	0	0	0	0	0	0	0	0	870	1	6	0	877	0	1067	5	0	1072	1949
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		99.2	0.1	0.7	0.0		0.0	99.5	0.5	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	44.6	0.1	0.3	0.0	45.0	0.0	54.7	0.3	0.0	55.0	
Exiting Leg Total	6					1937					0					6					1949

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:30 PM	0	0	0	0	0	0	0	0	0	0	115	0	0	0	115	0	145	0	0	145	260
4:45 PM	0	0	0	0	0	0	0	0	0	0	134	0	1	0	135	0	156	1	0	157	292
5:00 PM	0	0	0	0	0	0	0	0	0	0	121	0	0	0	121	0	159	1	0	160	281
5:15 PM	0	0	0	0	0	0	0	0	0	0	111	0	0	0	111	0	110	0	0	110	221
Total Volume	0	0	0	0	0	0	0	0	0	0	481	0	1	0	482	0	570	2	0	572	1054
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		99.8	0.0	0.2	0.0		0.0	99.7	0.3	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.897	0.000	0.250	0.000	0.893	0.000	0.896	0.500	0.000	0.894	0.902
Entering Leg	0	0	0	0	0	0	0	0	0	0	481	0	1	0	482	0	570	2	0	572	1054
Exiting Leg	2					1051					0					1					1054
Total	2					1051					482					573					2108

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**



Class: **Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)**

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	0	6
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	4	0	0	0	4
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	18	0	0	0	18
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4
5:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	2	0	0	0	2
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	13	0	0	0	13
Grand Total	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	31	0	0	0	31
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.1	0.0	0.0	0.0	6.1	0.0	93.9	0.0	0.0		93.9
Exiting Leg Total	0					33					0					0					33
Buses	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	24	0	0	0	24
% Buses	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	50.0	0.0	0.0	0.0	50.0	0.0	77.4	0.0	0.0	0.0	77.4
Exiting Leg Total	0					25					0					0					25
Single-Unit Trucks	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	5	0	0	0	5
% Single-Unit	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	50.0	0.0	0.0	0.0	50.0	0.0	16.1	0.0	0.0	0.0	16.1
Exiting Leg Total	0					6					0					0					6
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.5	0.0	0.0	0.0	6.5
Exiting Leg Total	0					2					0					0					2

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6	6
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	4
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	4
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	4	0	0	4	5
Total Volume	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	18	0	0	18	19
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0			
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.750	0.000	0.000	0.750	0.792	
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	13	13
Buses %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	72.2	0.0	0.0	72.2	68.4
Single-Unit Trucks	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	3	0	0	3	4
Single-Unit %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	0.0	100.0	0.0	16.7	0.0	0.0	0.0	16.7	21.1
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11.1	0.0	0.0	11.1	10.5
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	13	13
Single-Unit Trucks	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	3	0	0	3	4
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
Total Entering Leg	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	18	0	0	18	19
Buses	0					13					0					0					13	
Single-Unit Trucks	0					4					0					0					4	
Articulated Trucks	0					2					0					0					2	
Total Exiting Leg	0					19					0					0					19	

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	83	0	1	0	84	0	114	1	0	115	199
4:15 PM	0	0	0	0	0	0	0	0	0	0	81	0	3	0	84	0	111	1	0	112	196
4:30 PM	0	0	0	0	0	0	0	0	0	0	108	0	0	0	108	0	123	0	0	123	231
4:45 PM	0	0	0	0	0	0	0	0	0	0	126	0	0	0	126	0	131	1	0	132	258
Total	0	0	0	0	0	0	0	0	0	0	398	0	4	0	402	0	479	3	0	482	884
5:00 PM	0	0	0	0	0	0	0	0	0	0	108	0	0	0	108	0	142	1	0	143	251
5:15 PM	0	0	0	0	0	0	0	0	0	0	99	0	0	0	99	0	101	0	0	101	200
5:30 PM	0	0	0	0	0	0	0	0	0	0	81	0	0	0	81	0	121	0	0	121	202
5:45 PM	0	0	0	0	0	0	0	0	0	0	96	1	0	0	97	0	91	1	0	92	189
Total	0	0	0	0	0	0	0	0	0	0	384	1	0	0	385	0	455	2	0	457	842
Grand Total	0	0	0	0	0	0	0	0	0	0	782	1	4	0	787	0	934	5	0	939	1726
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		99.4	0.1	0.5	0.0		0.0	99.5	0.5	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	45.3	0.1	0.2	0.0	45.6	0.0	54.1	0.3	0.0	54.4	
Exiting Leg Total	6					1716					0					4					1726

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
4:30 PM	0	0	0	0	0	0	0	0	0	0	108	0	0	0	0	108	0	123	0	0	123	231
4:45 PM	0	0	0	0	0	0	0	0	0	0	126	0	0	0	0	126	0	131	1	0	132	258
5:00 PM	0	0	0	0	0	0	0	0	0	0	108	0	0	0	0	108	0	142	1	0	143	251
5:15 PM	0	0	0	0	0	0	0	0	0	0	99	0	0	0	0	99	0	101	0	0	101	200
Total Volume	0	0	0	0	0	0	0	0	0	0	441	0	0	0	0	441	0	497	2	0	499	940
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0	0.0		0.0	99.6	0.4	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.875	0.000	0.000	0.000	0.875		0.000	0.875	0.500	0.000	0.872	0.911
Entering Leg	0	0	0	0	0	0	0	0	0	0	441	0	0	0	0	441	0	497	2	0	499	940
Exiting Leg	2					938					0					0					940	
Total	2					938					441					499					1880	

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Light Goods Vehicle

	Driveway					Main Street					Mill Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
4:00 PM	0	0	0	0	0	0	0	0	0	0	11	0	0	0	0	11	0	11	0	0	11	22
4:15 PM	0	0	0	0	0	0	0	0	0	0	6	0	1	0	7	7	0	8	0	0	8	15
4:30 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	7	0	17	0	0	17	24
4:45 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	7	0	17	0	0	17	24
Total	0	0	0	0	0	0	0	0	0	0	31	0	1	0	32	32	0	53	0	0	53	85
5:00 PM	0	0	0	0	0	0	0	0	0	0	11	0	0	0	11	11	0	10	0	0	10	21
5:15 PM	0	0	0	0	0	0	0	0	0	0	8	0	0	0	8	8	0	7	0	0	7	15
5:30 PM	0	0	0	0	0	0	0	0	0	0	9	0	0	0	9	9	0	9	0	0	9	18
5:45 PM	0	0	0	0	0	0	0	0	0	0	10	0	0	0	10	10	0	9	0	0	9	19
Total	0	0	0	0	0	0	0	0	0	0	38	0	0	0	38	38	0	35	0	0	35	73
Grand Total	0	0	0	0	0	0	0	0	0	0	69	0	1	0	70	70	0	88	0	0	88	158
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		98.6	0.0	1.4	0.0			0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	43.7	0.0	0.6	0.0	44.3		0.0	55.7	0.0	0.0	55.7	
Exiting Leg Total	0					157					0					1					158	

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	11	0	0	0	11	0	11	0	0	11	22
4:15 PM	0	0	0	0	0	0	0	0	0	0	6	0	1	0	7	0	8	0	0	8	15
4:30 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	0	17	0	0	17	24
4:45 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	7	0	17	0	0	17	24
Total Volume	0	0	0	0	0	0	0	0	0	0	31	0	1	0	32	0	53	0	0	53	85
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		96.9	0.0	3.1	0.0		0.0	100.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.705	0.000	0.250	0.000	0.727	0.000	0.779	0.000	0.000	0.779	0.885
Entering Leg	0	0	0	0	0	0	0	0	0	0	31	0	1	0	32	0	53	0	0	53	85
Exiting Leg						84					0					1					85
Total	0					84					32					54					170

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Buses

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	0	5
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	0	13
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4
5:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	11	0	0	0	11
Grand Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	24	0	0	0	24
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.0	0.0	0.0	0.0	4.0	0.0	96.0	0.0	0.0	0.0	96.0
Exiting Leg Total	0					25					0					0					25

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Driveway					Main Street					Mill Road					Main Street						
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	0	5	5
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	3
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	3
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	0	13	13
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0			
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.650	0.000	0.000	0.650	0.650	
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0	0	13	13	
Exiting Leg																					0	13
Total	0					13					0					13					26	

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Single-Unit Trucks

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	3	0	0	3
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
Grand Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	5	0	0	5
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0			0.0	100.0	0.0	0.0	
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	16.7	0.0	0.0	0.0	16.7		0.0	83.3	0.0	0.0	83.3
Exiting Leg Total	0					6					0					0					6

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Driveway					Main Street					Mill Road					Main Street						
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	1
Total Volume	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	3	0	0	3	4
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0			
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.750	0.000	0.000	0.750	1.000	
Entering Leg	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	3	0	0	3	4	
Exiting Leg	0					4					0					0					4	
Total	0					4					1					3					8	

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Articulated Trucks

	Driveway					Main Street					Mill Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100.0	0.0	0.0	100.0	
Exiting Leg Total	0					2					0					0					2

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Driveway					Main Street					Mill Road					Main Street					
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Total
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.500	0.500
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
Exiting Leg	0					2					0					0					2
Total	0					2					0					2					4

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



46 Morton Street, Framingham, MA 01702
 Office: 508-875-0100 Fax: 508-875-0118
 Email: datarequests@pdillc.com

Bicycles (on Roadway and Crosswalks)

	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1		
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2	
5:00 PM	0	0	0	0	2	1	3	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	4	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	2	0	2	1	0	0	0	0	0	0	1	0	1	0	0	1	4	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	2	
Total	0	0	0	0	3	1	4	0	0	0	0	2	1	3	1	0	0	0	0	0	0	1	0	2	0	0	0	2	10
Grand Total	0	0	0	0	3	1	4	0	0	0	0	2	1	3	1	0	0	0	0	0	0	1	0	4	0	0	0	4	12
Approach %	0.0	0.0	0.0	0.0	75.0	25.0		0.0	0.0	0.0	0.0	66.7	33.3		100.0	0.0	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	25.0	8.3	33.3	0.0	0.0	0.0	0.0	16.7	8.3	25.0	8.3	0.0	0.0	0.0	0.0	0.0	8.3	0.0	33.3	0.0	0.0	0.0	0.0	33.3	
Exiting Leg Total	4							8							0							0							12

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

5:00 PM	Driveway							Main Street							Mill Road							Main Street							
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
5:00 PM	0	0	0	0	2	1	3	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	2	0	2	1	0	0	0	0	0	0	1	0	1	0	0	0	1	4
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	2
Total Volume	0	0	0	0	3	1	4	0	0	0	0	2	1	3	1	0	0	0	0	0	0	1	0	2	0	0	0	2	10
% Approach Total	0.0	0.0	0.0	0.0	75.0	25.0		0.0	0.0	0.0	0.0	66.7	33.3		100.0	0.0	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.375	0.250	0.333	0.000	0.000	0.000	0.000	0.250	0.250	0.375	0.250	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.500	0.000	0.000	0.000	0.000	0.500	0.625
Entering Leg	0	0	0	0	3	1	4	0	0	0	0	2	1	3	1	0	0	0	0	0	1	0	2	0	0	0	0	2	10
Exiting Leg	4							6							0							0							10
Total	8							9							1							2							20

PDI File #: **186243 A**
 Location: **N: Driveway S: Mill Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Pedestrians

	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
4:00 PM	0	0	0	0	55	31	86	0	0	0	0	0	1	1	0	0	0	0	7	4	11	0	0	0	0	0	0	0	98
4:15 PM	0	0	0	0	44	44	88	0	0	0	0	1	0	1	0	0	0	0	5	16	21	0	0	0	0	0	0	0	110
4:30 PM	0	0	0	0	31	32	63	0	0	0	0	1	6	7	0	0	0	0	7	13	20	0	0	0	0	0	0	0	90
4:45 PM	0	0	0	0	33	28	61	0	0	0	0	1	7	8	0	0	0	0	7	14	21	0	0	0	0	0	0	0	90
Total	0	0	0	0	163	135	298	0	0	0	0	3	14	17	0	0	0	0	26	47	73	0	0	0	0	0	0	0	388
5:00 PM	0	0	0	0	40	40	80	0	0	0	0	1	4	5	0	0	0	0	9	16	25	0	0	0	0	0	0	0	110
5:15 PM	0	0	0	0	31	27	58	0	0	0	0	0	5	5	0	0	0	0	15	10	25	0	0	0	0	0	0	0	88
5:30 PM	0	0	0	0	33	24	57	0	0	0	0	2	4	6	0	0	0	0	9	14	23	0	0	0	0	0	0	0	86
5:45 PM	0	0	0	0	25	36	61	0	0	0	0	2	1	3	0	0	0	0	17	13	30	0	0	0	0	0	0	0	94
Total	0	0	0	0	129	127	256	0	0	0	0	5	14	19	0	0	0	0	50	53	103	0	0	0	0	0	0	0	378
Grand Total	0	0	0	0	292	262	554	0	0	0	0	8	28	36	0	0	0	0	76	100	176	0	0	0	0	0	0	0	766
Approach %	0	0	0	0	52.7	47.3		0	0	0	0	22.2	77.8		0	0	0	0	43.2	56.8		0	0	0	0	0	0		
Total %	0	0	0	0	38.1	34.2	72.3	0	0	0	0	1.04	3.66	4.7	0	0	0	0	9.92	13.1	23	0	0	0	0	0	0		
Exiting Leg Total	554							36							176							0							766

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:15 PM	Driveway							Main Street							Mill Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
4:15 PM	0	0	0	0	44	44	88	0	0	0	0	1	0	1	0	0	0	0	5	16	21	0	0	0	0	0	0	0	110
4:30 PM	0	0	0	0	31	32	63	0	0	0	0	1	6	7	0	0	0	0	7	13	20	0	0	0	0	0	0	0	90
4:45 PM	0	0	0	0	33	28	61	0	0	0	0	1	7	8	0	0	0	0	7	14	21	0	0	0	0	0	0	0	90
5:00 PM	0	0	0	0	40	40	80	0	0	0	0	1	4	5	0	0	0	0	9	16	25	0	0	0	0	0	0	0	110
Total Volume	0	0	0	0	148	144	292	0	0	0	0	4	17	21	0	0	0	0	28	59	87	0	0	0	0	0	0	0	400
% Approach Total	0.0	0.0	0.0	0.0	50.7	49.3		0.0	0.0	0.0	0.0	19.0	81.0		0.0	0.0	0.0	0.0	32.2	67.8		0.0	0.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.841	0.818	0.830	0.000	0.000	0.000	0.000	1.000	0.607	0.656	0.000	0.000	0.000	0.000	0.778	0.922	0.870	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.909
Entering Leg	0	0	0	0	148	144	292	0	0	0	0	4	17	21	0	0	0	0	28	59	87	0	0	0	0	0	0	0	400
Exiting Leg	292							21							87							0							400
Total	584							42							174							0							800

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	27	24	0	51	27	10	0	37	12	41	0	53	141
11:45 AM	45	24	0	69	24	21	0	45	16	49	0	65	179
Total	72	48	0	120	51	31	0	82	28	90	0	118	320
12:00 PM	31	26	0	57	20	11	0	31	19	41	0	60	148
12:15 PM	28	32	0	60	31	15	0	46	15	32	0	47	153
12:30 PM	34	28	0	62	33	14	0	47	9	53	0	62	171
12:45 PM	39	28	0	67	25	15	0	40	13	32	0	45	152
Total	132	114	0	246	109	55	0	164	56	158	0	214	624
1:00 PM	38	17	0	55	19	15	0	34	16	36	0	52	141
1:15 PM	39	23	0	62	24	9	0	33	17	30	0	47	142
Total	77	40	0	117	43	24	0	67	33	66	0	99	283
Grand Total	281	202	0	483	203	110	0	313	117	314	0	431	1227
Approach %	58.2	41.8	0.0		64.9	35.1	0.0		27.1	72.9	0.0		
Total %	22.9	16.5	0.0	39.4	16.5	9.0	0.0	25.5	9.5	25.6	0.0	35.1	
Exiting Leg Total				517				319				391	1227
Cars	273	199	0	472	200	109	0	309	117	310	0	427	1208
% Cars	97.2	98.5	0.0	97.7	98.5	99.1	0.0	98.7	100.0	98.7	0.0	99.1	98.5
Exiting Leg Total				510				316				382	1208
Heavy Vehicles	8	3	0	11	3	1	0	4	0	4	0	4	19
% Heavy Vehicles	2.8	1.5	0.0	2.3	1.5	0.9	0.0	1.3	0.0	1.3	0.0	0.9	1.5
Exiting Leg Total				7				3				9	19

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:45 AM	45	24	0	69	24	21	0	45	16	49	0	65	179
12:00 PM	31	26	0	57	20	11	0	31	19	41	0	60	148
12:15 PM	28	32	0	60	31	15	0	46	15	32	0	47	153
12:30 PM	34	28	0	62	33	14	0	47	9	53	0	62	171
Total Volume	138	110	0	248	108	61	0	169	59	175	0	234	651
% Approach Total	55.6	44.4	0.0		63.9	36.1	0.0		25.2	74.8	0.0		
PHF	0.767	0.859	0.000	0.899	0.818	0.726	0.000	0.899	0.776	0.825	0.000	0.900	0.909
Cars	134	109	0	243	107	60	0	167	59	172	0	231	641
Cars %	97.1	99.1	0.0	98.0	99.1	98.4	0.0	98.8	100.0	98.3	0.0	98.7	98.5
Heavy Vehicles	4	1	0	5	1	1	0	2	0	3	0	3	10
Heavy Vehicles %	2.9	0.9	0.0	2.0	0.9	1.6	0.0	1.2	0.0	1.7	0.0	1.3	1.5
Cars Enter Leg	134	109	0	243	107	60	0	167	59	172	0	231	641
Heavy Enter Leg	4	1	0	5	1	1	0	2	0	3	0	3	10
Total Entering Leg	138	110	0	248	108	61	0	169	59	175	0	234	651
Cars Exiting Leg				279				168				194	641
Heavy Exiting Leg				4				1				5	10
Total Exiting Leg				283				169				199	651

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**



Class: **Cars-Combined (Motorcycles, Cars, Light Goods)**

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	25	24	0	49	26	10	0	36	12	41	0	53	138
11:45 AM	45	23	0	68	23	21	0	44	16	48	0	64	176
Total	70	47	0	117	49	31	0	80	28	89	0	117	314
12:00 PM	28	26	0	54	20	11	0	31	19	39	0	58	143
12:15 PM	28	32	0	60	31	14	0	45	15	32	0	47	152
12:30 PM	33	28	0	61	33	14	0	47	9	53	0	62	170
12:45 PM	38	28	0	66	25	15	0	40	13	31	0	44	150
Total	127	114	0	241	109	54	0	163	56	155	0	211	615
1:00 PM	37	16	0	53	18	15	0	33	16	36	0	52	138
1:15 PM	39	22	0	61	24	9	0	33	17	30	0	47	141
Total	76	38	0	114	42	24	0	66	33	66	0	99	279
Grand Total	273	199	0	472	200	109	0	309	117	310	0	427	1208
Approach %	57.8	42.2	0.0		64.7	35.3	0.0		27.4	72.6	0.0		
Total %	22.6	16.5	0.0	39.1	16.6	9.0	0.0	25.6	9.7	25.7	0.0	35.3	
Exiting Leg Total				510				316				382	1208

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:45 AM	45	23	0	68	23	21	0	44	16	48	0	64	176
12:00 PM	28	26	0	54	20	11	0	31	19	39	0	58	143
12:15 PM	28	32	0	60	31	14	0	45	15	32	0	47	152
12:30 PM	33	28	0	61	33	14	0	47	9	53	0	62	170
Total Volume	134	109	0	243	107	60	0	167	59	172	0	231	641
% Approach Total	55.1	44.9	0.0		64.1	35.9	0.0		25.5	74.5	0.0		
PHF	0.744	0.852	0.000	0.893	0.811	0.714	0.000	0.888	0.776	0.811	0.000	0.902	0.911
Entering Leg	134	109	0	243	107	60	0	167	59	172	0	231	641
Exiting Leg				279				168				194	641
Total				522				335				425	1282

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**



Class: **Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)**

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	2	0	0	2	1	0	0	1	0	0	0	0	3
11:45 AM	0	1	0	1	1	0	0	1	0	1	0	1	3
Total	2	1	0	3	2	0	0	2	0	1	0	1	6
12:00 PM	3	0	0	3	0	0	0	0	0	2	0	2	5
12:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	1
12:30 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
12:45 PM	1	0	0	1	0	0	0	0	0	1	0	1	2
Total	5	0	0	5	0	1	0	1	0	3	0	3	9
1:00 PM	1	1	0	2	1	0	0	1	0	0	0	0	3
1:15 PM	0	1	0	1	0	0	0	0	0	0	0	0	1
Total	1	2	0	3	1	0	0	1	0	0	0	0	4
Grand Total	8	3	0	11	3	1	0	4	0	4	0	4	19
Approach %	72.7	27.3	0.0		75.0	25.0	0.0		0.0	100.0	0.0		
Total %	42.1	15.8	0.0	57.9	15.8	5.3	0.0	21.1	0.0	21.1	0.0	21.1	
Exiting Leg Total	7				3				9				19
Buses	6	0	0	6	0	0	0	0	0	0	0	0	6
% Buses	75.0	0.0	0.0	54.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	31.6
Exiting Leg Total	0				0				6				6
Single-Unit Trucks	2	3	0	5	3	1	0	4	0	4	0	4	13
% Single-Unit	25.0	100.0	0.0	45.5	100.0	100.0	0.0	100.0	0.0	100.0	0.0	100.0	68.4
Exiting Leg Total	7				3				3				13
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Exiting Leg Total	0				0				0				0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	2	0	0	2	1	0	0	1	0	0	0	0	3
11:45 AM	0	1	0	1	1	0	0	1	0	1	0	1	3
12:00 PM	3	0	0	3	0	0	0	0	0	2	0	2	5
12:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Volume	5	1	0	6	2	1	0	3	0	3	0	3	12
% Approach Total	83.3	16.7	0.0		66.7	33.3	0.0		0.0	100.0	0.0		
PHF	0.417	0.250	0.000	0.500	0.500	0.250	0.000	0.750	0.000	0.375	0.000	0.375	0.600
Buses	3	0	0	3	0	0	0	0	0	0	0	0	3
Buses %	60.0	0.0	0.0	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	25.0
Single-Unit Trucks	2	1	0	3	2	1	0	3	0	3	0	3	9
Single-Unit %	40.0	100.0	0.0	50.0	100.0	100.0	0.0	100.0	0.0	100.0	0.0	100.0	75.0
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Buses	3	0	0	3	0	0	0	0	0	0	0	0	3
Single-Unit Trucks	2	1	0	3	2	1	0	3	0	3	0	3	9
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Entering Leg	5	1	0	6	2	1	0	3	0	3	0	3	12
Buses				0				0				3	3
Single-Unit Trucks				5				1				3	9
Articulated Trucks				0				0				0	0
Total Exiting Leg				5				1				6	12

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	25	23	0	48	25	8	0	33	11	39	0	50	131
11:45 AM	36	19	0	55	23	19	0	42	14	43	0	57	154
Total	61	42	0	103	48	27	0	75	25	82	0	107	285
12:00 PM	22	23	0	45	18	10	0	28	19	32	0	51	124
12:15 PM	25	30	0	55	30	14	0	44	15	30	0	45	144
12:30 PM	32	24	0	56	31	13	0	44	9	46	0	55	155
12:45 PM	35	24	0	59	19	14	0	33	12	30	0	42	134
Total	114	101	0	215	98	51	0	149	55	138	0	193	557
1:00 PM	33	15	0	48	17	15	0	32	15	27	0	42	122
1:15 PM	33	20	0	53	20	8	0	28	16	27	0	43	124
Total	66	35	0	101	37	23	0	60	31	54	0	85	246
Grand Total	241	178	0	419	183	101	0	284	111	274	0	385	1088
Approach %	57.5	42.5	0.0		64.4	35.6	0.0		28.8	71.2	0.0		
Total %	22.2	16.4	0.0	38.5	16.8	9.3	0.0	26.1	10.2	25.2	0.0	35.4	
Exiting Leg Total				457				289				342	1088

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:45 AM	36	19	0	55	23	19	0	42	14	43	0	57	154
12:00 PM	22	23	0	45	18	10	0	28	19	32	0	51	124
12:15 PM	25	30	0	55	30	14	0	44	15	30	0	45	144
12:30 PM	32	24	0	56	31	13	0	44	9	46	0	55	155
Total Volume	115	96	0	211	102	56	0	158	57	151	0	208	577
% Approach Total	54.5	45.5	0.0		64.6	35.4	0.0		27.4	72.6	0.0		
PHF	0.799	0.800	0.000	0.942	0.823	0.737	0.000	0.898	0.750	0.821	0.000	0.912	0.931
Entering Leg	115	96	0	211	102	56	0	158	57	151	0	208	577
Exiting Leg				253				153				171	577
Total				464				311				379	1154

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



46 Morton Street, Framingham, MA 01702
 Office: 508-875-0100 Fax: 508-875-0118
 Email: datarequests@pdillc.com

Light Goods Vehicle

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	0	1	0	1	1	1	0	2	0	2	0	2	5
11:45 AM	7	3	0	10	0	2	0	2	2	3	0	5	17
Total	7	4	0	11	1	3	0	4	2	5	0	7	22
12:00 PM	4	3	0	7	2	1	0	3	0	5	0	5	15
12:15 PM	2	2	0	4	1	0	0	1	0	1	0	1	6
12:30 PM	1	3	0	4	2	0	0	2	0	7	0	7	13
12:45 PM	2	4	0	6	5	1	0	6	1	0	0	1	13
Total	9	12	0	21	10	2	0	12	1	13	0	14	47
1:00 PM	4	1	0	5	1	0	0	1	1	7	0	8	14
1:15 PM	6	1	0	7	3	1	0	4	1	1	0	2	13
Total	10	2	0	12	4	1	0	5	2	8	0	10	27
Grand Total	26	18	0	44	15	6	0	21	5	26	0	31	96
Approach %	59.1	40.9	0.0		71.4	28.6	0.0		16.1	83.9	0.0		
Total %	27.1	18.8	0.0	45.8	15.6	6.3	0.0	21.9	5.2	27.1	0.0	32.3	
Exiting Leg Total				41				23				32	96

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
12:30 PM	1	3	0	4	2	0	0	2	0	7	0	7	13
12:45 PM	2	4	0	6	5	1	0	6	1	0	0	1	13
1:00 PM	4	1	0	5	1	0	0	1	1	7	0	8	14
1:15 PM	6	1	0	7	3	1	0	4	1	1	0	2	13
Total Volume	13	9	0	22	11	2	0	13	3	15	0	18	53
% Approach Total	59.1	40.9	0.0		84.6	15.4	0.0		16.7	83.3	0.0		
PHF	0.542	0.563	0.000	0.786	0.550	0.500	0.000	0.542	0.750	0.536	0.000	0.563	0.946
Entering Leg	13	9	0	22	11	2	0	13	3	15	0	18	53
Exiting Leg				26				12				15	53
Total				48				25				33	106

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



46 Morton Street, Framingham, MA 01702
 Office: 508-875-0100 Fax: 508-875-0118
 Email: datarequests@pdillc.com

Buses

	Mill Road					Mill Road Plaza Driveway					Mill Road					Total
	from North					from East					from South					
	Thru	Left	U-Turn	Total		Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total		
11:30 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
12:00 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
12:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
Total	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	4
1:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
Grand Total	6	0	0	0	6	0	0	0	0	0	0	0	0	0	0	6
Approach %	100.0	0.0	0.0			0.0	0.0	0.0			0.0	0.0	0.0			
Total %	100.0	0.0	0.0		100.0	0.0	0.0	0.0		0.0	0.0	0.0		0.0		
Exiting Leg Total	0					0					6					6

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:00 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
12:00 PM	2	0	0	2	0	0	0	0	0	0	0	0	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
12:45 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
Total Volume	4	0	0	4	0	0	0	0	0	0	0	0	4
% Approach Total	100.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.500	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.500
Entering Leg	4	0	0	4	0	0	0	0	0	0	0	0	4
Exiting Leg				0				0				4	4
Total				4				0				4	8

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Single-Unit Trucks

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	1	0	0	1	1	0	0	1	0	0	0	0	2
11:45 AM	0	1	0	1	1	0	0	1	0	1	0	1	3
Total	1	1	0	2	2	0	0	2	0	1	0	1	5
12:00 PM	1	0	0	1	0	0	0	0	0	2	0	2	3
12:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	1	0	1	1
Total	1	0	0	1	0	1	0	1	0	3	0	3	5
1:00 PM	0	1	0	1	1	0	0	1	0	0	0	0	2
1:15 PM	0	1	0	1	0	0	0	0	0	0	0	0	1
Total	0	2	0	2	1	0	0	1	0	0	0	0	3
Grand Total	2	3	0	5	3	1	0	4	0	4	0	4	13
Approach %	40.0	60.0	0.0		75.0	25.0	0.0		0.0	100.0	0.0		
Total %	15.4	23.1	0.0	38.5	23.1	7.7	0.0	30.8	0.0	30.8	0.0	30.8	
Exiting Leg Total	7				3				3				13

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	1	0	0	1	1	0	0	1	0	0	0	0	2
11:45 AM	0	1	0	1	1	0	0	1	0	1	0	1	3
12:00 PM	1	0	0	1	0	0	0	0	0	2	0	2	3
12:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Volume	2	1	0	3	2	1	0	3	0	3	0	3	9
% Approach Total	66.7	33.3	0.0		66.7	33.3	0.0		0.0	100.0	0.0		
PHF	0.500	0.250	0.000	0.750	0.500	0.250	0.000	0.750	0.000	0.375	0.000	0.375	0.750
Entering Leg	2	1	0	3	2	1	0	3	0	3	0	3	9
Exiting Leg	5				1				3				9
Total	8				4				6				18

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



46 Morton Street, Framingham, MA 01702
 Office: 508-875-0100 Fax: 508-875-0118
 Email: datarequests@pdillc.com

Articulated Trucks

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Exiting Leg Total	0				0				0				0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0
% Approach Total	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0
Exiting Leg				0				0				0	0
Total				0				0				0	

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**



Bicycles (on Roadway and Crosswalks)

	Mill Road						Mill Road Plaza Driveway						Mill Road						Total	
	from North						from East						from South							
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total		
11:30 AM	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	1	1	0	1	0	3	1	0	0	0	0	0	1	1	1	0	0	0	2	6
Total	2	1	0	1	0	4	1	0	0	0	0	1	1	1	0	0	0	2	7	
12:00 PM	0	0	0	0	1	1	0	0	0	0	0	0	0	1	0	0	0	0	1	2
12:15 PM	2	0	0	0	0	2	0	0	0	0	0	0	1	0	0	0	0	0	1	3
12:30 PM	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	1	1
12:45 PM	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4	4
Total	6	0	0	0	1	7	1	0	0	0	0	1	1	1	0	0	0	2	10	
1:00 PM	3	0	0	0	0	3	0	0	0	0	0	0	0	2	0	0	0	2	5	5
1:15 PM	1	0	0	0	0	1	1	0	0	0	0	1	1	1	0	0	0	2	4	4
Total	4	0	0	0	0	4	1	0	0	0	0	1	1	3	0	0	0	4	9	9
Grand Total	12	1	0	1	1	15	3	0	0	0	0	3	3	5	0	0	0	8	26	26
Approach %	80.0	6.7	0.0	6.7	6.7		100.0	0.0	0.0	0.0	0.0		37.5	62.5	0.0	0.0	0.0			
Total %	46.2	3.8	0.0	3.8	3.8	57.7	11.5	0.0	0.0	0.0	0.0	11.5	11.5	19.2	0.0	0.0	0.0	30.8		
Exiting Leg Total	10						4						12						26	

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Mill Road						Mill Road Plaza Driveway						Mill Road						Total	
	from North						from East						from South							
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total		
12:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	0	1
12:45 PM	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	4
1:00 PM	3	0	0	0	0	3	0	0	0	0	0	0	0	0	2	0	0	0	2	5
1:15 PM	1	0	0	0	0	1	1	0	0	0	0	0	1	1	1	0	0	0	2	4
Total Volume	8	0	0	0	0	8	2	0	0	0	0	0	2	1	3	0	0	0	4	14
% Approach Total	100.0	0.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0	0.0		25.0	75.0	0.0	0.0	0.0			
PHF	0.500	0.000	0.000	0.000	0.000	0.500	0.500	0.000	0.000	0.000	0.000	0.500	0.250	0.375	0.000	0.000	0.000	0.500		0.700
Entering Leg	8	0	0	0	0	8	2	0	0	0	0	2	1	3	0	0	0	4		14
Exiting Leg	5						1						8							14
Total	13						3						12							28

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Saturday, May 05, 2018**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Pedestrians

	Mill Road						Mill Road Plaza Driveway						Mill Road						Total
	from North						from East						from South						
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total	
11:30 AM	0	0	0	17	6	23	0	0	0	2	2	4	0	0	0	0	0	0	27
11:45 AM	0	0	0	16	12	28	0	0	0	32	6	38	0	0	0	0	0	0	66
Total	0	0	0	33	18	51	0	0	0	34	8	42	0	0	0	0	0	0	93
12:00 PM	0	0	0	21	3	24	0	0	0	8	7	15	0	0	0	0	0	0	39
12:15 PM	0	0	0	57	6	63	0	0	0	1	24	25	0	0	0	0	0	0	88
12:30 PM	0	0	0	21	6	27	0	0	0	18	8	26	0	0	0	0	0	0	53
12:45 PM	0	0	0	11	19	30	0	0	0	3	11	14	0	0	0	0	1	1	45
Total	0	0	0	110	34	144	0	0	0	30	50	80	0	0	0	0	1	1	225
1:00 PM	0	0	0	20	6	26	0	0	0	16	23	39	0	0	0	0	8	8	73
1:15 PM	0	0	0	7	3	10	0	0	0	67	25	92	0	0	0	0	0	0	102
Total	0	0	0	27	9	36	0	0	0	83	48	131	0	0	0	0	8	8	175
Grand Total	0	0	0	170	61	231	0	0	0	147	106	253	0	0	0	0	9	9	493
Approach %	0	0	0	73.593	26.407		0	0	0	58.103	41.897		0	0	0	0	100		
Total %	0	0	0	34.483	12.373	46.856	0	0	0	29.817	21.501	51.318	0	0	0	0	1.8256	1.8256	
Exiting Leg Total	231						253						9						493

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Mill Road						Mill Road Plaza Driveway						Mill Road						Total
	from North						from East						from South						
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total	
12:30 PM	0	0	0	21	6	27	0	0	0	18	8	26	0	0	0	0	0	0	53
12:45 PM	0	0	0	11	19	30	0	0	0	3	11	14	0	0	0	0	1	1	45
1:00 PM	0	0	0	20	6	26	0	0	0	16	23	39	0	0	0	0	8	8	73
1:15 PM	0	0	0	7	3	10	0	0	0	67	25	92	0	0	0	0	0	0	102
Total Volume	0	0	0	59	34	93	0	0	0	104	67	171	0	0	0	0	9	9	273
% Approach Total	0.0	0.0	0.0	63.4	36.6		0.0	0.0	0.0	60.8	39.2		0.0	0.0	0.0	0.0	100.0		
PHF	0.000	0.000	0.000	0.702	0.447	0.775	0.000	0.000	0.000	0.388	0.670	0.465	0.000	0.000	0.000	0.000	0.281	0.281	0.669
Entering Leg	0	0	0	59	34	93	0	0	0	104	67	171	0	0	0	0	9	9	273
Exiting Leg	93						171						9						273
Total	186						342						18						546

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	40	36	0	76	32	12	0	44	24	58	0	82	202
4:15 PM	40	35	0	75	34	27	0	61	19	57	0	76	212
4:30 PM	42	45	0	87	45	10	0	55	19	77	0	96	238
4:45 PM	44	44	0	88	49	16	0	65	30	82	0	112	265
Total	166	160	0	326	160	65	0	225	92	274	0	366	917
5:00 PM	53	35	0	88	45	18	0	63	20	83	0	103	254
5:15 PM	50	40	0	90	36	20	0	56	21	71	0	92	238
5:30 PM	42	36	0	78	43	20	0	63	21	49	0	70	211
5:45 PM	47	38	0	85	51	10	0	61	14	64	0	78	224
Total	192	149	0	341	175	68	0	243	76	267	0	343	927
Grand Total	358	309	0	667	335	133	0	468	168	541	0	709	1844
Approach %	53.7	46.3	0.0		71.6	28.4	0.0		23.7	76.3	0.0		
Total %	19.4	16.8	0.0	36.2	18.2	7.2	0.0	25.4	9.1	29.3	0.0	38.4	
Exiting Leg Total	876				477				491				1844
Cars	350	307	0	657	333	133	0	466	168	541	0	709	1832
% Cars	97.8	99.4	0.0	98.5	99.4	100.0	0.0	99.6	100.0	100.0	0.0	100.0	99.3
Exiting Leg Total	874				475				483				1832
Heavy Vehicles	8	2	0	10	2	0	0	2	0	0	0	0	12
% Heavy Vehicles	2.2	0.6	0.0	1.5	0.6	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.7
Exiting Leg Total	2				2				8				12

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:30 PM	42	45	0	87	45	10	0	55	19	77	0	96	238
4:45 PM	44	44	0	88	49	16	0	65	30	82	0	112	265
5:00 PM	53	35	0	88	45	18	0	63	20	83	0	103	254
5:15 PM	50	40	0	90	36	20	0	56	21	71	0	92	238
Total Volume	189	164	0	353	175	64	0	239	90	313	0	403	995
% Approach Total	53.5	46.5	0.0		73.2	26.8	0.0		22.3	77.7	0.0		
PHF	0.892	0.911	0.000	0.981	0.893	0.800	0.000	0.919	0.750	0.943	0.000	0.900	0.939
Cars	183	163	0	346	174	64	0	238	90	313	0	403	987
Cars %	96.8	99.4	0.0	98.0	99.4	100.0	0.0	99.6	100.0	100.0	0.0	100.0	99.2
Heavy Vehicles	6	1	0	7	1	0	0	1	0	0	0	0	8
Heavy Vehicles %	3.2	0.6	0.0	2.0	0.6	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.8
Cars Enter Leg	183	163	0	346	174	64	0	238	90	313	0	403	987
Heavy Enter Leg	6	1	0	7	1	0	0	1	0	0	0	0	8
Total Entering Leg	189	164	0	353	175	64	0	239	90	313	0	403	995
Cars Exiting Leg				487				253				247	987
Heavy Exiting Leg				1				1				6	8
Total Exiting Leg				488				254				253	995

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**



Class: **Cars-Combined (Motorcycles, Cars, Light Goods)**

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	40	36	0	76	32	12	0	44	24	58	0	82	202
4:15 PM	39	35	0	74	34	27	0	61	19	57	0	76	211
4:30 PM	39	44	0	83	45	10	0	55	19	77	0	96	234
4:45 PM	43	44	0	87	48	16	0	64	30	82	0	112	263
Total	161	159	0	320	159	65	0	224	92	274	0	366	910
5:00 PM	53	35	0	88	45	18	0	63	20	83	0	103	254
5:15 PM	48	40	0	88	36	20	0	56	21	71	0	92	236
5:30 PM	41	35	0	76	43	20	0	63	21	49	0	70	209
5:45 PM	47	38	0	85	50	10	0	60	14	64	0	78	223
Total	189	148	0	337	174	68	0	242	76	267	0	343	922
Grand Total	350	307	0	657	333	133	0	466	168	541	0	709	1832
Approach %	53.3	46.7	0.0		71.5	28.5	0.0		23.7	76.3	0.0		
Total %	19.1	16.8	0.0	35.9	18.2	7.3	0.0	25.4	9.2	29.5	0.0	38.7	
Exiting Leg Total	874				475				483				1832

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:30 PM	39	44	0	83	45	10	0	55	19	77	0	96	234
4:45 PM	43	44	0	87	48	16	0	64	30	82	0	112	263
5:00 PM	53	35	0	88	45	18	0	63	20	83	0	103	254
5:15 PM	48	40	0	88	36	20	0	56	21	71	0	92	236
Total Volume	183	163	0	346	174	64	0	238	90	313	0	403	987
% Approach Total	52.9	47.1	0.0		73.1	26.9	0.0		22.3	77.7	0.0		
PHF	0.863	0.926	0.000	0.983	0.906	0.800	0.000	0.930	0.750	0.943	0.000	0.900	0.938
Entering Leg	183	163	0	346	174	64	0	238	90	313	0	403	987
Exiting Leg				487				253				247	987
Total				833				491				650	1974

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**



Class: **Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)**

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
4:30 PM	3	1	0	4	0	0	0	0	0	0	0	0	4
4:45 PM	1	0	0	1	1	0	0	1	0	0	0	0	2
Total	5	1	0	6	1	0	0	1	0	0	0	0	7
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	2	0	0	2	0	0	0	0	0	0	0	0	2
5:30 PM	1	1	0	2	0	0	0	0	0	0	0	0	2
5:45 PM	0	0	0	0	1	0	0	1	0	0	0	0	1
Total	3	1	0	4	1	0	0	1	0	0	0	0	5
Grand Total	8	2	0	10	2	0	0	2	0	0	0	0	12
Approach %	80.0	20.0	0.0		100.0	0.0	0.0		0.0	0.0	0.0		
Total %	66.7	16.7	0.0	83.3	16.7	0.0	0.0	16.7	0.0	0.0	0.0	0.0	
Exiting Leg Total	2				2				8				12
Buses	7	1	0	8	1	0	0	1	0	0	0	0	9
% Buses	87.5	50.0	0.0	80.0	50.0	0.0	0.0	50.0	0.0	0.0	0.0	0.0	75.0
Exiting Leg Total	1				1				7				9
Single-Unit Trucks	1	1	0	2	1	0	0	1	0	0	0	0	3
% Single-Unit	12.5	50.0	0.0	20.0	50.0	0.0	0.0	50.0	0.0	0.0	0.0	0.0	25.0
Exiting Leg Total	1				1				1				3
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Exiting Leg Total	0				0				0				0

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:30 PM	3	1	0	4	0	0	0	0	0	0	0	0	4
4:45 PM	1	0	0	1	1	0	0	1	0	0	0	0	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	2	0	0	2	0	0	0	0	0	0	0	0	2
Total Volume	6	1	0	7	1	0	0	1	0	0	0	0	8
% Approach Total	85.7	14.3	0.0		100.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.500	0.250	0.000	0.438	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.500
Buses	5	0	0	5	0	0	0	0	0	0	0	0	5
Buses %	83.3	0.0	0.0	71.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	62.5
Single-Unit Trucks	1	1	0	2	1	0	0	1	0	0	0	0	3
Single-Unit %	16.7	100.0	0.0	28.6	100.0	0.0	0.0	100.0	0.0	0.0	0.0	0.0	37.5
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Buses	5	0	0	5	0	0	0	0	0	0	0	0	5
Single-Unit Trucks	1	1	0	2	1	0	0	1	0	0	0	0	3
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Entering Leg	6	1	0	7	1	0	0	1	0	0	0	0	8
Buses				0				0				5	5
Single-Unit Trucks				1				1				1	3
Articulated Trucks				0				0				0	0
Total Exiting Leg				1				1				6	8

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	38	34	0	72	27	10	0	37	20	49	0	69	178
4:15 PM	33	32	0	65	34	23	0	57	19	49	0	68	190
4:30 PM	34	41	0	75	42	10	0	52	17	71	0	88	215
4:45 PM	36	37	0	73	43	12	0	55	26	78	0	104	232
Total	141	144	0	285	146	55	0	201	82	247	0	329	815
5:00 PM	49	32	0	81	37	17	0	54	18	75	0	93	228
5:15 PM	41	38	0	79	30	18	0	48	20	67	0	87	214
5:30 PM	37	30	0	67	40	16	0	56	17	41	0	58	181
5:45 PM	44	36	0	80	45	10	0	55	12	53	0	65	200
Total	171	136	0	307	152	61	0	213	67	236	0	303	823
Grand Total	312	280	0	592	298	116	0	414	149	483	0	632	1638
Approach %	52.7	47.3	0.0		72.0	28.0	0.0		23.6	76.4	0.0		
Total %	19.0	17.1	0.0	36.1	18.2	7.1	0.0	25.3	9.1	29.5	0.0	38.6	
Exiting Leg Total	781				429				428				1638

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:30 PM	34	41	0	75	42	10	0	52	17	71	0	88	215
4:45 PM	36	37	0	73	43	12	0	55	26	78	0	104	232
5:00 PM	49	32	0	81	37	17	0	54	18	75	0	93	228
5:15 PM	41	38	0	79	30	18	0	48	20	67	0	87	214
Total Volume	160	148	0	308	152	57	0	209	81	291	0	372	889
% Approach Total	51.9	48.1	0.0		72.7	27.3	0.0		21.8	78.2	0.0		
PHF	0.816	0.902	0.000	0.951	0.884	0.792	0.000	0.950	0.779	0.933	0.000	0.894	0.958
Entering Leg	160	148	0	308	152	57	0	209	81	291	0	372	889
Exiting Leg				443				229				217	889
Total				751				438				589	1778

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Light Goods Vehicle

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	1	1	0	2	4	2	0	6	4	7	0	11	19
4:15 PM	4	3	0	7	0	4	0	4	0	8	0	8	19
4:30 PM	5	3	0	8	3	0	0	3	1	6	0	7	18
4:45 PM	6	4	0	10	5	4	0	9	4	2	0	6	25
Total	16	11	0	27	12	10	0	22	9	23	0	32	81
5:00 PM	3	2	0	5	6	0	0	6	2	6	0	8	19
5:15 PM	4	1	0	5	5	2	0	7	1	3	0	4	16
5:30 PM	3	3	0	6	3	4	0	7	4	6	0	10	23
5:45 PM	2	2	0	4	2	0	0	2	2	7	0	9	15
Total	12	8	0	20	16	6	0	22	9	22	0	31	73
Grand Total	28	19	0	47	28	16	0	44	18	45	0	63	154
Approach %	59.6	40.4	0.0		63.6	36.4	0.0		28.6	71.4	0.0		
Total %	18.2	12.3	0.0	30.5	18.2	10.4	0.0	28.6	11.7	29.2	0.0	40.9	
Exiting Leg Total	73				37				44				154

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:45 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:45 PM	6	4	0	10	5	4	0	9	4	2	0	6	25
5:00 PM	3	2	0	5	6	0	0	6	2	6	0	8	19
5:15 PM	4	1	0	5	5	2	0	7	1	3	0	4	16
5:30 PM	3	3	0	6	3	4	0	7	4	6	0	10	23
Total Volume	16	10	0	26	19	10	0	29	11	17	0	28	83
% Approach Total	61.5	38.5	0.0		65.5	34.5	0.0		39.3	60.7	0.0		
PHF	0.667	0.625	0.000	0.650	0.792	0.625	0.000	0.806	0.688	0.708	0.000	0.700	0.830
Entering Leg	16	10	0	26	19	10	0	29	11	17	0	28	83
Exiting Leg				36				21				26	83
Total				62				50				54	166

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Buses

	Mill Road					Mill Road Plaza Driveway					Mill Road					Total
	from North					from East					from South					
	Thru	Left	U-Turn	Total		Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total		
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	1	0	0	1		0	0	0	0	0	0	0	0	0	0	1
4:30 PM	3	0	0	3		0	0	0	0	0	0	0	0	0	0	3
4:45 PM	1	0	0	1		0	0	0	0	0	0	0	0	0	0	1
Total	5	0	0	5		0	0	0	0	0	0	0	0	0	0	5
5:00 PM	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0
5:15 PM	1	0	0	1		0	0	0	0	0	0	0	0	0	0	1
5:30 PM	1	1	0	2		0	0	0	0	0	0	0	0	0	0	2
5:45 PM	0	0	0	0		1	0	0	1	0	0	0	0	0	0	1
Total	2	1	0	3		1	0	0	1	0	0	0	0	0	0	4
Grand Total	7	1	0	8		1	0	0	1	0	0	0	0	0	0	9
Approach %	87.5	12.5	0.0			100.0	0.0	0.0		0.0	0.0	0.0				
Total %	77.8	11.1	0.0	88.9		11.1	0.0	0.0	11.1	0.0	0.0	0.0	0.0			
Exiting Leg Total	1					1					7					9

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
4:30 PM	3	0	0	3	0	0	0	0	0	0	0	0	3
4:45 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
Total Volume	5	0	0	5	0	0	0	0	0	0	0	0	5
% Approach Total	100.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.417	0.000	0.000	0.417	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.417
Entering Leg	5	0	0	5	0	0	0	0	0	0	0	0	5
Exiting Leg				0								5	5
Total				5				0				5	10

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Single-Unit Trucks

	Mill Road					Mill Road Plaza Driveway					Mill Road					Total
	from North					from East					from South					
	Thru	Left	U-Turn	Total		Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total		
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1
Total	0	1	0	1	1	1	0	0	1	0	0	0	0	0	0	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
Grand Total	1	1	0	2	1	0	0	1	0	0	0	0	0	0	0	3
Approach %	50.0	50.0	0.0		100.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0		
Total %	33.3	33.3	0.0	66.7	33.3	0.0	0.0	33.3	0.0	0.0	0.0	0.0		0.0		
Exiting Leg Total	1					1					1					3

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:30 PM	0	1	0	1	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	1	0	0	1	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
Total Volume	1	1	0	2	1	0	0	1	0	0	0	0	3
% Approach Total	50.0	50.0	0.0		100.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.250	0.250	0.000	0.500	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.750
Entering Leg	1	1	0	2	1	0	0	1	0	0	0	0	3
Exiting Leg				1				1				1	3
Total				3				2				1	6

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Articulated Trucks

	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Exiting Leg Total	0				0				0				0

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Mill Road				Mill Road Plaza Driveway				Mill Road				Total
	from North				from East				from South				
	Thru	Left	U-Turn	Total	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0
% Approach Total	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0
Exiting Leg				0				0				0	0
Total				0				0				0	0

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**



Bicycles (on Roadway and Crosswalks)

	Mill Road						Mill Road Plaza Driveway						Mill Road						Total
	from North						from East						from South						
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	1	0	0	0	3	4	1	0	0	0	0	1	0	1	0	0	0	1	6
4:30 PM	1	0	0	0	0	1	0	0	0	0	0	0	1	1	0	0	0	2	3
4:45 PM	0	0	0	0	0	0	1	0	0	0	1	2	0	2	0	0	0	2	4
Total	2	0	0	0	3	5	2	0	0	0	1	3	1	4	0	0	0	5	13
5:00 PM	0	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	1	2
5:15 PM	1	0	0	0	0	1	0	2	0	0	0	2	0	1	0	0	0	1	4
5:30 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Total	2	1	0	1	0	4	0	2	0	0	0	2	0	2	0	0	0	2	8
Grand Total	4	1	0	1	3	9	2	2	0	0	1	5	1	6	0	0	0	7	21
Approach %	44.4	11.1	0.0	11.1	33.3		40.0	40.0	0.0	0.0	20.0		14.3	85.7	0.0	0.0	0.0		
Total %	19.0	4.8	0.0	4.8	14.3	42.9	9.5	9.5	0.0	0.0	4.8	23.8	4.8	28.6	0.0	0.0	0.0	33.3	
Exiting Leg Total	12						3						6						21

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:15 PM	Mill Road						Mill Road Plaza Driveway						Mill Road						Total
	from North						from East						from South						
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total	
4:15 PM	1	0	0	0	3	4	1	0	0	0	0	1	0	1	0	0	0	1	6
4:30 PM	1	0	0	0	0	1	0	0	0	0	0	0	1	1	0	0	0	2	3
4:45 PM	0	0	0	0	0	0	1	0	0	0	1	2	0	2	0	0	0	2	4
5:00 PM	0	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	1	2
Total Volume	2	0	0	1	3	6	2	0	0	0	1	3	1	5	0	0	0	6	15
% Approach Total	33.3	0.0	0.0	16.7	50.0		66.7	0.0	0.0	0.0	33.3		16.7	83.3	0.0	0.0	0.0		
PHF	0.500	0.000	0.000	0.250	0.250	0.375	0.500	0.000	0.000	0.000	0.250	0.375	0.250	0.625	0.000	0.000	0.000	0.750	0.625
Entering Leg	2	0	0	1	3	6	2	0	0	0	1	3	1	5	0	0	0	6	15
Exiting Leg	11						2						2						15
Total	17						5						8						30

PDI File #: **186243 C**
 Location: **N: Mill Road S: Mill Road**
 Location: **E: Mill Road Plaza Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529-002**
 Count Date: **Tuesday, May 08, 2018**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Pedestrians

	Mill Road						Mill Road Plaza Driveway						Mill Road						Total
	from North						from East						from South						
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total	
4:00 PM	0	0	0	11	2	13	0	0	0	3	13	16	0	0	0	0	0	0	29
4:15 PM	0	0	0	9	1	10	0	0	0	0	6	6	0	0	0	0	0	0	16
4:30 PM	0	0	0	8	7	15	0	0	0	2	40	42	0	0	0	0	1	1	58
4:45 PM	0	0	0	5	5	10	0	0	0	3	5	8	0	0	0	0	0	0	18
Total	0	0	0	33	15	48	0	0	0	8	64	72	0	0	0	0	1	1	121
5:00 PM	0	0	0	4	5	9	1	0	0	3	21	25	0	0	0	0	0	0	34
5:15 PM	0	0	0	6	4	10	0	0	0	4	9	13	0	0	0	0	0	0	23
5:30 PM	0	0	0	14	4	18	0	0	0	1	1	2	0	0	0	0	0	0	20
5:45 PM	0	0	0	4	5	9	0	0	0	3	4	7	0	0	0	1	0	1	17
Total	0	0	0	28	18	46	1	0	0	11	35	47	0	0	0	1	0	1	94
Grand Total	0	0	0	61	33	94	1	0	0	19	99	119	0	0	0	1	1	2	215
Approach %	0	0	0	64.894	35.106		0.8403	0	0	15.966	83.193		0	0	0	50	50		
Total %	0	0	0	28.372	15.349	43.721	0.4651	0	0	8.8372	46.047	55.349	0	0	0	0.4651	0.4651	0.9302	
Exiting Leg Total	95						118						2						215

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Mill Road						Mill Road Plaza Driveway						Mill Road						Total	
	from North						from East						from South							
	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	U-Turn	CW-WB	CW-EB	Total		
4:30 PM	0	0	0	8	7	15	0	0	0	2	40	42	0	0	0	0	1	1	58	
4:45 PM	0	0	0	5	5	10	0	0	0	3	5	8	0	0	0	0	0	0	18	
5:00 PM	0	0	0	4	5	9	1	0	0	3	21	25	0	0	0	0	0	0	34	
5:15 PM	0	0	0	6	4	10	0	0	0	4	9	13	0	0	0	0	0	0	23	
Total Volume	0	0	0	23	21	44	1	0	0	12	75	88	0	0	0	0	1	1	133	
% Approach Total	0.0	0.0	0.0	52.3	47.7		1.1	0.0	0.0	13.6	85.2		0.0	0.0	0.0	0.0	100.0			
PHF	0.000	0.000	0.000	0.719	0.750	0.733	0.250	0.000	0.000	0.750	0.469	0.524	0.000	0.000	0.000	0.000	0.250	0.250	0.573	
Entering Leg	0	0	0	23	21	44	1	0	0	12	75	88	0	0	0	0	1	1	133	
Exiting Leg							45												1	133
Total	89						175						2						266	



46 Morton Street, Framingham, MA 01702
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Groups Printed- Pedestrians - Bicycles

	Mill Road From North		Mid Block Crosswalk From East		Mill Road From South		Mid Block Crosswalk From West		
Start Time	EB	WB	EB	WB	EB	WB	EB	Peds	Int. Total
11:30 AM	0	0	0	19	0	0	34	0	53
11:45 AM	0	0	0	26	0	0	14	0	40
Total	0	0	0	45	0	0	48	0	93
12:00 PM	0	0	0	7	0	0	34	0	41
12:15 PM	0	0	0	14	0	0	33	0	47
12:30 PM	0	0	0	10	0	0	35	0	45
12:45 PM	0	0	0	20	0	0	33	0	53
Total	0	0	0	51	0	0	135	0	186
01:00 PM	0	0	0	15	0	0	22	0	37
01:15 PM	0	0	0	22	0	0	24	0	46
Grand Total	0	0	0	133	0	0	229	0	362
Apprch %	0	0	0	100	0	0	100	0	
Total %	0	0	0	36.7	0	0	63.3	0	
Pedestrians	0	0	0	133	0	0	229	0	362
% Pedestrians	0	0	0	100	0	0	100	0	100
Bicycles	0	0	0	0	0	0	0	0	0
% Bicycles	0	0	0	0	0	0	0	0	0

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PRECISION
D A T A
INDUSTRIES, LLC

46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
Email: datarequests@pdillc.com

Mill Road
south of Main St (Mid Block Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 B Pathway
Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

Groups Printed- Pedestrians

	Mill Road From North		Mid Block Crosswalk From East		Mill Road From South		Mid Block Crosswalk From West		Int. Total
Start Time	EB	WB	EB	WB	EB	WB	EB	WB	
11:30 AM	0	0	0	19	0	0	34	0	53
11:45 AM	0	0	0	26	0	0	14	0	40
Total	0	0	0	45	0	0	48	0	93
12:00 PM	0	0	0	7	0	0	34	0	41
12:15 PM	0	0	0	14	0	0	33	0	47
12:30 PM	0	0	0	10	0	0	35	0	45
12:45 PM	0	0	0	20	0	0	33	0	53
Total	0	0	0	51	0	0	135	0	186
01:00 PM	0	0	0	15	0	0	22	0	37
01:15 PM	0	0	0	22	0	0	24	0	46
Grand Total	0	0	0	133	0	0	229	0	362
Apprch %	0	0	0	100	0	0	100	0	
Total %	0	0	0	36.7	0	0	63.3	0	

	Mill Road From North			Mid Block Crosswalk From East			Mill Road From South			Mid Block Crosswalk From West			Int. Total
Start Time	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	
Peak Hour Analysis From 11:30 AM to 01:15 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 12:00 PM													
12:00 PM	0	0	0	0	7	7	0	0	0	34	0	34	41
12:15 PM	0	0	0	0	14	14	0	0	0	33	0	33	47
12:30 PM	0	0	0	0	10	10	0	0	0	35	0	35	45
12:45 PM	0	0	0	0	20	20	0	0	0	33	0	33	53
Total Volume	0	0	0	0	51	51	0	0	0	135	0	135	186
% App. Total	0	0		0	100		0	0		100	0		
PHF	.000	.000	.000	.000	.638	.638	.000	.000	.000	.964	.000	.964	.877



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File Name : 186243 B Pathway
Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

Groups Printed- Bicycles

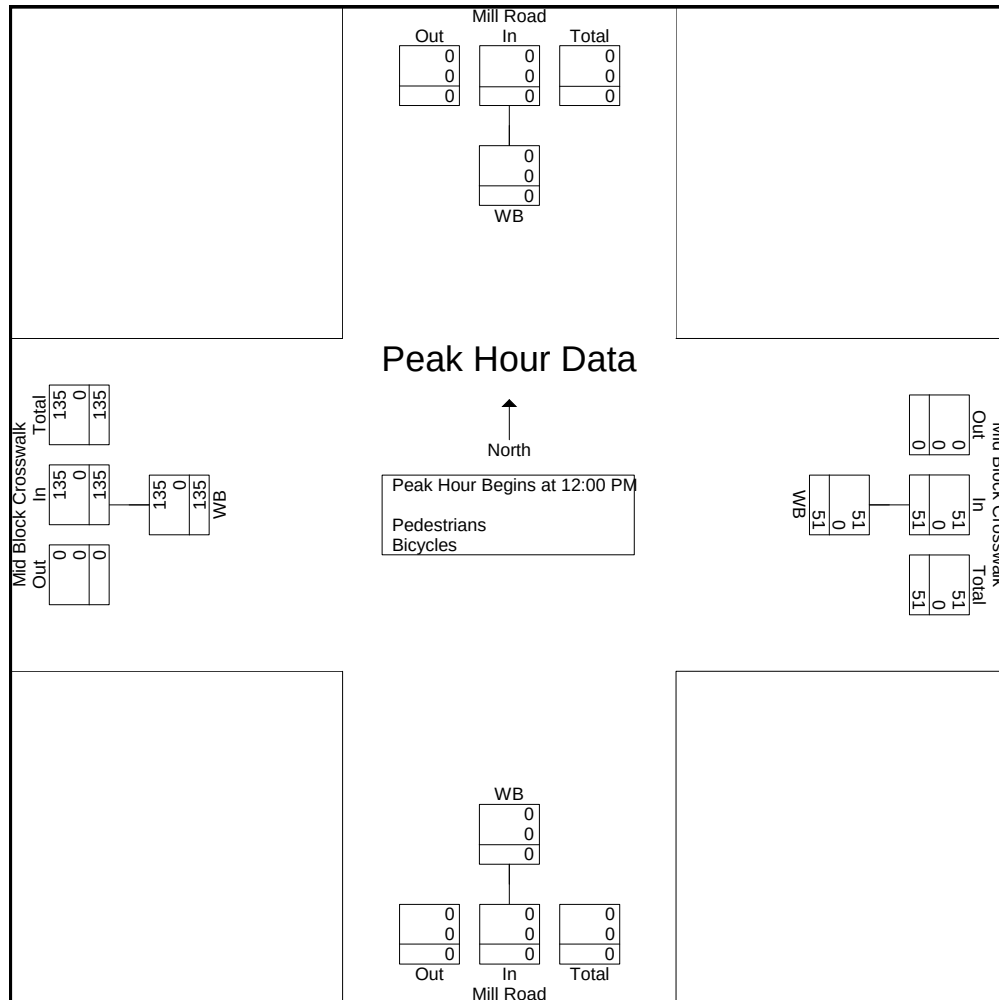
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Peak Hour Analysis From 11:30 AM to 01:15 PM - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 12:00 PM





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Office: 508-875-0100 Fax: 508-875-0118
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Mill Road
south of Main St (Mid Block Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 BBB Pathway
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Pedestrians - Bicycles

	Mill Road From North		Mid Block Crosswalk From East		Mill Road From South		Mid Block Crosswalk From West		Int. Total
Start Time	EB	WB	EB	WB	EB	WB	EB	WB	
04:00 PM	0	0	0	9	0	0	14	0	23
04:15 PM	0	0	0	25	0	0	24	0	49
04:30 PM	0	0	0	41	0	0	19	0	60
04:45 PM	0	0	0	18	0	0	20	0	38
Total	0	0	0	93	0	0	77	0	170
05:00 PM	0	0	0	31	0	0	13	0	44
05:15 PM	0	0	0	9	0	0	19	0	28
05:30 PM	0	0	0	19	0	0	32	0	51
05:45 PM	0	0	0	20	0	0	18	0	38
Total	0	0	0	79	0	0	82	0	161
Grand Total	0	0	0	172	0	0	159	0	331
Apprch %	0	0	0	100	0	0	100	0	
Total %	0	0	0	52	0	0	48	0	
Pedestrians	0	0	0	171	0	0	157	0	328
% Pedestrians	0	0	0	99.4	0	0	98.7	0	99.1
Bicycles	0	0	0	1	0	0	2	0	3
% Bicycles	0	0	0	0.6	0	0	1.3	0	0.9

	Mill Road From North			Mid Block Crosswalk From East			Mill Road From South			Mid Block Crosswalk From West			Int. Total
Start Time	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:15 PM													
04:15 PM	0	0	0	0	25	25	0	0	0	24	0	24	49
04:30 PM	0	0	0	0	41	41	0	0	0	19	0	19	60
04:45 PM	0	0	0	0	18	18	0	0	0	20	0	20	38
05:00 PM	0	0	0	0	31	31	0	0	0	13	0	13	44
Total Volume	0	0	0	0	115	115	0	0	0	76	0	76	191
% App. Total	0	0	0	0	100	100	0	0	0	100	0	100	
PHF	.000	.000	.000	.000	.701	.701	.000	.000	.000	.792	.000	.792	.796
Pedestrians	0	0	0	0	115	115	0	0	0	75	0	75	190
% Pedestrians	0	0	0	0	100	100	0	0	0	98.7	0	98.7	99.5
Bicycles	0	0	0	0	0	0	0	0	0	1	0	1	1
% Bicycles	0	0	0	0	0	0	0	0	0	1.3	0	1.3	0.5



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Mill Road
south of Main St (Mid Block Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 BBB Pathway
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Pedestrians

	Mill Road From North		Mid Block Crosswalk From East		Mill Road From South		Mid Block Crosswalk From West		Int. Total
Start Time	EB	WB	EB	WB	EB	WB	EB	WB	
04:00 PM	0	0	0	9	0	0	14	0	23
04:15 PM	0	0	0	25	0	0	24	0	49
04:30 PM	0	0	0	41	0	0	18	0	59
04:45 PM	0	0	0	18	0	0	20	0	38
Total	0	0	0	93	0	0	76	0	169
05:00 PM	0	0	0	31	0	0	13	0	44
05:15 PM	0	0	0	9	0	0	19	0	28
05:30 PM	0	0	0	19	0	0	31	0	50
05:45 PM	0	0	0	19	0	0	18	0	37
Total	0	0	0	78	0	0	81	0	159
Grand Total	0	0	0	171	0	0	157	0	328
Apprch %	0	0	0	100	0	0	100	0	
Total %	0	0	0	52.1	0	0	47.9	0	

	Mill Road From North			Mid Block Crosswalk From East			Mill Road From South			Mid Block Crosswalk From West			Int. Total
Start Time	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:15 PM													
04:15 PM	0	0	0	0	25	25	0	0	0	24	0	24	49
04:30 PM	0	0	0	0	41	41	0	0	0	18	0	18	59
04:45 PM	0	0	0	0	18	18	0	0	0	20	0	20	38
05:00 PM	0	0	0	0	31	31	0	0	0	13	0	13	44
Total Volume	0	0	0	0	115	115	0	0	0	75	0	75	190
% App. Total	0	0		0	100		0	0		100	0		
PHF	.000	.000	.000	.000	.701	.701	.000	.000	.000	.781	.000	.781	.805



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Mill Road
south of Main St (Mid Block Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 BBB Pathway
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Bicycles

	Mill Road From North		Mid Block Crosswalk From East		Mill Road From South		Mid Block Crosswalk From West		Int. Total
Start Time	EB	WB	EB	WB	EB	WB	EB	WB	
04:00 PM	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	1	0	1
04:45 PM	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	1	0	1
05:00 PM	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	1	0	1
05:45 PM	0	0	0	1	0	0	0	0	1
Total	0	0	0	1	0	0	1	0	2
Grand Total	0	0	0	1	0	0	2	0	3
Apprch %	0	0	0	100	0	0	100	0	
Total %	0	0	0	33.3	0	0	66.7	0	

	Mill Road From North			Mid Block Crosswalk From East			Mill Road From South			Mid Block Crosswalk From West			Int. Total
Start Time	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 05:00 PM													
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	1	0	1	1
05:45 PM	0	0	0	0	1	1	0	0	0	0	0	0	1
Total Volume	0	0	0	0	1	1	0	0	0	1	0	1	2
% App. Total	0	0		0	100		0	0		100	0		
PHF	.000	.000	.000	.000	.250	.250	.000	.000	.000	.250	.000	.250	.500



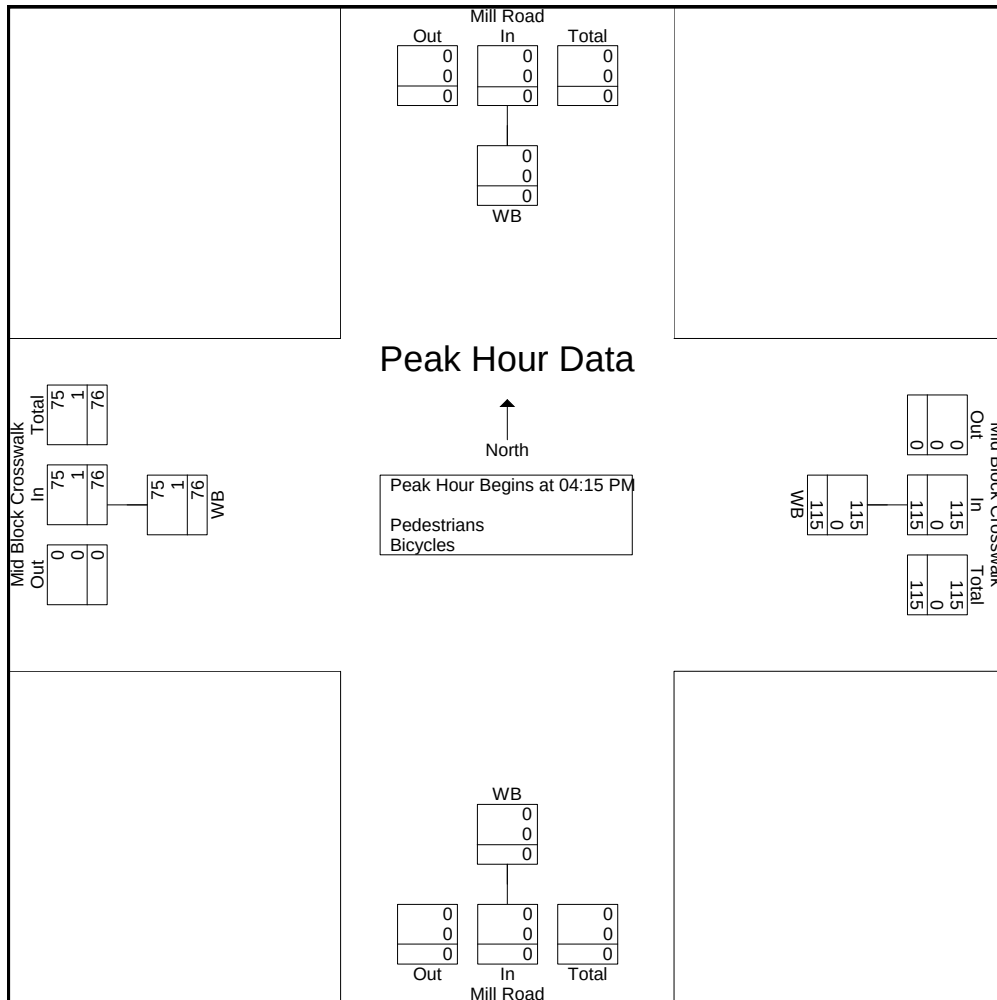
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46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
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Mill Road
south of Main St (Mid Block Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 BBB Pathway
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

	Mill Road From North			Mid Block Crosswalk From East			Mill Road From South			Mid Block Crosswalk From West			
Start Time	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	EB	WB	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:15 PM													
04:15 PM	0	0	0	0	25	25	0	0	0	24	0	24	49
04:30 PM	0	0	0	0	41	41	0	0	0	19	0	19	60
04:45 PM	0	0	0	0	18	18	0	0	0	20	0	20	38
05:00 PM	0	0	0	0	31	31	0	0	0	13	0	13	44
Total Volume	0	0	0	0	115	115	0	0	0	76	0	76	191
% App. Total	0	0		0	100		0	0		100	0		
PHF	.000	.000	.000	.000	.701	.701	.000	.000	.000	.792	.000	.792	.796
Pedestrians	0	0	0	0	115	115	0	0	0	75	0	75	190
% Pedestrians	0	0	0	0	100	100	0	0	0	98.7	0	98.7	99.5
Bicycles	0	0	0	0	0	0	0	0	0	1	0	1	1
% Bicycles	0	0	0	0	0	0	0	0	0	1.3	0	1.3	0.5





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File Name : 186243 D Pathway West
Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

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Main Street
west of Mill Rd (West Path Jay Walkers)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 D Pathway West
Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

Groups Printed- Pedestrians

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		Int. Total
Start Time	SB	NB	EB	WB	SB	NB	EB	WB	
11:30 AM	3	0	0	0	0	10	0	0	13
11:45 AM	9	0	0	0	0	7	0	0	16
Total	12	0	0	0	0	17	0	0	29
12:00 PM	17	0	0	0	0	6	0	0	23
12:15 PM	3	0	0	0	0	13	0	0	16
12:30 PM	10	0	0	0	0	9	0	0	19
12:45 PM	22	0	0	0	0	11	0	0	33
Total	52	0	0	0	0	39	0	0	91
01:00 PM	23	0	0	0	0	32	0	0	55
01:15 PM	32	0	0	0	0	17	0	0	49
Grand Total	119	0	0	0	0	105	0	0	224
Apprch %	100	0	0	0	0	100	0	0	
Total %	53.1	0	0	0	0	46.9	0	0	

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			Int. Total
Start Time	SB	NB	App. Total	EB	WB	App. Total	SB	NB	App. Total	EB	WB	App. Total	
Peak Hour Analysis From 11:30 AM to 01:15 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 12:30 PM													
12:30 PM	10	0	10	0	0	0	0	9	9	0	0	0	19
12:45 PM	22	0	22	0	0	0	0	11	11	0	0	0	33
01:00 PM	23	0	23	0	0	0	0	32	32	0	0	0	55
01:15 PM	32	0	32	0	0	0	0	17	17	0	0	0	49
Total Volume	87	0	87	0	0	0	0	69	69	0	0	0	156
% App. Total	100	0		0	0		0	100		0	0		
PHF	.680	.000	.680	.000	.000	.000	.000	.539	.539	.000	.000	.000	.709



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Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

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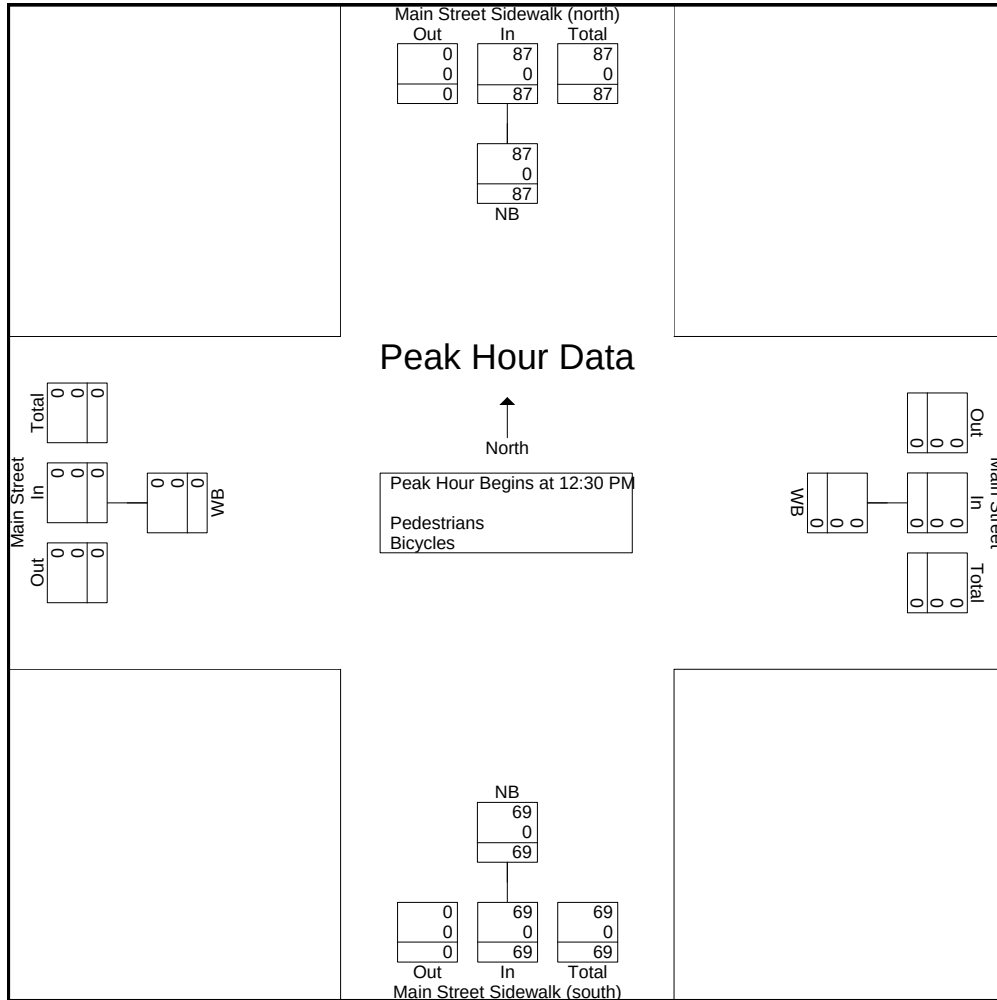
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Main Street
west of Mill Rd (West Path Jay Walkers)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 D Pathway West
Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			
Start Time	SB	NB	App. Total	EB	WB	App. Total	SB	NB	App. Total	EB	Peds	App. Total	Int. Total
Peak Hour Analysis From 11:30 AM to 01:15 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 12:30 PM													
12:30 PM	10	0	10	0	0	0	0	9	9	0	0	0	19
12:45 PM	22	0	22	0	0	0	0	11	11	0	0	0	33
01:00 PM	23	0	23	0	0	0	0	32	32	0	0	0	55
01:15 PM	32	0	32	0	0	0	0	17	17	0	0	0	49
Total Volume	87	0	87	0	0	0	0	69	69	0	0	0	156
% App. Total	100	0		0	0		0	100		0	0		
PHF	.680	.000	.680	.000	.000	.000	.000	.539	.539	.000	.000	.000	.709
Pedestrians	87	0	87	0	0	0	0	69	69	0	0	0	156
% Pedestrians	100	0	100	0	0	0	0	100	100	0	0	0	100
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0





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File Name : 186243 DDD Pathway West
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Pedestrians - Bicycles

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Main Street
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Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Pedestrians

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		Int. Total
Start Time	SB	NB	EB	WB	SB	NB	EB	WB	
04:00 PM	5	0	0	0	0	8	0	0	13
04:15 PM	8	0	0	0	0	8	0	0	16
04:30 PM	10	0	0	0	0	12	0	0	22
04:45 PM	9	0	0	0	0	8	0	0	17
Total	32	0	0	0	0	36	0	0	68
05:00 PM	6	0	0	0	0	11	0	0	17
05:15 PM	10	0	0	0	0	11	0	0	21
05:30 PM	11	0	0	0	0	13	0	0	24
05:45 PM	10	0	0	0	0	19	0	0	29
Total	37	0	0	0	0	54	0	0	91
Grand Total	69	0	0	0	0	90	0	0	159
Apprch %	100	0	0	0	0	100	0	0	
Total %	43.4	0	0	0	0	56.6	0	0	

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			Int. Total
Start Time	SB	NB	App. Total	EB	WB	App. Total	SB	NB	App. Total	EB	WB	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 05:00 PM													
05:00 PM	6	0	6	0	0	0	0	11	11	0	0	0	17
05:15 PM	10	0	10	0	0	0	0	11	11	0	0	0	21
05:30 PM	11	0	11	0	0	0	0	13	13	0	0	0	24
05:45 PM	10	0	10	0	0	0	0	19	19	0	0	0	29
Total Volume	37	0	37	0	0	0	0	54	54	0	0	0	91
% App. Total	100	0		0	0		0	100		0	0		
PHF	.841	.000	.841	.000	.000	.000	.000	.711	.711	.000	.000	.000	.784



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Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Bicycles

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		
Start Time	SB	NB	EB	WB	SB	NB	EB	Peds	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0
04:30 PM	1	0	0	0	0	0	0	0	1
04:45 PM	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	0	0	0	0	1
05:00 PM	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0
Grand Total	1	0	0	0	0	0	0	0	1
Apprch %	100	0	0	0	0	0	0	0	
Total %	100	0	0	0	0	0	0	0	

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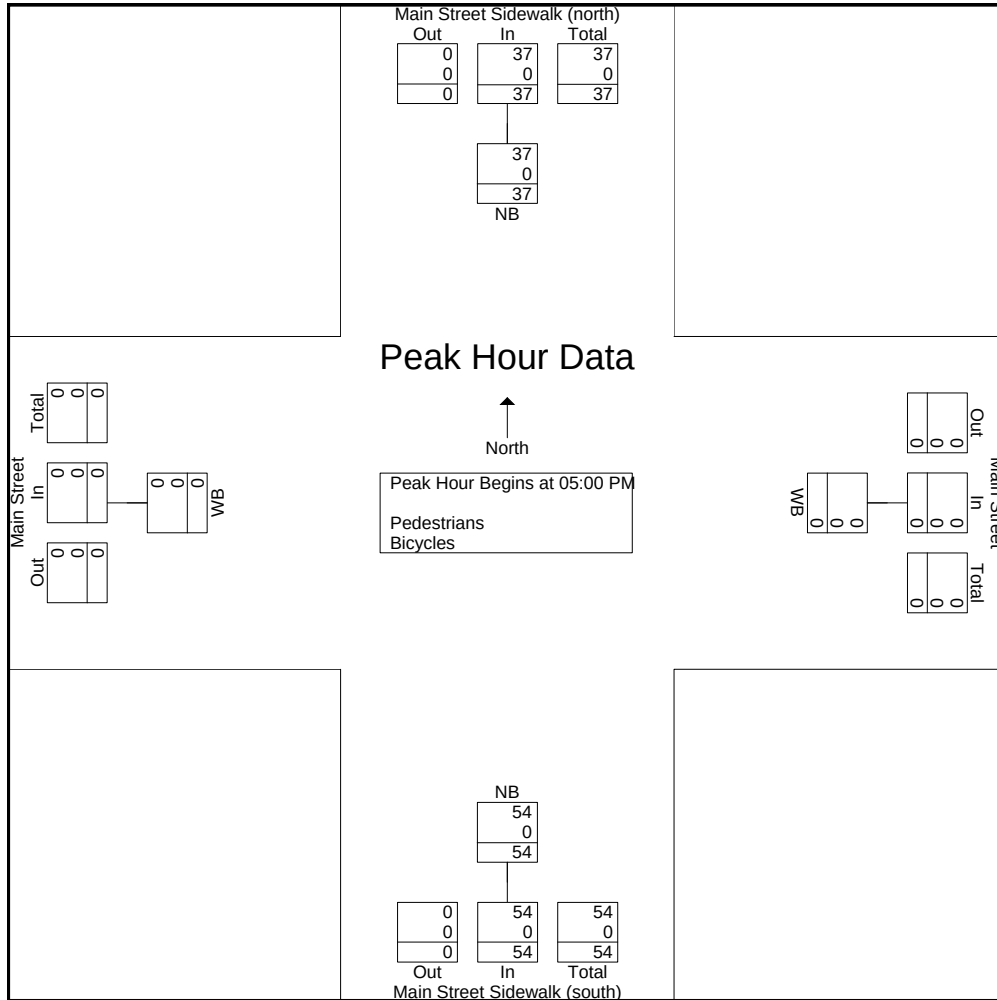
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Main Street
west of Mill Rd (West Path Jay Walkers)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 DDD Pathway West
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			
Start Time	SB	NB	App. Total	EB	WB	App. Total	SB	NB	App. Total	EB	Peds	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 05:00 PM													
05:00 PM	6	0	6	0	0	0	0	11	11	0	0	0	17
05:15 PM	10	0	10	0	0	0	0	11	11	0	0	0	21
05:30 PM	11	0	11	0	0	0	0	13	13	0	0	0	24
05:45 PM	10	0	10	0	0	0	0	19	19	0	0	0	29
Total Volume	37	0	37	0	0	0	0	54	54	0	0	0	91
% App. Total	100	0		0	0		0	100		0	0		
PHF	.841	.000	.841	.000	.000	.000	.000	.711	.711	.000	.000	.000	.784
Pedestrians	37	0	37	0	0	0	0	54	54	0	0	0	91
% Pedestrians	100	0	100	0	0	0	0	100	100	0	0	0	100
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0





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Page No : 1

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Main Street (East Path Jay Walkers &
west of Mill Rd (Madbury West Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 E Pathway
Site Code : 22-1529-
Start Date : 5/5/2018
Page No : 1

Groups Printed- Pedestrians

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		
Start Time	SB J-Walk	SB X-Walk	EB	WB	NB J-Walk	NB X-Walk	EB	WB	Int. Total
11:30 AM	0	109	0	0	0	41	0	0	150
11:45 AM	0	59	0	0	7	84	0	0	150
Total	0	168	0	0	7	125	0	0	300
12:00 PM	1	22	0	0	1	39	0	0	63
12:15 PM	0	22	0	0	0	44	0	0	66
12:30 PM	2	54	0	0	0	58	0	0	114
12:45 PM	0	47	0	0	1	31	0	0	79
Total	3	145	0	0	2	172	0	0	322
01:00 PM	6	61	0	0	0	32	0	0	99
01:15 PM	13	48	0	0	3	47	0	0	111
Grand Total	22	422	0	0	12	376	0	0	832
Apprch %	5	95	0	0	3.1	96.9	0	0	
Total %	2.6	50.7	0	0	1.4	45.2	0	0	

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			
Start Time	SB J-Walk	SB X-Walk	App. Total	EB	WB	App. Total	NB J-Walk	NB X-Walk	App. Total	EB	WB	App. Total	Int. Total
Peak Hour Analysis From 11:30 AM to 01:15 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 11:30 AM													
11:30 AM	0	109	109	0	0	0	0	41	41	0	0	0	150
11:45 AM	0	59	59	0	0	0	7	84	91	0	0	0	150
12:00 PM	1	22	23	0	0	0	1	39	40	0	0	0	63
12:15 PM	0	22	22	0	0	0	0	44	44	0	0	0	66
Total Volume	1	212	213	0	0	0	8	208	216	0	0	0	429
% App. Total	0.5	99.5		0	0		3.7	96.3		0	0		
PHF	.250	.486	.489	.000	.000	.000	.286	.619	.593	.000	.000	.000	.715



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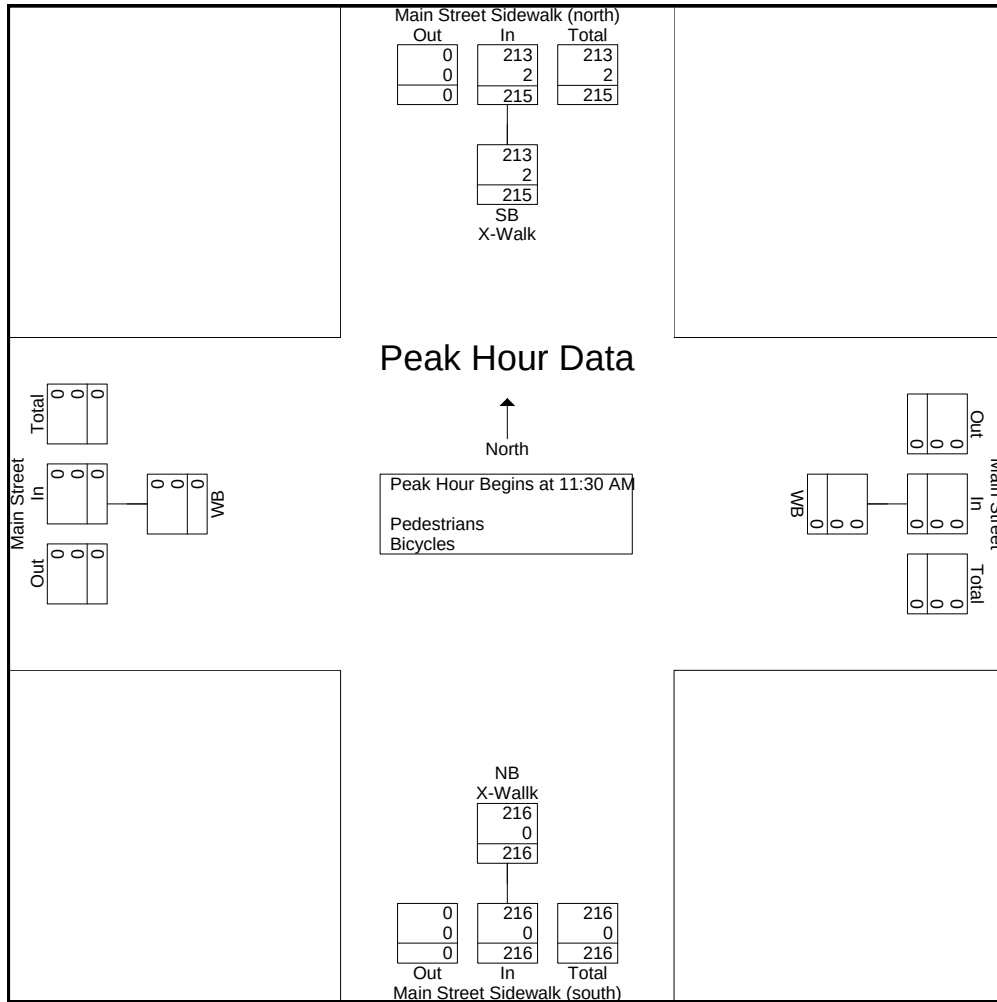
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Page No : 1

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			
Start Time	SB J-Walk	SB X-Walk	App. Total	EB	WB	App. Total	NB J-Walk	NB X-Walk	App. Total	EB	Peds	App. Total	Int. Total
Peak Hour Analysis From 11:30 AM to 01:15 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 11:30 AM													
11:30 AM	0	110	110	0	0	0	0	41	41	0	0	0	151
11:45 AM	0	59	59	0	0	0	7	84	91	0	0	0	150
12:00 PM	1	23	24	0	0	0	1	39	40	0	0	0	64
12:15 PM	0	22	22	0	0	0	0	44	44	0	0	0	66
Total Volume	1	214	215	0	0	0	8	208	216	0	0	0	431
% App. Total	0.5	99.5		0	0		3.7	96.3		0	0		
PHF	.250	.486	.489	.000	.000	.000	.286	.619	.593	.000	.000	.000	.714
Pedestrians	1	212	213	0	0	0	8	208	216	0	0	0	429
% Pedestrians	100	99.1	99.1	0	0	0	100	100	100	0	0	0	99.5
Bicycles	0	2	2	0	0	0	0	0	0	0	0	0	2
% Bicycles	0	0.9	0.9	0	0	0	0	0	0	0	0	0	0.5





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City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 EEE Pathway
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

Groups Printed- Pedestrians - Bicycles

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		Int. Total
Start Time	SB J-Walk	SB X-Walk	EB	WB	NB J-Walk	NB X-Walk	EB	Peds	
04:00 PM	0	17	0	0	0	11	0	0	28
04:15 PM	0	11	0	0	1	10	0	0	22
04:30 PM	0	14	0	0	1	11	0	0	26
04:45 PM	1	12	0	0	0	20	0	0	33
Total	1	54	0	0	2	52	0	0	109
05:00 PM	0	10	0	0	0	9	0	0	19
05:15 PM	0	8	0	0	0	6	0	0	14
05:30 PM	0	14	0	0	0	14	0	0	28
05:45 PM	0	14	0	0	4	11	0	0	29
Total	0	46	0	0	4	40	0	0	90
Grand Total	1	100	0	0	6	92	0	0	199
Apprch %	1	99	0	0	6.1	93.9	0	0	
Total %	0.5	50.3	0	0	3	46.2	0	0	
Pedestrians	1	100	0	0	5	83	0	0	189
% Pedestrians	100	100	0	0	83.3	90.2	0	0	95
Bicycles	0	0	0	0	1	9	0	0	10
% Bicycles	0	0	0	0	16.7	9.8	0	0	5

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			Int. Total
Start Time	SB J-Walk	SB X-Walk	App. Total	EB	WB	App. Total	NB J-Walk	NB X-Walk	App. Total	EB	Peds	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM													
04:00 PM	0	17	17	0	0	0	0	11	11	0	0	0	28
04:15 PM	0	11	11	0	0	0	1	10	11	0	0	0	22
04:30 PM	0	14	14	0	0	0	1	11	12	0	0	0	26
04:45 PM	1	12	13	0	0	0	0	20	20	0	0	0	33
Total Volume	1	54	55	0	0	0	2	52	54	0	0	0	109
% App. Total	1.8	98.2		0	0		3.7	96.3		0	0		
PHF	.250	.794	.809	.000	.000	.000	.500	.650	.675	.000	.000	.000	.826
Pedestrians	1	54	55	0	0	0	1	44	45	0	0	0	100
% Pedestrians	100	100	100	0	0	0	50.0	84.6	83.3	0	0	0	91.7
Bicycles	0	0	0	0	0	0	1	8	9	0	0	0	9
% Bicycles	0	0	0	0	0	0	50.0	15.4	16.7	0	0	0	8.3



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Page No : 1

Groups Printed- Pedestrians

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		
Start Time	SB J-Walk	SB X-Walk	EB	WB	NB J-Walk	NB X-Walk	EB	WB	Int. Total
04:00 PM	0	17	0	0	0	10	0	0	27
04:15 PM	0	11	0	0	1	10	0	0	22
04:30 PM	0	14	0	0	0	9	0	0	23
04:45 PM	1	12	0	0	0	15	0	0	28
Total	1	54	0	0	1	44	0	0	100
05:00 PM	0	10	0	0	0	8	0	0	18
05:15 PM	0	8	0	0	0	6	0	0	14
05:30 PM	0	14	0	0	0	14	0	0	28
05:45 PM	0	14	0	0	4	11	0	0	29
Total	0	46	0	0	4	39	0	0	89
Grand Total	1	100	0	0	5	83	0	0	189
Apprch %	1	99	0	0	5.7	94.3	0	0	
Total %	0.5	52.9	0	0	2.6	43.9	0	0	

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			
Start Time	SB J-Walk	SB X-Walk	App. Total	EB	WB	App. Total	NB J-Walk	NB X-Walk	App. Total	EB	WB	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM													
04:00 PM	0	17	17	0	0	0	0	10	10	0	0	0	27
04:15 PM	0	11	11	0	0	0	1	10	11	0	0	0	22
04:30 PM	0	14	14	0	0	0	0	9	9	0	0	0	23
04:45 PM	1	12	13	0	0	0	0	15	15	0	0	0	28
Total Volume	1	54	55	0	0	0	1	44	45	0	0	0	100
% App. Total	1.8	98.2		0	0		2.2	97.8		0	0		
PHF	.250	.794	.809	.000	.000	.000	.250	.733	.750	.000	.000	.000	.893



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Page No : 1

Groups Printed- Bicycles

	Main Street Sidewalk (north) From North		Main Street From East		Main Street Sidewalk (south) From South		Main Street From West		Int. Total
Start Time	SB J-Walk	SB X-Walk	EB	WB	NB J-Walk	NB X-Walk	EB	Peds	
04:00 PM	0	0	0	0	0	1	0	0	1
04:15 PM	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	1	2	0	0	3
04:45 PM	0	0	0	0	0	5	0	0	5
Total	0	0	0	0	1	8	0	0	9
05:00 PM	0	0	0	0	0	1	0	0	1
05:15 PM	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	1	0	0	1
Grand Total	0	0	0	0	1	9	0	0	10
Apprch %	0	0	0	0	10	90	0	0	
Total %	0	0	0	0	10	90	0	0	

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			Int. Total
Start Time	SB J-Walk	SB X-Walk	App. Total	EB	WB	App. Total	NB J-Walk	NB X-Walk	App. Total	EB	Peds	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM													
04:00 PM	0	0	0	0	0	0	0	1	1	0	0	0	1
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	1	2	3	0	0	0	3
04:45 PM	0	0	0	0	0	0	0	5	5	0	0	0	5
Total Volume	0	0	0	0	0	0	1	8	9	0	0	0	9
% App. Total	0	0		0	0		11.1	88.9		0	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.400	.450	.000	.000	.000	.450



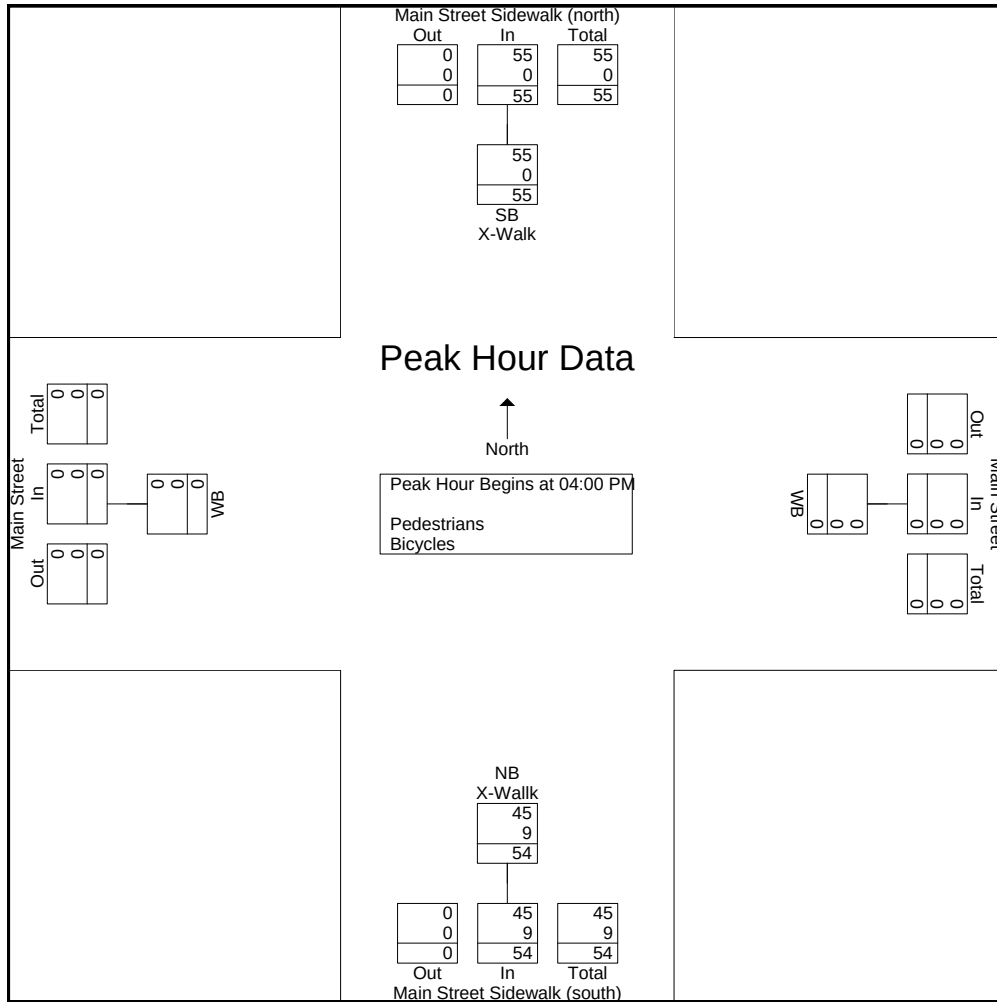
PRECISION
D A T A
INDUSTRIES, LLC

46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
Email: datarequests@pdillc.com

Main Street (East Path Jay Walkers &
west of Mill Rd (Madbury West Crosswalk)
City, State: Durham, NH
Client: Tighe & Bond/V. Kalikiri

File Name : 186243 EEE Pathway
Site Code : 22-1529-
Start Date : 5/8/2018
Page No : 1

	Main Street Sidewalk (north) From North			Main Street From East			Main Street Sidewalk (south) From South			Main Street From West			
Start Time	SB J-Walk	SB X-Walk	App. Total	EB	WB	App. Total	NB J-Walk	NB X-Walk	App. Total	EB	Peds	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM													
04:00 PM	0	17	17	0	0	0	0	11	11	0	0	0	28
04:15 PM	0	11	11	0	0	0	1	10	11	0	0	0	22
04:30 PM	0	14	14	0	0	0	1	11	12	0	0	0	26
04:45 PM	1	12	13	0	0	0	0	20	20	0	0	0	33
Total Volume	1	54	55	0	0	0	2	52	54	0	0	0	109
% App. Total	1.8	98.2		0	0		3.7	96.3		0	0		
PHF	.250	.794	.809	.000	.000	.000	.500	.650	.675	.000	.000	.000	.826
Pedestrians	1	54	55	0	0	0	1	44	45	0	0	0	100
% Pedestrians	100	100	100	0	0	0	50.0	84.6	83.3	0	0	0	91.7
Bicycles	0	0	0	0	0	0	1	8	9	0	0	0	9
% Bicycles	0	0	0	0	0	0	50.0	15.4	16.7	0	0	0	8.3





Start Time	Cars	Medium Heavy	Large Heavy											Total
05/05/18	70	1	0	0	0	0	0	0	0	0	0	0	0	71
01:00	49	2	0	0	0	0	0	0	0	0	0	0	0	51
02:00	30	2	0	0	0	0	0	0	0	0	0	0	0	32
03:00	8	1	0	0	0	0	0	0	0	0	0	0	0	9
04:00	4	2	0	0	0	0	0	0	0	0	0	0	0	6
05:00	17	1	0	0	0	0	0	0	0	0	0	0	0	18
06:00	25	2	1	0	0	0	0	0	0	0	0	0	0	28
07:00	52	1	0	0	0	0	0	0	0	0	0	0	0	53
08:00	160	1	0	0	0	0	0	0	0	0	0	0	0	161
09:00	262	3	1	0	0	0	0	0	0	0	0	0	0	266
10:00	283	0	0	0	0	0	0	0	0	0	0	0	0	283
11:00	251	5	1	0	0	0	0	0	0	0	0	0	0	257
12 PM	243	5	0	0	0	0	0	0	0	0	0	0	0	248
13:00	230	4	0	0	0	0	0	0	0	0	0	0	0	234
14:00	207	1	0	0	0	0	0	0	0	0	0	0	0	208
15:00	183	4	0	0	0	0	0	0	0	0	0	0	0	187
16:00	194	4	0	0	0	0	0	0	0	0	0	0	0	198
17:00	204	1	0	0	0	0	0	0	0	0	0	0	0	205
18:00	231	2	0	0	0	0	0	0	0	0	0	0	0	233
19:00	177	1	0	0	0	0	0	0	0	0	0	0	0	178
20:00	174	3	0	0	0	0	0	0	0	0	0	0	0	177
21:00	134	1	0	0	0	0	0	0	0	0	0	0	0	135
22:00	112	1	0	0	0	0	0	0	0	0	0	0	0	113
23:00	83	0	0	0	0	0	0	0	0	0	0	0	0	83
Total	3383	48	3	0	0	0	0	0	0	0	0	0	0	3434
Percent	98.5%	1.4%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	10:00	11:00	06:00											10:00
Vol.	283	5	1											283
PM Peak	12:00	12:00												12:00
Vol.	243	5												248



Start Time	Cars	Medium Heavy	Large Heavy											Total
05/05/18	65	4	0	0	0	0	0	0	0	0	0	0	0	69
01:00	58	0	0	0	0	0	0	0	0	0	0	0	0	58
02:00	26	1	0	0	0	0	0	0	0	0	0	0	0	27
03:00	16	0	0	0	0	0	0	0	0	0	0	0	0	16
04:00	14	0	0	0	0	0	0	0	0	0	0	0	0	14
05:00	17	3	0	0	0	0	0	0	0	0	0	0	0	20
06:00	41	0	1	0	0	0	0	0	0	0	0	0	0	42
07:00	101	1	0	0	0	0	0	0	0	0	0	0	0	102
08:00	198	5	0	0	0	0	0	0	0	0	0	0	0	203
09:00	372	2	0	0	0	0	0	0	0	0	0	0	0	374
10:00	296	2	0	0	0	0	0	0	0	0	0	0	0	298
11:00	282	4	0	0	0	0	0	0	0	0	0	0	0	286
12 PM	267	3	0	0	0	0	0	0	0	0	0	0	0	270
13:00	219	2	0	0	0	0	0	0	0	0	0	0	0	221
14:00	238	0	0	0	0	0	0	0	0	0	0	0	0	238
15:00	222	0	0	0	0	0	0	0	0	0	0	0	0	222
16:00	246	4	0	0	0	0	0	0	0	0	0	0	0	250
17:00	224	1	0	0	0	0	0	0	0	0	0	0	0	225
18:00	261	0	0	0	0	0	0	0	0	0	0	0	0	261
19:00	207	3	0	0	0	0	0	0	0	0	0	0	0	210
20:00	200	0	0	0	0	0	0	0	0	0	0	0	0	200
21:00	160	0	0	0	0	0	0	0	0	0	0	0	0	160
22:00	113	0	0	0	0	0	0	0	0	0	0	0	0	113
23:00	89	1	0	0	0	0	0	0	0	0	0	0	0	90
Total	3932	36	1	0	0	0	0	0	0	0	0	0	0	3969
Percent	99.1%	0.9%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	09:00	08:00	06:00											09:00
Vol.	372	5	1											374
PM Peak	12:00	16:00												12:00
Vol.	267	4												270



PRECISION
D A T A
INDUSTRIES, LLC

46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
Email: datarequests@pdillc.com

Mill Road
south of Main Street
City, State: Durham, NH
Client: Tighe & Bond/ V. Kalikiri

186243 A Volume
Site Code: 22-1529-002

Start	SB		NB		Combin		5/5/2018	
Time	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	Sat	
12:00	20	56	21	60	41	116		
12:15	21	60	19	64	40	124		
12:30	17	64	16	87	33	151		
12:45	13	71	13	69	26	140	518	
01:00	14	56	19	56	33	112		
01:15	16	61	14	55	30	116		
01:30	8	64	10	59	18	123		
01:45	13	51	15	58	28	109	455	
02:00	12	42	10	53	22	95		
02:15	9	53	6	59	15	112		
02:30	9	48	6	54	15	102		
02:45	2	32	5	27	7	59	446	
03:00	4	50	6	62	10	112		
03:15	3	54	0	50	3	104		
03:30	0	35	6	50	6	85		
03:45	2	9	4	16	6	25	409	
04:00	1	55	3	65	4	120		
04:15	0	54	4	63	4	117		
04:30	2	43	2	60	4	103		
04:45	3	6	5	14	8	20	448	
05:00	3	56	4	52	7	108		
05:15	2	45	5	53	7	98		
05:30	3	46	4	52	7	98		
05:45	10	18	7	20	17	38	430	
06:00	6	73	8	68	14	141		
06:15	7	71	13	67	20	138		
06:30	4	49	9	71	13	120		
06:45	11	28	12	42	23	70	494	
07:00	10	35	25	52	35	87		
07:15	10	45	25	52	35	97		
07:30	14	58	18	60	32	118		
07:45	19	53	34	102	53	155	388	
08:00	30	47	43	43	73	90		
08:15	30	39	43	54	73	93		
08:30	47	51	54	47	101	98		
08:45	54	161	63	203	117	364	377	
09:00	72	34	83	62	155	96		
09:15	62	36	85	44	147	80		
09:30	70	37	113	30	183	67		
09:45	62	266	93	374	155	640	295	
10:00	79	36	78	34	157	70		
10:15	65	26	73	30	138	56		
10:30	68	27	76	29	144	56		
10:45	71	283	71	298	142	581	226	
11:00	69	22	71	23	140	45		
11:15	67	19	74	24	141	43		
11:30	52	21	67	24	119	45		
11:45	69	257	74	286	143	543	173	
Total	1235	2199	1509	2460	2744	4659		
Percent	45.0%	47.2%	55.0%	52.8%				
Day Total		3434		3969		7403		
Peak	10:00	-	05:45	-	09:00	-	05:45	-
Vol.	283	-	251	-	374	-	525	-
P.H.F.	0.896	-	0.860	-	0.827	-	0.869	-



Page 1

186243 AA Class
Site Code: 22-1529-002

Start Time	Cars	Medium Heavy	Large Heavy											Total
05/08/18														
8	34	0	0	0	0	0	0	0	0	0	0	0	0	34
01:00	12	0	0	0	0	0	0	0	0	0	0	0	0	12
02:00	6	0	1	0	0	0	0	0	0	0	0	0	0	7
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	8	2	2	0	0	0	0	0	0	0	0	0	0	12
05:00	30	1	2	0	0	0	0	0	0	0	0	0	0	33
06:00	40	5	0	0	0	0	0	0	0	0	0	0	0	45
07:00	127	11	1	0	0	0	0	0	0	0	0	0	0	139
08:00	221	15	1	0	0	0	0	0	0	0	0	0	0	237
09:00	165	9	1	0	0	0	0	0	0	0	0	0	0	175
10:00	183	12	1	0	0	0	0	0	0	0	0	0	0	196
11:00	229	7	0	0	0	0	0	0	0	0	0	0	0	236
12 PM	256	14	0	0	0	0	0	0	0	0	0	0	0	270
13:00	213	8	0	0	0	0	0	0	0	0	0	0	0	221
14:00	252	9	0	0	0	0	0	0	0	0	0	0	0	261
15:00	317	6	0	0	0	0	0	0	0	0	0	0	0	323
16:00	316	6	0	0	0	0	0	0	0	0	0	0	0	322
17:00	336	4	0	0	0	0	0	0	0	0	0	0	0	340
18:00	269	2	0	0	0	0	0	0	0	0	0	0	0	271
19:00	212	3	0	0	0	0	0	0	0	0	0	0	0	215
20:00	181	1	0	0	0	0	0	0	0	0	0	0	0	182
21:00	80	1	0	0	0	0	0	0	0	0	0	0	0	81
22:00	66	0	0	0	0	0	0	0	0	0	0	0	0	66
23:00	35	0	0	0	0	0	0	0	0	0	0	0	0	35
Total	3588	116	9	0	0	0	0	0	0	0	0	0	0	3713
Percent	96.6%	3.1%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	11:00	08:00	04:00											08:00
Vol.	229	15	2											237
PM Peak	17:00	12:00											17:00	
Vol.	336	14											340	



Start Time	Cars	Medium Heavy	Large Heavy											Total
05/08/18	38	0	0	0	0	0	0	0	0	0	0	0	0	38
01:00	20	0	0	0	0	0	0	0	0	0	0	0	0	20
02:00	9	0	1	0	0	0	0	0	0	0	0	0	0	10
03:00	2	0	0	0	0	0	0	0	0	0	0	0	0	2
04:00	10	2	0	0	0	0	0	0	0	0	0	0	0	12
05:00	23	3	3	0	0	0	0	0	0	0	0	0	0	29
06:00	72	1	1	0	0	0	0	0	0	0	0	0	0	74
07:00	211	7	0	0	0	0	0	0	0	0	0	0	0	218
08:00	196	7	1	0	0	0	0	0	0	0	0	0	0	204
09:00	194	6	2	0	0	0	0	0	0	0	0	0	0	202
10:00	242	11	1	0	0	0	0	0	0	0	0	0	0	254
11:00	285	5	0	0	0	0	0	0	0	0	0	0	0	290
12 PM	287	11	1	0	0	0	0	0	0	0	0	0	0	299
13:00	299	8	0	0	0	0	0	0	0	0	0	0	0	307
14:00	363	3	1	0	0	0	0	0	0	0	0	0	0	367
15:00	373	4	1	0	0	0	0	0	0	0	0	0	0	378
16:00	440	2	0	0	0	0	0	0	0	0	0	0	0	442
17:00	440	1	0	0	0	0	0	0	0	0	0	0	0	441
18:00	354	0	0	0	0	0	0	0	0	0	0	0	0	354
19:00	252	0	0	0	0	0	0	0	0	0	0	0	0	252
20:00	206	0	0	0	0	0	0	0	0	0	0	0	0	206
21:00	109	0	0	0	0	0	0	0	0	0	0	0	0	109
22:00	79	0	0	0	0	0	0	0	0	0	0	0	0	79
23:00	41	0	0	0	0	0	0	0	0	0	0	0	0	41
Total	4545	71	12	0	0	0	0	0	0	0	0	0	0	4628
Percent	98.2%	1.5%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	11:00	10:00	05:00											11:00
Vol.	285	11	3											290
PM Peak	16:00	12:00	12:00											16:00
Vol.	440	11	1											442



PRECISION
D A T A
INDUSTRIES, LLC

46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
Email: datarequests@pdillc.com

Mill Road
south of Main Street
City, State: Durham, NH
Client: Tighe & Bond/ V. Kalikiri

186243 AA Volume
Site Code: 22-1529-002

Start	SB		NB		Combin		5/8/2018	
Time	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	Tue	
12:00	11	65	11	79	22	144		
12:15	9	85	7	75	16	160		
12:30	7	54	9	67	16	121		
12:45	7	66	11	78	18	144	569	
01:00	4	54	6	93	10	147		
01:15	2	60	6	65	8	125		
01:30	3	55	4	75	7	130		
01:45	3	52	4	74	7	126	528	
02:00	4	74	5	88	9	162		
02:15	1	65	1	97	2	162		
02:30	1	64	1	106	2	170		
02:45	1	58	3	76	4	134	628	
03:00	0	83	2	71	2	154		
03:15	0	86	0	102	0	188		
03:30	0	81	0	113	0	194		
03:45	0	73	0	92	0	165	701	
04:00	4	76	3	94	7	170		
04:15	2	76	1	94	3	170		
04:30	2	85	6	121	8	206		
04:45	4	85	2	133	6	218	764	
05:00	4	88	5	127	9	215		
05:15	7	89	11	106	18	195		
05:30	7	77	4	93	11	170		
05:45	15	86	9	115	24	201	781	
06:00	5	66	10	79	15	145		
06:15	11	68	13	103	24	171		
06:30	22	81	29	88	51	169		
06:45	7	56	22	84	29	140	625	
07:00	25	46	37	57	62	103		
07:15	25	60	45	59	70	119		
07:30	28	57	61	67	89	124		
07:45	61	52	75	69	136	121	467	
08:00	76	48	66	65	142	113		
08:15	55	53	51	49	106	102		
08:30	40	52	36	49	76	101		
08:45	66	29	51	43	117	72	388	
09:00	56	20	45	38	101	58		
09:15	33	24	47	26	80	50		
09:30	41	26	43	24	84	50		
09:45	45	11	67	21	112	32	190	
10:00	45	21	70	28	115	49		
10:15	47	9	55	19	102	28		
10:30	45	17	66	18	111	35		
10:45	59	19	63	14	122	33	145	
11:00	68	8	67	13	135	21		
11:15	54	11	76	7	130	18		
11:30	46	6	64	13	110	19		
11:45	68	10	83	8	151	18	76	
Total	1126	2587	1353	3275	2479	5862		
Percent	45.4%	44.1%	54.6%	55.9%				
Day Total		3713		4628		8341		
Peak	08:00	-	04:30	-	11:00	-	04:30	-
Vol.	237	-	347	-	290	-	487	-
P.H.F.	0.780	-	0.975	-	0.873	-	0.915	-



Start Time	Cars	Medium Heavy	Large Heavy											Total
05/05/18														
8:00	37	1	0	0	0	0	0	0	0	0	0	0	0	38
01:00	21	0	0	0	0	0	0	0	0	0	0	0	0	21
02:00	15	0	0	0	0	0	0	0	0	0	0	0	0	15
03:00	5	0	0	0	0	0	0	0	0	0	0	0	0	5
04:00	10	0	0	0	0	0	0	0	0	0	0	0	0	10
05:00	1	1	0	0	0	0	0	0	0	0	0	0	0	2
06:00	12	0	1	0	0	0	0	0	0	0	0	0	0	13
07:00	51	2	0	0	0	0	0	0	0	0	0	0	0	53
08:00	128	1	0	0	0	0	0	0	0	0	0	0	0	129
09:00	204	2	0	0	0	0	0	0	0	0	0	0	0	206
10:00	208	0	1	0	0	0	0	0	0	0	0	0	0	209
11:00	165	3	0	0	0	0	0	0	0	0	0	0	0	168
12 PM	164	1	0	0	0	0	0	0	0	0	0	0	0	165
13:00	143	2	0	0	0	0	0	0	0	0	0	0	0	145
14:00	111	0	0	0	0	0	0	0	0	0	0	0	0	111
15:00	121	1	0	0	0	0	0	0	0	0	0	0	0	122
16:00	133	0	0	0	0	0	0	0	0	0	0	0	0	133
17:00	138	0	0	0	0	0	0	0	0	0	0	0	0	138
18:00	163	0	0	0	0	0	0	0	0	0	0	0	0	163
19:00	132	2	0	0	0	0	0	0	0	0	0	0	0	134
20:00	109	0	0	0	0	0	0	0	0	0	0	0	0	109
21:00	79	0	0	0	0	0	0	0	0	0	0	0	0	79
22:00	49	0	0	0	0	0	0	0	0	0	0	0	0	49
23:00	38	0	0	0	0	0	0	0	0	0	0	0	0	38
Total	2237	16	2	0	0	0	0	0	0	0	0	0	0	2255
Percent	99.2%	0.7%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	10:00	11:00	06:00											10:00
Vol.	208	3	1											209
PM Peak	12:00	13:00											12:00	
Vol.	164	2											165	



Start Time	Cars	Medium Heavy	Large Heavy											Total
05/05/18														
8:00	26	1	0	0	0	0	0	0	0	0	0	0	0	27
01:00	17	0	0	0	0	0	0	0	0	0	0	0	0	17
02:00	15	0	0	0	0	0	0	0	0	0	0	0	0	15
03:00	6	0	0	0	0	0	0	0	0	0	0	0	0	6
04:00	5	1	0	0	0	0	0	0	0	0	0	0	0	6
05:00	11	0	0	0	0	0	0	0	0	0	0	0	0	11
06:00	22	2	1	0	0	0	0	0	0	0	0	0	0	25
07:00	65	1	0	0	0	0	0	0	0	0	0	0	0	66
08:00	165	1	0	0	0	0	0	0	0	0	0	0	0	166
09:00	213	2	0	0	0	0	0	0	0	0	0	0	0	215
10:00	212	1	0	0	0	0	0	0	0	0	0	0	0	213
11:00	158	3	0	0	0	0	0	0	0	0	0	0	0	161
12 PM	169	0	0	0	0	0	0	0	0	0	0	0	0	169
13:00	141	2	0	0	0	0	0	0	0	0	0	0	0	143
14:00	108	0	0	0	0	0	0	0	0	0	0	0	0	108
15:00	108	1	0	0	0	0	0	0	0	0	0	0	0	109
16:00	136	0	0	0	0	0	0	0	0	0	0	0	0	136
17:00	159	0	0	0	0	0	0	0	0	0	0	0	0	159
18:00	152	0	0	0	0	0	0	0	0	0	0	0	0	152
19:00	129	1	0	0	0	0	0	0	0	0	0	0	0	130
20:00	104	0	0	0	0	0	0	0	0	0	0	0	0	104
21:00	51	0	0	0	0	0	0	0	0	0	0	0	0	51
22:00	44	0	0	0	0	0	0	0	0	0	0	0	0	44
23:00	27	0	0	0	0	0	0	0	0	0	0	0	0	27
Total	2243	16	1	0	0	0	0	0	0	0	0	0	0	2260
Percent	99.2%	0.7%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	09:00	11:00	06:00											09:00
Vol.	213	3	1											215
PM Peak	12:00	13:00											12:00	
Vol.	169	2											169	



PRECISION
D A T A
INDUSTRIES, LLC

46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
Email: datarequests@pdillc.com

Mill Road Plaza Driveway
east of Mill Road
City, State: Durham, NH
Client: Tighe & Bond/ V. Kalikiri

186243 B Volume
Site Code: 22-1529-002

Start	WB				EB				Combin ed		5/5/2018		
Time	A.M.		P.M.		A.M.		P.M.		A.M.		P.M.		Sat
12:00	10		33		9		45		19		78		
12:15	11		45		7		46		18		91		
12:30	7		48		5		37		12		85		
12:45	10	38	39	165	6	27	41	169	16	65	80	334	
01:00	5		34		4		32		9		66		
01:15	5		34		5		40		10		74		
01:30	4		39		2		41		6		80		
01:45	7	21	38	145	6	17	30	143	13	38	68	288	
02:00	6		30		4		27		10		57		
02:15	2		34		7		26		9		60		
02:30	5		24		3		23		8		47		
02:45	2	15	23	111	1	15	32	108	3	30	55	219	
03:00	2		38		2		26		4		64		
03:15	0		21		1		27		1		48		
03:30	2		31		0		27		2		58		
03:45	1	5	32	122	3	6	29	109	4	11	61	231	
04:00	2		37		1		38		3		75		
04:15	4		30		0		33		4		63		
04:30	1		29		2		36		3		65		
04:45	3	10	37	133	3	6	29	136	6	16	66	269	
05:00	2		26		2		32		4		58		
05:15	0		34		1		37		1		71		
05:30	0		38		2		41		2		79		
05:45	0	2	40	138	6	11	49	159	6	13	89	297	
06:00	1		42		5		45		6		87		
06:15	2		50		6		47		8		97		
06:30	3		37		2		33		5		70		
06:45	7	13	34	163	12	25	27	152	19	38	61	315	
07:00	10		32		13		31		23		63		
07:15	14		37		16		33		30		70		
07:30	9		37		16		37		25		74		
07:45	20	53	28	134	21	66	29	130	41	119	57	264	
08:00	28		27		30		26		58		53		
08:15	25		29		36		28		61		57		
08:30	30		19		48		33		78		52		
08:45	46	129	34	109	52	166	17	104	98	295	51	213	
09:00	48		33		59		13		107		46		
09:15	53		22		51		9		104		31		
09:30	60		11		56		15		116		26		
09:45	45	206	13	79	49	215	14	51	94	421	27	130	
10:00	61		17		59		14		120		31		
10:15	50		14		49		11		99		25		
10:30	49		11		52		11		101		22		
10:45	49	209	7	49	53	213	8	44	102	422	15	93	
11:00	45		10		47		8		92		18		
11:15	42		16		39		7		81		23		
11:30	36		8		35		6		71		14		
11:45	45	168	4	38	40	161	6	27	85	329	10	65	
Total	869		1386		928		1332		1797		2718		
Percent	48.4%		51.0%		51.6%		49.0%						
Day Total		2255				2260				4515			
Peak	09:15	-	05:30	-	08:45	-	05:30	-	09:15	-	05:30	-	-
Vol.	219	-	170	-	218	-	182	-	434	-	352	-	-
P.H.F.	0.898		0.850		0.924		0.929		0.904		0.907		



Page 1

186243 BB Class
Site Code: 22-1529-002

Start Time	Cars	Medium Heavy	Large Heavy											Total
05/08/1														
8	18	0	0	0	0	0	0	0	0	0	0	0	0	18
01:00	13	0	0	0	0	0	0	0	0	0	0	0	0	13
02:00	4	1	0	0	0	0	0	0	0	0	0	0	0	5
03:00	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:00	5	1	0	0	0	0	0	0	0	0	0	0	0	6
05:00	1	2	4	0	0	0	0	0	0	0	0	0	0	7
06:00	30	0	1	0	0	0	0	0	0	0	0	0	0	31
07:00	60	2	0	0	0	0	0	0	0	0	0	0	0	62
08:00	84	5	3	0	0	0	0	0	0	0	0	0	0	92
09:00	117	1	1	0	0	0	0	0	0	0	0	0	0	119
10:00	137	4	1	0	0	0	0	0	0	0	0	0	0	142
11:00	153	2	0	0	0	0	0	0	0	0	0	0	0	155
12 PM	169	4	1	0	0	0	0	0	0	0	0	0	0	174
13:00	188	1	0	0	0	0	0	0	0	0	0	0	0	189
14:00	173	3	0	0	0	0	0	0	0	0	0	0	0	176
15:00	205	1	0	0	0	0	0	0	0	0	0	0	0	206
16:00	224	1	0	0	0	0	0	0	0	0	0	0	0	225
17:00	239	1	0	0	0	0	0	0	0	0	0	0	0	240
18:00	218	1	0	0	0	0	0	0	0	0	0	0	0	219
19:00	164	0	0	0	0	0	0	0	0	0	0	0	0	164
20:00	145	0	0	0	0	0	0	0	0	0	0	0	0	145
21:00	51	0	0	0	0	0	0	0	0	0	0	0	0	51
22:00	34	0	0	0	0	0	0	0	0	0	0	0	0	34
23:00	9	0	0	0	0	0	0	0	0	0	0	0	0	9
Total	2442	30	11	0	0	0	0	0	0	0	0	0	0	2483
Percent	98.3%	1.2%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	11:00	08:00	05:00											11:00
Vol.	153	5	4											155
PM Peak	17:00	12:00	12:00											17:00
Vol.	239	4	1											240



Start Time	Cars	Medium Heavy	Large Heavy											Total
05/08/18														
8:00	13	0	0	0	0	0	0	0	0	0	0	0	0	13
01:00	10	0	0	0	0	0	0	0	0	0	0	0	0	10
02:00	6	0	1	0	0	0	0	0	0	0	0	0	0	7
03:00	2	0	0	0	0	0	0	0	0	0	0	0	0	2
04:00	9	2	2	0	0	0	0	0	0	0	0	0	0	13
05:00	26	1	2	0	0	0	0	0	0	0	0	0	0	29
06:00	38	1	1	0	0	0	0	0	0	0	0	0	0	40
07:00	74	4	1	0	0	0	0	0	0	0	0	0	0	79
08:00	128	4	2	0	0	0	0	0	0	0	0	0	0	134
09:00	110	3	1	0	0	0	0	0	0	0	0	0	0	114
10:00	140	4	0	0	0	0	0	0	0	0	0	0	0	144
11:00	172	4	0	0	0	0	0	0	0	0	0	0	0	176
12 PM	178	6	0	0	0	0	0	0	0	0	0	0	0	184
13:00	161	2	0	0	0	0	0	0	0	0	0	0	0	163
14:00	162	1	0	0	0	0	0	0	0	0	0	0	0	163
15:00	209	1	0	0	0	0	0	0	0	0	0	0	0	210
16:00	248	1	0	0	0	0	0	0	0	0	0	0	0	249
17:00	223	1	0	0	0	0	0	0	0	0	0	0	0	224
18:00	202	1	0	0	0	0	0	0	0	0	0	0	0	203
19:00	151	0	0	0	0	0	0	0	0	0	0	0	0	151
20:00	118	0	0	0	0	0	0	0	0	0	0	0	0	118
21:00	27	0	0	0	0	0	0	0	0	0	0	0	0	27
22:00	19	0	0	0	0	0	0	0	0	0	0	0	0	19
23:00	11	0	0	0	0	0	0	0	0	0	0	0	0	11
Total	2437	36	10	0	0	0	0	0	0	0	0	0	0	2483
Percent	98.1%	1.4%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	
AM Peak	11:00	07:00	04:00											11:00
Vol.	172	4	2											176
PM Peak	16:00	12:00											16:00	
Vol.	248	6											249	



PRECISION
D A T A
INDUSTRIES, LLC

46 Morton Street, Framingham, MA 01702
Office: 508-875-0100 Fax: 508-875-0118
Email: datarequests@pdillc.com

Mill Road Plaza Driveway
east of Mill Road
City, State: Durham, NH
Client: Tighe & Bond/ V. Kalikiri

186243 BB Volume
Site Code: 22-1529-002

Start	WB		EB		Combin		5/8/2018	
Time	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	Tue	
12:00	4	34	4	44	8	78		
12:15	3	44	5	53	8	97		
12:30	4	49	2	37	6	86		
12:45	7	47	2	50	9	97	358	
01:00	4	52	3	43	7	95		
01:15	3	43	2	41	5	84		
01:30	4	51	4	46	8	97		
01:45	2	43	1	33	3	76	352	
02:00	2	55	4	53	6	108		
02:15	2	45	2	41	4	86		
02:30	0	46	1	34	1	80		
02:45	1	30	0	35	1	65	339	
03:00	1	38	0	50	1	88		
03:15	0	54	0	50	0	104		
03:30	0	52	0	55	0	107		
03:45	0	62	2	55	2	117	416	
04:00	1	44	4	61	5	105		
04:15	2	64	2	54	4	118		
04:30	2	56	2	63	4	119		
04:45	1	61	5	71	6	132	474	
05:00	1	61	7	54	8	115		
05:15	3	55	11	61	14	116		
05:30	1	63	4	57	5	120		
05:45	2	61	7	52	9	113	464	
06:00	2	48	7	46	9	94		
06:15	2	64	12	48	14	112		
06:30	20	57	14	60	34	117		
06:45	7	50	7	49	14	99	422	
07:00	14	42	15	33	29	75		
07:15	11	36	15	37	26	73		
07:30	26	45	19	52	45	97		
07:45	11	41	30	29	41	70	315	
08:00	22	46	39	37	61	83		
08:15	28	31	35	33	63	64		
08:30	21	33	19	33	40	66		
08:45	21	35	41	15	62	50	263	
09:00	31	19	30	5	61	24		
09:15	22	17	20	9	42	26		
09:30	36	7	26	8	62	15		
09:45	30	8	38	5	68	13	78	
10:00	38	12	32	6	70	18		
10:15	43	12	38	2	81	14		
10:30	34	2	40	6	74	8		
10:45	27	8	34	5	61	13	53	
11:00	41	2	45	3	86	5		
11:15	42	3	44	3	86	6		
11:30	32	1	36	0	68	1		
11:45	40	3	51	5	91	8	20	
Total	651	1832	761	1722	1412	3554		
Percent	46.1%	51.5%	53.9%	48.5%				
Day Total		2483		2483		4966		
Peak	11:00	-	04:15	-	11:00	-	04:15	-
Vol.	155	-	242	-	176	-	484	-
P.H.F.	0.901	-	0.945	-	0.863	-	0.917	-

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Madbury Road					Main Street				Main Street				Total
	from North					from East				from West				
	Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0		114	0	0	114	131	77	0	208	322
11:45 AM	0	0	0	0		115	0	0	115	146	96	0	242	357
Total	0	0	0	0		229	0	0	229	277	173	0	450	679
12:00 PM	0	0	0	0		135	0	0	135	191	93	0	284	419
12:15 PM	0	0	0	0		113	0	0	113	155	71	0	226	339
12:30 PM	0	0	0	0		125	0	0	125	154	82	0	236	361
12:45 PM	0	0	0	0		79	0	0	79	157	80	0	237	316
Total	0	0	0	0		452	0	0	452	657	326	0	983	1435
1:00 PM	0	0	0	0		110	0	0	110	127	62	0	189	299
1:15 PM	0	0	0	0		120	0	0	120	125	63	0	188	308
Total	0	0	0	0		230	0	0	230	252	125	0	377	607
Grand Total	0	0	0	0		911	0	0	911	1186	624	0	1810	2721
Approach %	0.0	0.0	0.0			100.0	0.0	0.0		65.5	34.5	0.0		
Total %	0.0	0.0	0.0	0.0		33.5	0.0	0.0	33.5	43.6	22.9	0.0	66.5	
Exiting Leg Total	1535					1186				0				2721
Cars	0	0	0	0		899	0	0	899	1174	618	0	1792	2691
% Cars	0.0	0.0	0.0	0.0		98.7	0.0	0.0	98.7	99.0	99.0	0.0	99.0	98.9
Exiting Leg Total	1517					1174				0				2691
Heavy Vehicles	0	0	0	0		12	0	0	12	12	6	0	18	30
% Heavy Vehicles	0.0	0.0	0.0	0.0		1.3	0.0	0.0	1.3	1.0	1.0	0.0	1.0	1.1
Exiting Leg Total	18					12				0				30

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:45 AM	0	0	0	0	115	0	0	115	146	96	0	242	357
12:00 PM	0	0	0	0	135	0	0	135	191	93	0	284	419
12:15 PM	0	0	0	0	113	0	0	113	155	71	0	226	339
12:30 PM	0	0	0	0	125	0	0	125	154	82	0	236	361
Total Volume	0	0	0	0	488	0	0	488	646	342	0	988	1476
% Approach Total	0.0	0.0	0.0		100.0	0.0	0.0		65.4	34.6	0.0		
PHF	0.000	0.000	0.000	0.000	0.904	0.000	0.000	0.904	0.846	0.891	0.000	0.870	0.881
Cars	0	0	0	0	485	0	0	485	639	339	0	978	1463
Cars %	0.0	0.0	0.0	0.0	99.4	0.0	0.0	99.4	98.9	99.1	0.0	99.0	99.1
Heavy Vehicles	0	0	0	0	3	0	0	3	7	3	0	10	13
Heavy Vehicles %	0.0	0.0	0.0	0.0	0.6	0.0	0.0	0.6	1.1	0.9	0.0	1.0	0.9
Cars Enter Leg	0	0	0	0	485	0	0	485	639	339	0	978	1463
Heavy Enter Leg	0	0	0	0	3	0	0	3	7	3	0	10	13
Total Entering Leg	0	0	0	0	488	0	0	488	646	342	0	988	1476
Cars Exiting Leg	824				639				0				1463
Heavy Exiting Leg	6				7				0				13
Total Exiting Leg	830				646				0				1476

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars

	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	111	0	0	111	130	76	0	206	317
11:45 AM	0	0	0	0	114	0	0	114	143	95	0	238	352
Total	0	0	0	0	225	0	0	225	273	171	0	444	669
12:00 PM	0	0	0	0	134	0	0	134	188	91	0	279	413
12:15 PM	0	0	0	0	112	0	0	112	155	71	0	226	338
12:30 PM	0	0	0	0	125	0	0	125	153	82	0	235	360
12:45 PM	0	0	0	0	78	0	0	78	155	80	0	235	313
Total	0	0	0	0	449	0	0	449	651	324	0	975	1424
1:00 PM	0	0	0	0	108	0	0	108	126	61	0	187	295
1:15 PM	0	0	0	0	117	0	0	117	124	62	0	186	303
Total	0	0	0	0	225	0	0	225	250	123	0	373	598
Grand Total	0	0	0	0	899	0	0	899	1174	618	0	1792	2691
Approach %	0.0	0.0	0.0		100.0	0.0	0.0		65.5	34.5	0.0		
Total %	0.0	0.0	0.0	0.0	33.4	0.0	0.0	33.4	43.6	23.0	0.0	66.6	
Exiting Leg Total	1517				1174				0				2691

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Madbury Road					Main Street				Main Street				Total
	from North					from East				from West				
	Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:45 AM	0	0	0	0	0	114	0	0	114	143	95	0	238	352
12:00 PM	0	0	0	0	0	134	0	0	134	188	91	0	279	413
12:15 PM	0	0	0	0	0	112	0	0	112	155	71	0	226	338
12:30 PM	0	0	0	0	0	125	0	0	125	153	82	0	235	360
Total Volume	0	0	0	0	0	485	0	0	485	639	339	0	978	1463
% Approach Total	0.0	0.0	0.0			100.0	0.0	0.0		65.3	34.7	0.0		
PHF	0.000	0.000	0.000	0.000		0.905	0.000	0.000	0.905	0.850	0.892	0.000	0.876	0.886
Entering Leg	0	0	0	0		485	0	0	485	639	339	0	978	1463
Exiting Leg	824					639				0				1463
Total	824					1124				978				2926

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**



Class: **Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)**

	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	3	0	0	3	1	1	0	2	5
11:45 AM	0	0	0	0	1	0	0	1	3	1	0	4	5
Total	0	0	0	0	4	0	0	4	4	2	0	6	10
12:00 PM	0	0	0	0	1	0	0	1	3	2	0	5	6
12:15 PM	0	0	0	0	1	0	0	1	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	1	0	0	1	1
12:45 PM	0	0	0	0	1	0	0	1	2	0	0	2	3
Total	0	0	0	0	3	0	0	3	6	2	0	8	11
1:00 PM	0	0	0	0	2	0	0	2	1	1	0	2	4
1:15 PM	0	0	0	0	3	0	0	3	1	1	0	2	5
Total	0	0	0	0	5	0	0	5	2	2	0	4	9
Grand Total	0	0	0	0	12	0	0	12	12	6	0	18	30
Approach %	0.0	0.0	0.0		100.0	0.0	0.0		66.7	33.3	0.0		
Total %	0.0	0.0	0.0	0.0	40.0	0.0	0.0	40.0	40.0	20.0	0.0	60.0	
Exiting Leg Total	18				12				0				30
Buses	0	0	0	0	5	0	0	5	4	4	0	8	13
% Buses	0.0	0.0	0.0	0.0	41.7	0.0	0.0	41.7	33.3	66.7	0.0	44.4	43.3
Exiting Leg Total	9				4				0				13
Single-Unit Trucks	0	0	0	0	7	0	0	7	8	2	0	10	17
% Single-Unit	0.0	0.0	0.0	0.0	58.3	0.0	0.0	58.3	66.7	33.3	0.0	55.6	56.7
Exiting Leg Total	9				8				0				17
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Exiting Leg Total	0				0				0				0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	3	0	0	3	1	1	0	2	5
11:45 AM	0	0	0	0	1	0	0	1	3	1	0	4	5
12:00 PM	0	0	0	0	1	0	0	1	3	2	0	5	6
12:15 PM	0	0	0	0	1	0	0	1	0	0	0	0	1
Total Volume	0	0	0	0	6	0	0	6	7	4	0	11	17
% Approach Total	0.0	0.0	0.0		100.0	0.0	0.0		63.6	36.4	0.0		
PHF	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.500	0.583	0.500	0.000	0.550	0.708
Buses	0	0	0	0	3	0	0	3	2	3	0	5	8
Buses %	0.0	0.0	0.0	0.0	50.0	0.0	0.0	50.0	28.6	75.0	0.0	45.5	47.1
Single-Unit Trucks	0	0	0	0	3	0	0	3	5	1	0	6	9
Single-Unit %	0.0	0.0	0.0	0.0	50.0	0.0	0.0	50.0	71.4	25.0	0.0	54.5	52.9
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Buses	0	0	0	0	3	0	0	3	2	3	0	5	8
Single-Unit Trucks	0	0	0	0	3	0	0	3	5	1	0	6	9
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Entering Leg	0	0	0	0	6	0	0	6	7	4	0	11	17
Buses	6				2				0				8
Single-Unit Trucks	4				5				0				9
Articulated Trucks	0				0				0				0
Total Exiting Leg	10				7				0				17

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Buses

	Madbury Road					Main Street					Main Street					Total
	from North					from East					from West					
	Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total			
11:30 AM	0	0	0	0	0	1	0	0	1	0	1	0	1	2		
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	1	1		
Total	0	0	0	0	0	1	0	0	1	0	2	0	2	3		
12:00 PM	0	0	0	0	0	1	0	0	1	2	1	0	3	4		
12:15 PM	0	0	0	0	0	1	0	0	1	0	0	0	0	1		
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
12:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	1	1		
Total	0	0	0	0	0	2	0	0	2	3	1	0	4	6		
1:00 PM	0	0	0	0	0	1	0	0	1	1	1	0	2	3		
1:15 PM	0	0	0	0	0	1	0	0	1	0	0	0	0	1		
Total	0	0	0	0	0	2	0	0	2	1	1	0	2	4		
Grand Total	0	0	0	0	0	5	0	0	5	4	4	0	8	13		
Approach %	0.0	0.0	0.0			100.0	0.0	0.0		50.0	50.0	0.0				
Total %	0.0	0.0	0.0	0.0		38.5	0.0	0.0	38.5	30.8	30.8	0.0	61.5			
Exiting Leg Total	9					4					0					13

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	1	0	0	1	0	1	0	1	2
11:45 AM	0	0	0	0	0	0	0	0	0	1	0	1	1
12:00 PM	0	0	0	0	1	0	0	1	2	1	0	3	4
12:15 PM	0	0	0	0	1	0	0	1	0	0	0	0	1
Total Volume	0	0	0	0	3	0	0	3	2	3	0	5	8
% Approach Total	0.0	0.0	0.0		100.0	0.0	0.0		40.0	60.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.750	0.000	0.000	0.750	0.250	0.750	0.000	0.417	0.500
Entering Leg	0	0	0	0	3	0	0	3	2	3	0	5	8
Exiting Leg				6				2				0	8
Total				6				5				5	16

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Single-Unit Trucks

	Madbury Road					Main Street					Main Street					Total
	from North					from East					from West					
	Right	Left	U-Turn	Total		Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total			
11:30 AM	0	0	0	0		2	0	0	2	1	0	0	1	3		
11:45 AM	0	0	0	0		1	0	0	1	3	0	0	3	4		
Total	0	0	0	0		3	0	0	3	4	0	0	4	7		
12:00 PM	0	0	0	0		0	0	0	0	1	1	0	2	2		
12:15 PM	0	0	0	0		0	0	0	0	0	0	0	0	0		
12:30 PM	0	0	0	0		0	0	0	0	1	0	0	1	1		
12:45 PM	0	0	0	0		1	0	0	1	1	0	0	1	2		
Total	0	0	0	0		1	0	0	1	3	1	0	4	5		
1:00 PM	0	0	0	0		1	0	0	1	0	0	0	0	1		
1:15 PM	0	0	0	0		2	0	0	2	1	1	0	2	4		
Total	0	0	0	0		3	0	0	3	1	1	0	2	5		
Grand Total	0	0	0	0		7	0	0	7	8	2	0	10	17		
Approach %	0.0	0.0	0.0			100.0	0.0	0.0		80.0	20.0	0.0				
Total %	0.0	0.0	0.0	0.0		41.2	0.0	0.0	41.2	47.1	11.8	0.0	58.8			
Exiting Leg Total	9					8					0					17

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	2	0	0	2	1	0	0	1	3
11:45 AM	0	0	0	0	1	0	0	1	3	0	0	3	4
12:00 PM	0	0	0	0	0	0	0	0	1	1	0	2	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	3	0	0	3	5	1	0	6	9
% Approach Total	0.0	0.0	0.0		100.0	0.0	0.0		83.3	16.7	0.0		
PHF	0.000	0.000	0.000	0.000	0.375	0.000	0.000	0.375	0.417	0.250	0.000	0.500	0.563
Entering Leg	0	0	0	0	3	0	0	3	5	1	0	6	9
Exiting Leg				4				5				0	9
Total				4				8				6	18

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



46 Morton Street, Framingham, MA 01702
 Office: 508-875-0100 Fax: 508-875-0118
 Email: datarequests@pdilic.com

Articulated Trucks

	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Exiting Leg Total	0				0				0				0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Madbury Road				Main Street				Main Street				Total
	from North				from East				from West				
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0
% Approach Total	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0
Exiting Leg				0				0				0	0
Total				0				0				0	0

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**



Bicycles (on Roadway and Crosswalks)

	Madbury Road						Main Street							Main Street							Total
	from North						from East							from West							
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total			
11:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	1	2	0	0	0	0	2	3	
11:45 AM	0	0	0	0	1	1	0	0	0	0	0	0	0	4	2	0	0	0	6	7	
Total	0	0	0	0	1	1	1	0	0	0	0	0	1	6	2	0	0	0	8	10	
12:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	0	1	
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	3	0	4	4	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1	
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	0	1	0	0	0	0	0	1	2	0	0	3	0	5	6	
1:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	0	1	
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	0	1	
Grand Total	0	0	0	0	1	1	3	0	0	0	0	0	3	8	2	0	3	0	13	17	
Approach %	0.0	0.0	0.0	0.0	100.0		100.0	0.0	0.0	0.0	0.0	0.0		61.5	15.4	0.0	23.1	0.0			
Total %	0.0	0.0	0.0	0.0	5.9	5.9	17.6	0.0	0.0	0.0	0.0	0.0	17.6	47.1	11.8	0.0	17.6	0.0	76.5		
Exiting Leg Total	6						8							3							17

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Madbury Road						Main Street						Main Street							
	from North						from East						from West							
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total		
11:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	1	2	0	0	0	0	2	3
11:45 AM	0	0	0	0	1	1	0	0	0	0	0	0	4	2	0	0	0	0	6	7
12:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	3	0	4	4
Total Volume	0	0	0	0	1	1	2	0	0	0	0	0	2	7	2	0	3	0	12	15
% Approach Total	0.0	0.0	0.0	0.0	100.0		100.0	0.0	0.0	0.0	0.0	0.0		58.3	16.7	0.0	25.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.250	0.250	0.500	0.000	0.000	0.000	0.000	0.500		0.438	0.250	0.000	0.250	0.000	0.500	0.536
Entering Leg	0	0	0	0	1	1	2	0	0	0	0	2	7	2	0	3	0	12		15
Exiting Leg	5						7						3							15
Total	6						9						15							30

PDI File #: **197243 A**
 Location: **N: Madbury Road**
 Location: **E: Main Street W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Pedestrians

	Madbury Road						Main Street						Main Street						Total
	from North						from East						from West						
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	4	5	9	0	0	0	0	0	0	0	0	0	9	13	22	31
11:45 AM	0	0	0	3	4	7	0	0	0	0	0	0	0	0	0	5	8	13	20
Total	0	0	0	7	9	16	0	0	0	0	0	0	0	0	0	14	21	35	51
12:00 PM	0	0	0	7	12	19	0	0	0	0	0	0	0	0	0	12	6	18	37
12:15 PM	0	0	0	9	0	9	0	0	0	0	0	0	0	0	0	16	13	29	38
12:30 PM	0	0	0	8	4	12	0	0	0	0	0	0	0	0	0	12	16	28	40
12:45 PM	0	0	0	3	7	10	0	0	0	0	0	0	0	0	0	14	8	22	32
Total	0	0	0	27	23	50	0	0	0	0	0	0	0	0	0	54	43	97	147
1:00 PM	0	0	0	1	3	4	0	0	0	0	0	0	0	0	0	10	14	24	28
1:15 PM	0	0	0	9	0	9	0	0	0	0	0	0	0	0	0	21	7	28	37
Total	0	0	0	10	3	13	0	0	0	0	0	0	0	0	0	31	21	52	65
Grand Total	0	0	0	44	35	79	0	0	0	0	0	0	0	0	0	99	85	184	263
Approach %	0	0	0	55.696	44.304		0	0	0	0	0	0	0	0	0	53.804	46.196		
Total %	0	0	0	16.73	13.308	30.038	0	0	0	0	0	0	0	0	0	37.643	32.319	69.962	
Exiting Leg Total	79						0						184						263

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:00 PM	Madbury Road						Main Street						Main Street							
	from North						from East						from West							Total
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total		
12:00 PM	0	0	0	7	12	19	0	0	0	0	0	0	0	0	0	12	6	18	37	
12:15 PM	0	0	0	9	0	9	0	0	0	0	0	0	0	0	0	16	13	29	38	
12:30 PM	0	0	0	8	4	12	0	0	0	0	0	0	0	0	0	12	16	28	40	
12:45 PM	0	0	0	3	7	10	0	0	0	0	0	0	0	0	0	14	8	22	32	
Total Volume	0	0	0	27	23	50	0	0	0	0	0	0	0	0	0	54	43	97	147	
% Approach Total	0.0	0.0	0.0	54.0	46.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	55.7	44.3		
PHF	0.000	0.000	0.000	0.750	0.479	0.658	0.000	0.000	0.000	0.000	0.000	0.000		0.000	0.000	0.000	0.844	0.672	0.836	
Entering Leg	0	0	0	27	23	50	0	0	0	0	0	0	0	0	0	54	43	97	147	
Exiting Leg	50						0						97						147	
Total	100						0						194						294	

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	114	0	0	0	114	0	0	0	0	0	0	0	0	0	0	0	2	152	0	154	268
4:15 PM	119	0	0	0	119	0	0	0	0	0	0	1	2	0	3	0	1	160	0	161	283
4:30 PM	102	0	2	0	104	0	0	0	0	0	0	0	1	0	1	0	4	166	0	170	275
4:45 PM	125	0	1	0	126	0	0	0	0	0	0	0	0	0	0	0	1	164	0	165	291
Total	460	0	3	0	463	0	0	0	0	0	0	1	3	0	4	0	8	642	0	650	1117
5:00 PM	108	0	2	0	110	1	0	0	0	1	0	1	1	0	2	0	1	186	0	187	300
5:15 PM	98	0	1	0	99	0	0	0	0	0	0	0	1	0	1	0	1	184	0	185	285
5:30 PM	112	0	1	0	113	0	0	0	0	0	0	2	1	0	3	0	1	156	0	157	273
5:45 PM	108	0	2	0	110	0	0	0	0	0	0	1	1	0	2	0	2	156	0	158	270
Total	426	0	6	0	432	1	0	0	0	1	0	4	4	0	8	0	5	682	0	687	1128
Grand Total	886	0	9	0	895	1	0	0	0	1	0	5	7	0	12	0	13	1324	0	1337	2245
Approach %	99.0	0.0	1.0	0.0		100.0	0.0	0.0	0.0	0.0	0.0	41.7	58.3	0.0		0.0	1.0	99.0	0.0		
Total %	39.5	0.0	0.4	0.0	39.9	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.3	0.0	0.5	0.0	0.6	59.0	0.0	59.6	
Exiting Leg Total	1330					22					0					893					2245
Cars	869	0	9	0	878	1	0	0	0	1	0	5	7	0	12	0	13	1309	0	1322	2213
% Cars	98.1	0.0	100.0	0.0	98.1	100.0	0.0	0.0	0.0	100.0	0.0	100.0	100.0	0.0	100.0	0.0	100.0	98.9	0.0	98.9	98.6
Exiting Leg Total	1315					22					0					876					2213
Heavy Vehicles	17	0	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0	15	0	15	32
% Heavy Vehicles	1.9	0.0	0.0	0.0	1.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1	0.0	1.1	1.4
Exiting Leg Total	15					0					0					17					32

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:30 PM	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:30 PM	102	0	2	0	104	0	0	0	0	0	0	0	1	0	1	0	4	166	0	170	275
4:45 PM	125	0	1	0	126	0	0	0	0	0	0	0	0	0	0	0	1	164	0	165	291
5:00 PM	108	0	2	0	110	1	0	0	0	1	0	1	1	0	2	0	1	186	0	187	300
5:15 PM	98	0	1	0	99	0	0	0	0	0	0	0	1	0	1	0	1	184	0	185	285
Total Volume	433	0	6	0	439	1	0	0	0	1	0	1	3	0	4	0	7	700	0	707	1151
% Approach Total	98.6	0.0	1.4	0.0		100.0	0.0	0.0	0.0		0.0	25.0	75.0	0.0		0.0	1.0	99.0	0.0		
PHF	0.866	0.000	0.750	0.000	0.871	0.250	0.000	0.000	0.000	0.250	0.000	0.250	0.750	0.000	0.500	0.000	0.438	0.941	0.000	0.945	0.959
Cars	425	0	6	0	431	1	0	0	0	1	0	1	3	0	4	0	7	693	0	700	1136
Cars %	98.2	0.0	100.0	0.0	98.2	100.0	0.0	0.0	0.0	100.0	0.0	100.0	100.0	0.0	100.0	0.0	100.0	99.0	0.0	99.0	98.7
Heavy Vehicles	8	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	15
Heavy Vehicles %	1.8	0.0	0.0	0.0	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	0.0	1.0	1.3
Cars Enter Leg	425	0	6	0	431	1	0	0	0	1	0	1	3	0	4	0	7	693	0	700	1136
Heavy Enter Leg	8	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	15
Total Entering Leg	433	0	6	0	439	1	0	0	0	1	0	1	3	0	4	0	7	700	0	707	1151
Cars Exiting Leg	695					13					0					428					1136
Heavy Exiting Leg	7					0					0					8					15
Total Exiting Leg	702					13					0					436					1151

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Cars

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	111	0	0	0	111	0	0	0	0	0	0	0	0	0	0	0	2	149	0	151	262
4:15 PM	117	0	0	0	117	0	0	0	0	0	0	1	2	0	3	0	1	159	0	160	280
4:30 PM	101	0	2	0	103	0	0	0	0	0	0	0	1	0	1	0	4	164	0	168	272
4:45 PM	122	0	1	0	123	0	0	0	0	0	0	0	0	0	0	0	1	162	0	163	286
Total	451	0	3	0	454	0	0	0	0	0	0	1	3	0	4	0	8	634	0	642	1100
5:00 PM	107	0	2	0	109	1	0	0	0	1	0	1	1	0	2	0	1	185	0	186	298
5:15 PM	95	0	1	0	96	0	0	0	0	0	0	0	1	0	1	0	1	182	0	183	280
5:30 PM	112	0	1	0	113	0	0	0	0	0	0	2	1	0	3	0	1	153	0	154	270
5:45 PM	104	0	2	0	106	0	0	0	0	0	0	1	1	0	2	0	2	155	0	157	265
Total	418	0	6	0	424	1	0	0	0	1	0	4	4	0	8	0	5	675	0	680	1113
Grand Total	869	0	9	0	878	1	0	0	0	1	0	5	7	0	12	0	13	1309	0	1322	2213
Approach %	99.0	0.0	1.0	0.0		100.0	0.0	0.0	0.0	0.0	0.0	41.7	58.3	0.0		0.0	1.0	99.0	0.0		
Total %	39.3	0.0	0.4	0.0	39.7	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.3	0.0	0.5	0.0	0.6	59.2	0.0	59.7	
Exiting Leg Total	1315					22					0					876					2213

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:15 PM	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:15 PM	117	0	0	0	117	0	0	0	0	0	0	1	2	0	3	0	1	159	0	160	280
4:30 PM	101	0	2	0	103	0	0	0	0	0	0	0	1	0	1	0	4	164	0	168	272
4:45 PM	122	0	1	0	123	0	0	0	0	0	0	0	0	0	0	0	1	162	0	163	286
5:00 PM	107	0	2	0	109	1	0	0	0	1	0	1	1	0	2	0	1	185	0	186	298
Total Volume	447	0	5	0	452	1	0	0	0	1	0	2	4	0	6	0	7	670	0	677	1136
% Approach Total	98.9	0.0	1.1	0.0		100.0	0.0	0.0	0.0		0.0	33.3	66.7	0.0		0.0	1.0	99.0	0.0		
PHF	0.916	0.000	0.625	0.000	0.919	0.250	0.000	0.000	0.000	0.250	0.000	0.500	0.500	0.000	0.500	0.000	0.438	0.905	0.000	0.910	0.953
Entering Leg	447	0	5	0	452	1	0	0	0	1	0	2	4	0	6	0	7	670	0	677	1136
Exiting Leg	673					12					0					451					1136
Total	1125					13					6					1128					2272

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class: **Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)**



	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	6
4:15 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
4:30 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	3
4:45 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Total	9	0	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0	8	0	8	17
5:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
5:15 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	3
5:45 PM	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	5
Total	8	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	15
Grand Total	17	0	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0	15	0	15	32
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
Total %	53.1	0.0	0.0	0.0	53.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	46.9	0.0	46.9	
Exiting Leg Total	15					0					0					17					32
Buses	10	0	0	0	10	0	0	0	0	0	0	0	0	0	0	0	0	11	0	11	21
% Buses	58.8	0.0	0.0	0.0	58.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	73.3	0.0	73.3	65.6
Exiting Leg Total	11					0					0					10					21
Single-Unit Trucks	5	0	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	9
% Single-Unit	29.4	0.0	0.0	0.0	29.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	26.7	0.0	26.7	28.1
Exiting Leg Total	4					0					0					5					9
Articulated Trucks	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
% Articulated	11.8	0.0	0.0	0.0	11.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.3
Exiting Leg Total	0					0					0					2					2

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	6
4:15 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
4:30 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	3
4:45 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Total Volume	9	0	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0	8	0	8	17
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
PHF	0.750	0.000	0.000	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.667	0.000	0.667	0.708
Buses	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	10
Buses %	44.4	0.0	0.0	0.0	44.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	75.0	0.0	75.0	58.8
Single-Unit Trucks	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Single-Unit %	33.3	0.0	0.0	0.0	33.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	25.0	0.0	25.0	29.4
Articulated Trucks	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Articulated %	22.2	0.0	0.0	0.0	22.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	11.8
Buses	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	10
Single-Unit Trucks	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Articulated Trucks	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Total Entering Leg	9	0	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0	8	0	8	17
Buses	6					0					0					4					10
Single-Unit Trucks	2					0					0					3					5
Articulated Trucks	0					0					0					2					2
Total Exiting Leg	8					0					0					9					17

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Buses

	Main Street					#19 Driveway					#21 Driveway					Main Street						Total
	from East					from South					from Southwest					from West						
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total		
4:00 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	4	
4:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
4:30 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	3	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2	
Total	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	10	
5:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:15 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	4	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2	
5:45 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	4	
Total	6	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	11	
Grand Total	10	0	0	0	10	0	0	0	0	0	0	0	0	0	0	0	0	11	0	11	21	
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0			
Total %	47.6	0.0	0.0	0.0	47.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	52.4	0.0	52.4		
Exiting Leg Total	11					0					0					10					21	

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

5:00 PM	Main Street					#19 Driveway					#21 Driveway					Main Street					
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
5:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:15 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	4
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
5:45 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	4
Total Volume	6	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	11
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
PHF	0.500	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.625	0.000	0.625	0.688
Entering Leg	6	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	11
Exiting Leg					5										0					6	11
Total					11					0				0						11	22

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Single-Unit Trucks

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total	
	from East					from South					from Southwest					from West						
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total		
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
4:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Total	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	
5:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	
5:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	4	
Grand Total	5	0	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	9	
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0			
Total %	55.6	0.0	0.0	0.0	55.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	44.4	0.0	44.4		
Exiting Leg Total	4					0					0					5					9	

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
4:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Total Volume	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
PHF	0.375	0.000	0.000	0.000	0.375	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.500	0.000	0.500	0.625
Entering Leg	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Exiting Leg	2					0					0					3					5
Total	5					0					0					5					10

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Articulated Trucks

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Total %	100.0	0.0	0.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Exiting Leg Total	0					0					0					2					2

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Main Street					#19 Driveway					#21 Driveway					Main Street					
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
4:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Total Volume	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
PHF	0.500	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.500
Entering Leg	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Exiting Leg	0					0					0					2					2
Total	2					0					0					2					4

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**



Bicycles (on Roadway and Crosswalks)

	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
4:00 PM	2	0	0	0	0	0	2	0	0	0	0	0	3	3	0	0	0	0	0	3	3	0	0	0	0	0	0	0	8
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
4:45 PM	1	0	0	0	0	0	1	0	0	0	0	0	1	1	0	0	0	0	0	1	1	0	0	0	0	0	0	3	
Total	4	0	0	0	0	0	4	0	0	0	0	0	4	4	0	0	0	0	0	4	4	0	0	0	0	0	0	12	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	1	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	0	0	1	2	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1	
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2	0	0	2	0	0	2	4	
Grand Total	4	0	0	0	0	0	4	0	0	0	0	0	4	4	0	0	0	0	1	5	6	0	0	2	0	0	2	16	
Approach %	100.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	100.0		0.0	0.0	0.0	0.0	16.7	83.3		0.0	0.0	100.0	0.0	0.0	0.0		
Total %	25.0	0.0	0.0	0.0	0.0	0.0	25.0	0.0	0.0	0.0	0.0	0.0	25.0	25.0	0.0	0.0	0.0	0.0	6.3	31.3	37.5	0.0	0.0	12.5	0.0	0.0	0.0	12.5	
Exiting Leg Total	2							4							6							4							16

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
4:00 PM	2	0	0	0	0	0	2	0	0	0	0	0	3	3	0	0	0	0	0	3	3	0	0	0	0	0	0	0	8
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
4:45 PM	1	0	0	0	0	0	1	0	0	0	0	0	1	1	0	0	0	0	0	1	1	0	0	0	0	0	0	3	
Total Volume	4	0	0	0	0	0	4	0	0	0	0	0	4	4	0	0	0	0	0	4	4	0	0	0	0	0	0	12	
% Approach Total	100.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	100.0		0.0	0.0	0.0	0.0	0.0	100.0		0.0	0.0	0.0	0.0	0.0	0.0		
PHF	0.500	0.000	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.333	0.333	0.000	0.000	0.000	0.000	0.000	0.333	0.333	0.000	0.000	0.000	0.000	0.000	0.000	0.375	
Entering Leg	4	0	0	0	0	0	4	0	0	0	0	0	4	4	0	0	0	0	0	4	4	0	0	0	0	0	0	12	
Exiting Leg	0							4							4							4							12
Total	4							8							8							4							24

PDI File #: **197243 B**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Thursday, October 24, 2019**
 Start Time: **4:00 PM**
 End Time: **6:00 PM**
 Class:



Pedestrians

	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	11	14	25	0	0	0	0	11	13	24	0	0	0	0	0	0	0	49
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	12	2	14	0	0	0	0	13	3	16	0	0	0	0	0	0	0	30
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	14	9	23	0	0	0	0	15	9	24	0	0	0	0	1	0	1	48
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	6	7	13	0	0	0	0	7	12	19	0	0	0	0	0	0	0	32
Total	0	0	0	0	0	0	0	0	0	0	0	43	32	75	0	0	0	0	46	37	83	0	0	0	0	1	0	1	159
5:00 PM	0	0	0	0	1	0	1	0	0	0	0	3	11	14	0	0	0	0	3	15	18	0	0	0	0	0	0	0	33
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	8	11	19	0	0	0	0	8	12	20	0	0	0	0	1	0	1	40
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	2	5	7	0	0	0	0	2	5	7	0	0	0	0	0	0	0	14
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	2	5	7	0	0	0	0	1	5	6	0	0	0	0	0	0	0	13
Total	0	0	0	0	1	0	1	0	0	0	0	15	32	47	0	0	0	0	14	37	51	0	0	0	0	1	0	1	100
Grand Total	0	0	0	0	1	0	1	0	0	0	0	58	64	122	0	0	0	0	60	74	134	0	0	0	0	2	0	2	259
Approach %	0	0	0	0	100	0		0	0	0	0	47.5	52.5		0	0	0	0	44.8	55.2		0	0	0	0	100	0		
Total %	0	0	0	0	0.39	0	0.39	0	0	0	0	22.4	24.7	47.1	0	0	0	0	23.2	28.6	51.7	0	0	0	0	0.77	0	0.77	
Exiting Leg Total	1							122							134							2							259

Peak Hour Analysis from 04:00 PM to 06:00 PM begins at:

4:00 PM	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	11	14	25	0	0	0	0	11	13	24	0	0	0	0	0	0	0	49
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	12	2	14	0	0	0	0	13	3	16	0	0	0	0	0	0	0	30
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	14	9	23	0	0	0	0	15	9	24	0	0	0	0	1	0	1	48
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	6	7	13	0	0	0	0	7	12	19	0	0	0	0	0	0	0	32
Total Volume	0	0	0	0	0	0	0	0	0	0	0	43	32	75	0	0	0	0	46	37	83	0	0	0	0	1	0	1	159
% Approach Total	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	57.3	42.7		0.0	0.0	0.0	0.0	55.4	44.6		0.0	0.0	0.0	0.0	100.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.768	0.571	0.750	0.000	0.000	0.000	0.000	0.767	0.712	0.865	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.811
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	43	32	75	0	0	0	0	46	37	83	0	0	0	0	1	0	1	159
Exiting Leg	0							75							83							1							159
Total	0							150							166							2							318

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	108	0	2	0	110	0	0	0	0	0	0	1	0	0	1	0	0	130	0	130	241
11:45 AM	116	0	0	0	116	1	0	0	0	1	0	0	1	0	1	1	1	148	0	150	268
Total	224	0	2	0	226	1	0	0	0	1	0	1	1	0	2	1	1	278	0	280	509
12:00 PM	125	0	2	0	127	1	0	0	0	1	0	2	0	0	2	0	3	187	0	190	320
12:15 PM	114	0	4	0	118	0	0	0	0	0	0	3	1	0	4	0	1	157	0	158	280
12:30 PM	122	0	1	0	123	0	0	0	0	0	0	1	4	0	5	0	0	157	0	157	285
12:45 PM	77	0	0	0	77	0	0	0	0	0	0	0	0	0	0	0	0	157	0	157	234
Total	438	0	7	0	445	1	0	0	0	1	0	6	5	0	11	0	4	658	0	662	1119
1:00 PM	109	0	1	0	110	0	0	0	0	0	0	3	1	0	4	0	0	128	0	128	242
1:15 PM	118	0	0	0	118	0	0	0	0	0	0	1	0	0	1	0	0	125	0	125	244
Total	227	0	1	0	228	0	0	0	0	0	0	4	1	0	5	0	0	253	0	253	486
Grand Total	889	0	10	0	899	2	0	0	0	2	0	11	7	0	18	1	5	1189	0	1195	2114
Approach %	98.9	0.0	1.1	0.0		100.0	0.0	0.0	0.0		0.0	61.1	38.9	0.0		0.1	0.4	99.5	0.0		
Total %	42.1	0.0	0.5	0.0	42.5	0.1	0.0	0.0	0.0	0.1	0.0	0.5	0.3	0.0	0.9	0.0	0.2	56.2	0.0	56.5	
Exiting Leg Total	1202					15					1					896					2114
Cars	877	0	10	0	887	2	0	0	0	2	0	11	7	0	18	1	5	1177	0	1183	2090
% Cars	98.7	0.0	100.0	0.0	98.7	100.0	0.0	0.0	0.0	100.0	0.0	100.0	100.0	0.0	100.0	100.0	100.0	99.0	0.0	99.0	98.9
Exiting Leg Total	1190					15					1					884					2090
Heavy Vehicles	12	0	0	0	12	0	0	0	0	0	0	0	0	0	0	0	0	12	0	12	24
% Heavy Vehicles	1.3	0.0	0.0	0.0	1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.0	0.0	1.0	1.1
Exiting Leg Total	12					0					0					12					24

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:45 AM	116	0	0	0	116	1	0	0	0	1	0	0	1	0	1	1	1	148	0	150	268
12:00 PM	125	0	2	0	127	1	0	0	0	1	0	2	0	0	2	0	3	187	0	190	320
12:15 PM	114	0	4	0	118	0	0	0	0	0	0	3	1	0	4	0	1	157	0	158	280
12:30 PM	122	0	1	0	123	0	0	0	0	0	0	1	4	0	5	0	0	157	0	157	285
Total Volume	477	0	7	0	484	2	0	0	0	2	0	6	6	0	12	1	5	649	0	655	1153
% Approach Total	98.6	0.0	1.4	0.0		100.0	0.0	0.0	0.0		0.0	50.0	50.0	0.0		0.2	0.8	99.1	0.0		
PHF	0.954	0.000	0.438	0.000	0.953	0.500	0.000	0.000	0.000	0.500	0.000	0.500	0.375	0.000	0.600	0.250	0.417	0.868	0.000	0.862	0.901
Cars	473	0	7	0	480	2	0	0	0	2	0	6	6	0	12	1	5	642	0	648	1142
Cars %	99.2	0.0	100.0	0.0	99.2	100.0	0.0	0.0	0.0	100.0	0.0	100.0	100.0	0.0	100.0	100.0	100.0	98.9	0.0	98.9	99.0
Heavy Vehicles	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	11
Heavy Vehicles %	0.8	0.0	0.0	0.0	0.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1	0.0	1.1	1.0
Cars Enter Leg	473	0	7	0	480	2	0	0	0	2	0	6	6	0	12	1	5	642	0	648	1142
Heavy Enter Leg	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	11
Total Entering Leg	477	0	7	0	484	2	0	0	0	2	0	6	6	0	12	1	5	649	0	655	1153
Cars Exiting Leg	650					12					1					479					1142
Heavy Exiting Leg	7					0					0					4					11
Total Exiting Leg	657					12					1					483					1153

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	105	0	2	0	107	0	0	0	0	0	0	1	0	0	1	0	0	129	0	129	237
11:45 AM	115	0	0	0	115	1	0	0	0	1	0	0	1	0	1	1	1	145	0	147	264
Total	220	0	2	0	222	1	0	0	0	1	0	1	1	0	2	1	1	274	0	276	501
12:00 PM	123	0	2	0	125	1	0	0	0	1	0	2	0	0	2	0	3	184	0	187	315
12:15 PM	113	0	4	0	117	0	0	0	0	0	0	3	1	0	4	0	1	157	0	158	279
12:30 PM	122	0	1	0	123	0	0	0	0	0	0	1	4	0	5	0	0	156	0	156	284
12:45 PM	76	0	0	0	76	0	0	0	0	0	0	0	0	0	0	0	0	155	0	155	231
Total	434	0	7	0	441	1	0	0	0	1	0	6	5	0	11	0	4	652	0	656	1109
1:00 PM	107	0	1	0	108	0	0	0	0	0	0	3	1	0	4	0	0	127	0	127	239
1:15 PM	116	0	0	0	116	0	0	0	0	0	0	1	0	0	1	0	0	124	0	124	241
Total	223	0	1	0	224	0	0	0	0	0	0	4	1	0	5	0	0	251	0	251	480
Grand Total	877	0	10	0	887	2	0	0	0	2	0	11	7	0	18	1	5	1177	0	1183	2090
Approach %	98.9	0.0	1.1	0.0		100.0	0.0	0.0	0.0		0.0	61.1	38.9	0.0		0.1	0.4	99.5	0.0		
Total %	42.0	0.0	0.5	0.0	42.4	0.1	0.0	0.0	0.0	0.1	0.0	0.5	0.3	0.0	0.9	0.0	0.2	56.3	0.0	56.6	
Exiting Leg Total	1190					15					1					884					2090

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:45 AM	Main Street					#19 Driveway					#21 Driveway					Main Street					
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:45 AM	115	0	0	0	115	1	0	0	0	1	0	0	1	0	1	1	1	145	0	147	264
12:00 PM	123	0	2	0	125	1	0	0	0	1	0	2	0	0	2	0	3	184	0	187	315
12:15 PM	113	0	4	0	117	0	0	0	0	0	0	3	1	0	4	0	1	157	0	158	279
12:30 PM	122	0	1	0	123	0	0	0	0	0	0	1	4	0	5	0	0	156	0	156	284
Total Volume	473	0	7	0	480	2	0	0	0	2	0	6	6	0	12	1	5	642	0	648	1142
% Approach Total	98.5	0.0	1.5	0.0		100.0	0.0	0.0	0.0		0.0	50.0	50.0	0.0		0.2	0.8	99.1	0.0		
PHF	0.961	0.000	0.438	0.000	0.960	0.500	0.000	0.000	0.000	0.500	0.000	0.500	0.375	0.000	0.600	0.250	0.417	0.872	0.000	0.866	0.906
Entering Leg	473	0	7	0	480	2	0	0	0	2	0	6	6	0	12	1	5	642	0	648	1142
Exiting Leg	650					12					1					479					1142
Total	1130					14					13					1127					2284

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class: **Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)**



	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	4
11:45 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	4
Total	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	8
12:00 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	5
12:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
12:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	3
Total	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	10
1:00 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
1:15 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
Total	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	6
Grand Total	12	0	0	0	12	0	0	0	0	0	0	0	0	0	0	0	0	12	0	12	24
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
Total %	50.0	0.0	0.0	0.0	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	50.0	0.0	50.0	
Exiting Leg Total	12					0					0					12					24
Buses	5	0	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	9
% Buses	41.7	0.0	0.0	0.0	41.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	33.3	0.0	33.3	37.5
Exiting Leg Total	4					0					0					5					9
Single-Unit Trucks	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	8	0	8	15
% Single-Unit	58.3	0.0	0.0	0.0	58.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	66.7	0.0	66.7	62.5
Exiting Leg Total	8					0					0					7					15
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Exiting Leg Total	0					0					0					0					0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	4
11:45 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	4
12:00 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	5
12:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Total Volume	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	14
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
PHF	0.583	0.000	0.000	0.000	0.583	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.583	0.000	0.583	0.700
Buses	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Buses %	42.9	0.0	0.0	0.0	42.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	28.6	0.0	28.6	35.7
Single-Unit Trucks	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	9
Single-Unit %	57.1	0.0	0.0	0.0	57.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	71.4	0.0	71.4	64.3
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Buses	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Single-Unit Trucks	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	9
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Entering Leg	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	14
Buses	2					0					0					3					5
Single-Unit Trucks	5					0					0					4					9
Articulated Trucks	0					0					0					0					0
Total Exiting Leg	7					0					0					7					14

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Buses

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	3
12:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	5
1:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
1:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
Grand Total	5	0	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	9
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
Total %	55.6	0.0	0.0	0.0	55.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	44.4	0.0	44.4	
Exiting Leg Total	4					0					0					5					9

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Main Street					#19 Driveway					#21 Driveway					Main Street					
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	3
12:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Total Volume	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
PHF	0.750	0.000	0.000	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.417
Entering Leg	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5
Exiting Leg	2					0					0					3					5
Total	5					0					0					5					10

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Single-Unit Trucks

	Main Street					#19 Driveway					#21 Driveway					Main Street					Total
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
11:45 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	4
Total	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	7
12:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
12:45 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	5
1:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
1:15 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
Total	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
Grand Total	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	8	0	8	15
Approach %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
Total %	46.7	0.0	0.0	0.0	46.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	53.3	0.0	53.3	
Exiting Leg Total	8					0					0					7					15

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Main Street					#19 Driveway					#21 Driveway					Main Street					
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	
11:30 AM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3
11:45 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	4
12:00 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	9
% Approach Total	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		
PHF	0.500	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.417	0.000	0.417	0.563
Entering Leg	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	9
Exiting Leg	5					0					0					4					9
Total	9					0					0					9					18

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Articulated Trucks

	Main Street					#19 Driveway					#21 Driveway					Main Street						Total
	from East					from South					from Southwest					from West						
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total		
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0			
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Exiting Leg Total	0					0					0					0						0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Main Street					#19 Driveway					#21 Driveway					Main Street					
	from East					from South					from Southwest					from West					
	Thru	Bear Left	Left	U-Turn	Total	Right	Left	Hard Left	U-Turn	Total	Hard Right	Bear Right	Hard Left	U-Turn	Total	Hard Right	Right	Thru	U-Turn	Total	Total
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exiting Leg	0					0					0					0					0
Total	0					0					0					0					0

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Bicycles (on Roadway and Crosswalks)

	Main Street							#19 Driveway							#21 Driveway							Main Street							Total	
	from East							from South							from Southwest							from West								
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total		
11:30 AM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	2
11:45 AM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	4
Total	2	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	3	0	0	0	4	6
12:00 PM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	2
1:00 PM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1	1	2	0	0	1	0	0	0	0	1	4
Total	1	0	0	0	0	0	1	0	0	0	0	1	0	1	0	0	0	0	1	1	2	0	0	1	0	0	0	0	1	5
Grand Total	4	0	0	0	0	0	4	0	0	0	0	1	0	1	0	0	0	0	1	1	2	0	1	5	0	0	0	0	6	13
Approach %	100.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0	50.0	50.0		0.0	16.7	83.3	0.0	0.0	0.0			
Total %	30.8	0.0	0.0	0.0	0.0	0.0	30.8	0.0	0.0	0.0	0.0	7.7	0.0	7.7	0.0	0.0	0.0	0.0	7.7	7.7	15.4	0.0	7.7	38.5	0.0	0.0	0.0	46.2		
Exiting Leg Total	5							2							2							4							13	

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	2
11:45 AM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	4
12:00 PM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
Total Volume	3	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	4	0	0	5	8
% Approach Total	100.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0	20.0	80.0	0.0	0.0	0.0		
PHF	0.750	0.000	0.000	0.000	0.000	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.333	0.000	0.000	0.000	0.417	0.500
Entering Leg	3	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	4	0	0	5	8
Exiting Leg	4							1							0							3							8
Total	7							1							0							8							16

PDI File #: **197243 BB**
 Location: **S: #19 Driveway**
 Location: **E: Main Street W: Main Street SW: #21 Driveway**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Pedestrians

	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	4	0	4	0	0	0	0	4	0	4	0	0	0	0	0	0	0	8
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	1	5	6	0	0	0	0	1	5	6	0	0	0	0	0	0	0	12
Total	0	0	0	0	0	0	0	0	0	0	0	5	5	10	0	0	0	0	5	5	10	0	0	0	0	0	0	0	20
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	12	5	17	0	0	0	0	10	4	14	0	0	0	0	0	0	0	31
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	1	2	0	0	0	0	1	1	2	0	0	0	0	0	0	0	4
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	7	4	11	0	0	0	0	10	7	17	0	0	0	0	0	0	0	28
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	4	8	12	0	0	0	0	4	10	14	0	0	0	0	0	0	0	26
Total	0	0	0	0	0	0	0	0	0	0	0	24	18	42	0	0	0	0	25	22	47	0	0	0	0	0	0	0	89
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	3	6	9	0	0	0	0	5	7	12	0	0	0	0	0	0	0	21
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	5	6	11	0	0	0	0	5	9	14	0	0	0	0	0	0	0	25
Total	0	0	0	0	0	0	0	0	0	0	0	8	12	20	0	0	0	0	10	16	26	0	0	0	0	0	0	0	46
Grand Total	0	0	0	0	0	0	0	0	0	0	0	37	35	72	0	0	0	0	40	43	83	0	0	0	0	0	0	0	155
Approach %	0	0	0	0	0	0	0	0	0	0	0	51.4	48.6		0	0	0	0	48.2	51.8		0	0	0	0	0	0		
Total %	0	0	0	0	0	0	0	0	0	0	0	23.9	22.6	46.5	0	0	0	0	25.8	27.7	53.5	0	0	0	0	0	0		
Exiting Leg Total	0							72							83							0							155

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:30 PM	Main Street							#19 Driveway							#21 Driveway							Main Street							Total
	from East							from South							from Southwest							from West							
	Thru	Bear Left	Left	U-Turn	CW-SB	CW-NB	Total	Right	Left	Hard Left	U-Turn	CW-WB	CW-EB	Total	Hard Right	Bear Right	Hard Left	U-Turn	CW-NWB	CW-SEB	Total	Hard Right	Right	Thru	U-Turn	CW-NB	CW-SB	Total	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	7	4	11	0	0	0	0	10	7	17	0	0	0	0	0	0	0	28
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	4	8	12	0	0	0	0	4	10	14	0	0	0	0	0	0	0	26
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	3	6	9	0	0	0	0	5	7	12	0	0	0	0	0	0	0	21
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	5	6	11	0	0	0	0	5	9	14	0	0	0	0	0	0	0	25
Total Volume	0	0	0	0	0	0	0	0	0	0	0	19	24	43	0	0	0	0	24	33	57	0	0	0	0	0	0	0	100
% Approach Total	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	44.2	55.8		0.0	0.0	0.0	0.0	42.1	57.9		0.0	0.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.679	0.750	0.896	0.000	0.000	0.000	0.000	0.600	0.825	0.838	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.893
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	19	24	43	0	0	0	0	24	33	57	0	0	0	0	0	0	0	100
Exiting Leg	0							43							57							0							100
Total	0							86							114							0							200

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars and Heavy Vehicles (Combined)

	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	63	81	0	144	97	0	48	1	146	41	85	0	0	126	416
11:45 AM	0	0	1	0	1	1	61	82	0	144	73	0	50	0	123	41	96	0	0	137	405
Total	0	0	1	0	1	1	124	163	0	288	170	0	98	1	269	82	181	0	0	263	821
12:00 PM	0	0	2	0	2	0	84	67	0	151	65	0	39	0	104	46	130	0	0	176	433
12:15 PM	0	1	0	0	1	0	71	87	0	158	79	0	41	0	120	55	102	0	0	157	436
12:30 PM	0	3	1	0	4	1	80	118	0	199	79	0	39	0	118	49	111	0	0	160	481
12:45 PM	0	8	0	0	8	1	45	96	0	142	76	2	30	0	108	57	95	1	0	153	411
Total	0	12	3	0	15	2	280	368	0	650	299	2	149	0	450	207	438	1	0	646	1761
1:00 PM	0	6	1	0	7	0	63	76	0	139	73	0	47	1	121	29	99	0	0	128	395
1:15 PM	0	1	1	0	2	0	77	93	0	170	73	0	41	0	114	33	91	0	0	124	410
Total	0	7	2	0	9	0	140	169	0	309	146	0	88	1	235	62	190	0	0	252	805
Grand Total	0	19	6	0	25	3	544	700	0	1247	615	2	335	2	954	351	809	1	0	1161	3387
Approach %	0.0	76.0	24.0	0.0		0.2	43.6	56.1	0.0		64.5	0.2	35.1	0.2		30.2	69.7	0.1	0.0		
Total %	0.0	0.6	0.2	0.0	0.7	0.1	16.1	20.7	0.0	36.8	18.2	0.1	9.9	0.1	28.2	10.4	23.9	0.0	0.0	34.3	
Exiting Leg Total	6					1430					1072					879					3387
Cars	0	19	6	0	25	3	535	697	0	1235	612	2	331	2	947	348	800	1	0	1149	3356
% Cars	0.0	100.0	100.0	0.0	100.0	100.0	98.3	99.6	0.0	99.0	99.5	100.0	98.8	100.0	99.3	99.1	98.9	100.0	0.0	99.0	99.1
Exiting Leg Total	6					1418					1066					866					3356
Heavy Vehicles	0	0	0	0	0	0	9	3	0	12	3	0	4	0	7	3	9	0	0	12	31
% Heavy Vehicles	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.4	0.0	1.0	0.5	0.0	1.2	0.0	0.7	0.9	1.1	0.0	0.0	1.0	0.9
Exiting Leg Total	0					12					6					13					31

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:00 PM	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
12:00 PM	0	0	2	0	2	0	84	67	0	151	65	0	39	0	104	46	130	0	0	176	433
12:15 PM	0	1	0	0	1	0	71	87	0	158	79	0	41	0	120	55	102	0	0	157	436
12:30 PM	0	3	1	0	4	1	80	118	0	199	79	0	39	0	118	49	111	0	0	160	481
12:45 PM	0	8	0	0	8	1	45	96	0	142	76	2	30	0	108	57	95	1	0	153	411
Total Volume	0	12	3	0	15	2	280	368	0	650	299	2	149	0	450	207	438	1	0	646	1761
% Approach Total	0.0	80.0	20.0	0.0		0.3	43.1	56.6	0.0		66.4	0.4	33.1	0.0		32.0	67.8	0.2	0.0		
PHF	0.000	0.375	0.375	0.000	0.469	0.500	0.833	0.780	0.000	0.817	0.946	0.250	0.909	0.000	0.938	0.908	0.842	0.250	0.000	0.918	0.915
Cars	0	12	3	0	15	2	277	365	0	644	298	2	148	0	448	206	434	1	0	641	1748
Cars %	0.0	100.0	100.0	0.0	100.0	100.0	98.9	99.2	0.0	99.1	99.7	100.0	99.3	0.0	99.6	99.5	99.1	100.0	0.0	99.2	99.3
Heavy Vehicles	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	1	4	0	0	5	13
Heavy Vehicles %	0.0	0.0	0.0	0.0	0.0	0.0	1.1	0.8	0.0	0.9	0.3	0.0	0.7	0.0	0.4	0.5	0.9	0.0	0.0	0.8	0.7
Cars Enter Leg	0	12	3	0	15	2	277	365	0	644	298	2	148	0	448	206	434	1	0	641	1748
Heavy Enter Leg	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	1	4	0	0	5	13
Total Entering Leg	0	12	3	0	15	2	280	368	0	650	299	2	149	0	450	207	438	1	0	646	1761
Cars Exiting Leg	5					735					583					425					1748
Heavy Exiting Leg	0					5					4					4					13
Total Exiting Leg	5					740					587					429					1761

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Cars

	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	60	81	0	141	97	0	48	1	146	40	85	0	0	125	412
11:45 AM	0	0	1	0	1	1	61	82	0	144	73	0	49	0	122	40	93	0	0	133	400
Total	0	0	1	0	1	1	121	163	0	285	170	0	97	1	268	80	178	0	0	258	812
12:00 PM	0	0	2	0	2	0	83	67	0	150	65	0	38	0	103	46	128	0	0	174	429
12:15 PM	0	1	0	0	1	0	70	84	0	154	78	0	41	0	119	55	101	0	0	156	430
12:30 PM	0	3	1	0	4	1	80	118	0	199	79	0	39	0	118	48	111	0	0	159	480
12:45 PM	0	8	0	0	8	1	44	96	0	141	76	2	30	0	108	57	94	1	0	152	409
Total	0	12	3	0	15	2	277	365	0	644	298	2	148	0	448	206	434	1	0	641	1748
1:00 PM	0	6	1	0	7	0	61	76	0	137	73	0	46	1	120	29	99	0	0	128	392
1:15 PM	0	1	1	0	2	0	76	93	0	169	71	0	40	0	111	33	89	0	0	122	404
Total	0	7	2	0	9	0	137	169	0	306	144	0	86	1	231	62	188	0	0	250	796
Grand Total	0	19	6	0	25	3	535	697	0	1235	612	2	331	2	947	348	800	1	0	1149	3356
Approach %	0.0	76.0	24.0	0.0		0.2	43.3	56.4	0.0		64.6	0.2	35.0	0.2		30.3	69.6	0.1	0.0		
Total %	0.0	0.6	0.2	0.0	0.7	0.1	15.9	20.8	0.0	36.8	18.2	0.1	9.9	0.1	28.2	10.4	23.8	0.0	0.0	34.2	
Exiting Leg Total	6					1418					1066					866					3356

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:00 PM	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
12:00 PM	0	0	2	0	2	0	83	67	0	150	65	0	38	0	103	46	128	0	0	174	429
12:15 PM	0	1	0	0	1	0	70	84	0	154	78	0	41	0	119	55	101	0	0	156	430
12:30 PM	0	3	1	0	4	1	80	118	0	199	79	0	39	0	118	48	111	0	0	159	480
12:45 PM	0	8	0	0	8	1	44	96	0	141	76	2	30	0	108	57	94	1	0	152	409
Total Volume	0	12	3	0	15	2	277	365	0	644	298	2	148	0	448	206	434	1	0	641	1748
% Approach Total	0.0	80.0	20.0	0.0		0.3	43.0	56.7	0.0		66.5	0.4	33.0	0.0		32.1	67.7	0.2	0.0		
PHF	0.000	0.375	0.375	0.000	0.469	0.500	0.834	0.773	0.000	0.809	0.943	0.250	0.902	0.000	0.941	0.904	0.848	0.250	0.000	0.921	0.910
Entering Leg	0	12	3	0	15	2	277	365	0	644	298	2	148	0	448	206	434	1	0	641	1748
Exiting Leg	5					735					583					425					1748
Total	20					1379					1031					1066					3496

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**



Heavy Vehicles-Combined (Buses, Single-Unit Trucks, Articulated Trucks)

	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	1	0	0	0	1	4
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	3	0	0	4	5
Total	0	0	0	0	0	0	3	0	0	3	0	0	1	0	1	2	3	0	0	5	9
12:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0	2	0	0	2	4
12:15 PM	0	0	0	0	0	0	1	3	0	4	1	0	0	0	1	0	1	0	0	1	6
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
12:45 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2
Total	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	1	4	0	0	5	13
1:00 PM	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	0	0	0	0	0	3
1:15 PM	0	0	0	0	0	0	1	0	0	1	2	0	1	0	3	0	2	0	0	2	6
Total	0	0	0	0	0	0	3	0	0	3	2	0	2	0	4	0	2	0	0	2	9
Grand Total	0	0	0	0	0	0	9	3	0	12	3	0	4	0	7	3	9	0	0	12	31
Approach %	0.0	0.0	0.0	0.0		0.0	75.0	25.0	0.0		42.9	0.0	57.1	0.0		25.0	75.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	29.0	9.7	0.0	38.7	9.7	0.0	12.9	0.0	22.6	9.7	29.0	0.0	0.0	38.7	
Exiting Leg Total	0					12					6					13					31
Buses	0	0	0	0	0	0	4	0	0	4	0	0	1	0	1	0	4	0	0	4	9
% Buses	0.0	0.0	0.0	0.0	0.0	0.0	44.4	0.0	0.0	33.3	0.0	0.0	25.0	0.0	14.3	0.0	44.4	0.0	0.0	33.3	29.0
Exiting Leg Total	0					4					0					5					9
Single-Unit Trucks	0	0	0	0	0	0	5	3	0	8	3	0	3	0	6	3	5	0	0	8	22
% Single-Unit	0.0	0.0	0.0	0.0	0.0	0.0	55.6	100.0	0.0	66.7	100.0	0.0	75.0	0.0	85.7	100.0	55.6	0.0	0.0	66.7	71.0
Exiting Leg Total	0					8					6					8					22
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Articulated	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Exiting Leg Total	0					0					0					0					0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	1	0	0	0	1	4
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	3	0	0	4	5
12:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0	2	0	0	2	4
12:15 PM	0	0	0	0	0	0	1	3	0	4	1	0	0	0	1	0	1	0	0	1	6
Total Volume	0	0	0	0	0	0	5	3	0	8	1	0	2	0	3	2	6	0	0	8	19
% Approach Total	0.0	0.0	0.0	0.0		0.0	62.5	37.5	0.0		33.3	0.0	66.7	0.0		25.0	75.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.417	0.250	0.000	0.500	0.250	0.000	0.500	0.000	0.750	0.500	0.500	0.000	0.000	0.500	0.792
Buses	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	0	2	0	0	2	5
Buses %	0.0	0.0	0.0	0.0	0.0	0.0	40.0	0.0	0.0	25.0	0.0	0.0	50.0	0.0	33.3	0.0	33.3	0.0	0.0	25.0	26.3
Single-Unit Trucks	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	2	4	0	0	6	14
Single-Unit %	0.0	0.0	0.0	0.0	0.0	0.0	60.0	100.0	0.0	75.0	100.0	0.0	50.0	0.0	66.7	100.0	66.7	0.0	0.0	75.0	73.7
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Articulated %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Buses	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	0	2	0	0	2	5
Single-Unit Trucks	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	2	4	0	0	6	14
Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Entering Leg	0	0	0	0	0	0	5	3	0	8	1	0	2	0	3	2	6	0	0	8	19
Buses	0					2					0					3					5
Single-Unit Trucks	0					5					5					4					14
Articulated Trucks	0					0					0					0					0
Total Exiting Leg	0					7					5					7					19

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Buses

	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	0	1	2
12:15 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
Total	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0	3	0	0	3	5
1:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
1:15 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2
Total	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	1	0	0	1	3
Grand Total	0	0	0	0	0	0	4	0	0	4	0	0	1	0	1	0	4	0	0	4	9
Approach %	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		0.0	0.0	100.0	0.0		0.0	100.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	44.4	0.0	0.0	44.4	0.0	0.0	11.1	0.0	11.1	0.0	44.4	0.0	0.0	44.4	
Exiting Leg Total	0					4					0					5					9

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	1	0	0	1	2
12:15 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2
Total Volume	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	0	2	0	0	2	5
% Approach Total	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0		0.0	0.0	100.0	0.0		0.0	100.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.500	0.000	0.000	0.250	0.000	0.250	0.000	0.500	0.000	0.000	0.500	0.625
Entering Leg	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	0	2	0	0	2	5
Exiting Leg	0					2					0					3					5
Total	0					4					1					5					10

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Single-Unit Trucks

	Driveway					Dover Road					Newmarket Road					Main Street					Total	
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
11:30 AM	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	1	0	0	0	0	1	3
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	3	0	0	0	4	5
Total	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	2	3	0	0	0	5	8
12:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	0	1	2
12:15 PM	0	0	0	0	0	0	0	3	0	3	1	0	0	0	1	0	0	0	0	0	0	4
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1
12:45 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
Total	0	0	0	0	0	0	2	3	0	5	1	0	0	0	1	1	1	0	0	0	2	8
1:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	0	0	0	0	0	0	2
1:15 PM	0	0	0	0	0	0	0	0	0	0	2	0	1	0	3	0	1	0	0	0	1	4
Total	0	0	0	0	0	0	1	0	0	1	2	0	2	0	4	0	1	0	0	0	1	6
Grand Total	0	0	0	0	0	0	5	3	0	8	3	0	3	0	6	3	5	0	0	0	8	22
Approach %	0.0	0.0	0.0	0.0		0.0	62.5	37.5	0.0		50.0	0.0	50.0	0.0		37.5	62.5	0.0	0.0			
Total %	0.0	0.0	0.0	0.0	0.0	0.0	22.7	13.6	0.0	36.4	13.6	0.0	13.6	0.0	27.3	13.6	22.7	0.0	0.0		36.4	
Exiting Leg Total	0					8					6					8					22	

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway					Dover Road					Newmarket Road					Main Street						
	from North					from East					from South					from West						
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total		
11:30 AM	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	1	0	0	0	1	3	
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	3	0	0	4	5	
12:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2	
12:15 PM	0	0	0	0	0	0	0	3	0	3	1	0	0	0	1	0	0	0	0	0	4	
Total Volume	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	2	4	0	0	6	14	
% Approach Total	0.0	0.0	0.0	0.0		0.0	50.0	50.0	0.0		50.0	0.0	50.0	0.0		33.3	66.7	0.0	0.0			
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.375	0.250	0.000	0.500	0.250	0.000	0.250	0.000	0.500	0.500	0.333	0.000	0.000	0.375	0.700	
Entering Leg	0	0	0	0	0	0	3	3	0	6	1	0	1	0	2	2	4	0	0	6	14	
Exiting Leg																						
Total	0					11					7					10					28	

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Articulated Trucks

	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Exiting Leg Total	0					0					0					0					0

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway					Dover Road					Newmarket Road					Main Street					Total
	from North					from East					from South					from West					
	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	Right	Thru	Left	U-Turn	Total	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Approach Total	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Entering Leg	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exiting Leg	0					0					0					0					0
Total	0					0					0					0					0

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Bicycles (on Roadway and Crosswalks)

	Driveway							Dover Road							Newmarket Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1
Total	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	1	2
12:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	1
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	1	2
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	1
Grand Total	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	1	0	1	1	1	0	0	0	0	2	5
Approach %	0.0	0.0	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	100.0	0.0		50.0	50.0	0.0	0.0	0.0	0.0		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	40.0	0.0	0.0	0.0	0.0	40.0	0.0	0.0	0.0	0.0	20.0	0.0	20.0	20.0	20.0	0.0	0.0	0.0	0.0	40.0	
Exiting Leg Total	0							1							2							2							5

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

11:30 AM	Driveway							Dover Road							Newmarket Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	
12:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total Volume	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	1	0	0	0	0	3	
% Approach Total	0.0	0.0	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0	100.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.750	
Entering Leg	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	1	0	0	0	0	3	
Exiting Leg							0							1							0					2	3		
Total							0							3							0					3	6		

PDI File #: **197243 C**
 Location: **N: Driveway S: Newmarket Road**
 Location: **E: Dover Road W: Main Street**
 City, State: **Durham, NH**
 Client: **Tighe & Bond/ V. Kalikiri**
 Site Code: **22-1529.002**
 Count Date: **Saturday, October 26, 2019**
 Start Time: **11:30 AM**
 End Time: **1:30 PM**
 Class:



Pedestrians

	Driveway							Dover Road							Newmarket Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
11:30 AM	0	0	0	0	1	0	1	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	1	0	1	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	0	0	0	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Total	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	0	0	0	4
1:00 PM	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	3
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	3
Grand Total	0	0	0	0	1	4	5	0	0	0	0	1	0	1	0	0	0	0	1	2	3	0	0	0	0	0	0	0	9
Approach %	0	0	0	0	20	80		0	0	0	0	100	0		0	0	0	0	33.3	66.7		0	0	0	0	0	0		
Total %	0	0	0	0	11.1	44.4	55.6	0	0	0	0	11.1	0	11.1	0	0	0	0	11.1	22.2	33.3	0	0	0	0	0	0	0	
Exiting Leg Total	5							1							3							0							9

Peak Hour Analysis from 11:30 AM to 01:30 PM begins at:

12:15 PM	Driveway							Dover Road							Newmarket Road							Main Street							Total
	from North							from East							from South							from West							
	Right	Thru	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	Left	U-Turn	CW-SB	CW-NB	Total	Right	Thru	Left	U-Turn	CW-WB	CW-EB	Total	Right	Thru	Left	U-Turn	CW-NB	CW-SB	Total	
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:45 PM	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
1:00 PM	0	0	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	3	
Total Volume	0	0	0	0	0	4	4	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	5	
% Approach Total	0.0	0.0	0.0	0.0	0.0	100.0		0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	100.0	0.0		0.0	0.0	0.0	0.0	0.0			
PHF	0.000	0.000	0.000	0.000	0.000	0.500	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.417	
Entering Leg	0	0	0	0	0	4	4	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	5	
Exiting Leg	4							0							1							0							5
Total	8							0							2							0							10

Main St & Mill Rd/Jenkins Ct
Durham, NH
3/28/2017
4th Tuesday

Source: RSG

	Main Street Eastbound Approach			Main Street Westbound Approach			Mill Rd Northbound Approach			Jenkins Ct Southbound Approach			Pedestrians (Crossing Approach)			
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	EB	WB	NB	SB
7:00 AM	3	53	18	0	0	0	0	0	0	0	0	0	0	0	1	0
7:15 AM	3	70	34	0	0	0	0	0	0	0	0	0	1	0	1	0
7:30 AM	3	69	53	0	0	0	0	0	0	0	0	0	4	0	2	0
7:45 AM	5	41	45	0	0	0	0	0	0	0	0	0	9	0	11	0
8:00 AM	1	58	49	0	0	0	0	0	0	0	0	0	4	0	9	0
8:15 AM	3	44	37	0	0	0	0	0	0	0	0	0	7	0	8	0
8:30 AM	0	57	48	0	0	0	0	0	0	0	0	0	4	0	7	0
8:45 AM	4	56	50	0	0	0	0	0	0	0	0	0	9	0	11	0
4:00 PM	7	123	65	0	0	0	0	0	0	0	0	0	22	0	14	36
4:15 PM	6	108	67	0	0	0	0	0	0	0	0	0	21	0	5	26
4:30 PM	7	136	69	0	0	0	0	0	0	0	0	0	22	0	2	23
4:45 PM	8	128	68	0	0	0	0	0	0	0	0	0	19	0	12	39
5:00 PM	7	149	90	0	0	0	0	0	0	0	0	0	13	0	16	37
5:15 PM	8	154	90	0	0	0	0	0	0	0	0	0	9	0	12	31
5:30 PM	11	115	61	0	0	0	0	0	0	0	0	0	16	0	4	19
5:45 PM	4	144	64	0	0	0	0	0	0	0	0	0	10	0	9	33

Main St & Mill Rd
Durham, NH
3/28/2017
4th Tuesday

Source: RSG

	Main Street Eastbound Approach			Main Street Westbound Approach			Mill Rd Northbound Approach			None Southbound Approach			Pedestrians (Crossing Approach)			
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	EB	WB	NB	SB
7:00 AM	0	53	0	0	0	0	0	0	55	0	0	0	1	0	1	0
7:15 AM	0	70	0	0	0	0	0	0	71	0	0	0	2	0	0	0
7:30 AM	0	69	0	0	0	0	0	0	58	0	0	0	6	0	2	0
7:45 AM	0	41	0	0	0	0	0	0	42	0	0	0	2	0	11	0
8:00 AM	0	58	0	0	0	0	0	0	36	0	0	0	2	0	7	0
8:15 AM	0	44	0	0	0	0	0	0	39	0	0	0	3	0	7	0
8:30 AM	0	57	0	0	0	0	0	0	48	0	0	0	3	0	3	0
8:45 AM	0	56	0	0	0	0	0	0	34	0	0	0	8	0	13	0
4:00 PM	0	123	0	0	0	0	0	0	98	0	0	0	9	0	14	0
4:15 PM	0	108	0	0	0	0	0	0	80	0	0	0	12	0	16	0
4:30 PM	0	136	0	0	0	0	0	0	124	0	0	0	11	0	10	0
4:45 PM	0	128	0	0	0	0	0	0	103	0	0	0	16	0	21	0
5:00 PM	0	149	0	0	0	0	0	0	155	0	0	0	20	0	34	0
5:15 PM	0	154	0	0	0	0	0	0	125	0	0	0	23	0	24	0
5:30 PM	0	115	0	0	0	0	0	0	87	0	0	0	7	0	14	0
5:45 PM	0	144	0	0	0	0	0	0	86	0	0	0	8	0	26	0

Main St & Madbury Rd
Durham, NH
3/30/2017
5th Thursday

Source: RSG

	Main Street Eastbound Approach			Main Street Westbound Approach			None Northbound Approach			Madbury Road Southbound Approach			Pedestrians (Crossing Approach)			
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	EB	WB	NB	SB
7:00 AM	40	53	0	0	0	88	0	0	0	0	0	0	4	0	0	2
7:15 AM	95	68	0	0	0	133	0	0	0	0	0	0	9	0	0	2
7:30 AM	42	91	0	0	0	121	0	0	0	0	0	0	5	0	0	8
7:45 AM	35	64	0	0	0	151	0	0	0	0	0	0	10	0	0	12
8:00 AM	32	65	0	0	0	84	0	0	0	0	0	0	10	0	0	2
8:15 AM	35	60	0	0	0	89	0	0	0	0	0	0	6	0	0	4
8:30 AM	48	61	0	0	0	122	0	0	0	0	0	0	2	0	0	3
8:45 AM	34	90	0	0	0	125	0	0	0	0	0	0	9	0	0	4
4:00 PM	82	181	0	0	0	99	0	0	0	0	0	0	28	0	0	11
4:15 PM	87	149	0	0	0	104	0	0	0	0	0	0	19	0	0	10
4:30 PM	111	186	0	0	0	110	0	0	0	0	0	0	33	0	0	14
4:45 PM	119	153	0	0	0	119	0	0	0	0	0	0	46	0	0	20
5:00 PM	99	200	0	0	0	132	0	0	0	0	0	0	34	0	0	16
5:15 PM	105	183	0	0	0	151	0	0	0	0	0	0	29	0	0	4
5:30 PM	104	149	0	0	0	124	0	0	0	0	0	0	18	0	0	9
5:45 PM	106	158	0	0	0	117	0	0	0	0	0	0	34	0	0	16

Mill Rd & Mill Plaza
Durham, NH
3/28/2017
4th Tuesday

Source: RSG

[illegible]

Mill Rd & C Lot
Durham, NH
4/5/2017
1st Wednesday

Source: RSG

[illegible]

Seasonal Adjustment Factor Worksheet

March 1.13
May 1.05
October 1.05

2016					2017					2018				
Group 4		Urban			Group 4		Urban			Group 4		Urban		
Averages:		Highways			Averages:		Highways			Averages:		Highways		
		Adjustment to					Adjustment to					Adjustment to		
Month	ADT	Average	Peak		Month	ADT	Average	Peak		Month	ADT	Average	Peak	
January	13573	1.16	1.25		January	12254	1.21	1.33		January	11281.56	1.13	1.24	
February	14038	1.12	1.21		February	13494	1.10	1.21		February	11848.06	1.08	1.18	
March	15731	1.00	1.08		March	14335	1.03	1.14		March	11828.19	1.08	1.18	
April	16139	0.97	1.05		April	15004	0.99	1.09		April	12491.31	1.02	1.12	
May	15705	1.00	1.08		May	15547	0.95	1.05		May	13586.88	0.94	1.03	
June	16766	0.94	1.01		June	16310	0.91	1.00		June	13910.81	0.92	1.00	
July	15752	1.00	1.08		July	15523	0.95	1.05		July	13765.06	0.93	1.01	
August	16529	0.95	1.03		August	15974	0.93	1.02		August	13944.94	0.92	1.00	
September	17007	0.92	1.00		September	15546	0.95	1.05		September	13168.25	0.97	1.06	
October	16598	0.94	1.02		October	15104	0.98	1.08		October	13367.06	0.96	1.04	
November	15649	1.00	1.09		November	14544	1.02	1.12		November	12215.06	1.05	1.14	
December	14638	1.07	1.16		December	14151	1.05	1.15		December	11962.56	1.07	1.17	
Average					Average					Average				
ADT:	15677.0833				ADT:	14815.5				ADT:	12780.8125			
Peak ADT:	17007				Peak ADT:	16310				Peak ADT:	13944.9375			

All data was provided by NHDOT.

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2019 Existing Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	522	225	409	289	4	148	2	425	2	3	6
Future Volume (vph)	3	522	225	409	289	4	148	2	425	2	3	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.998				0.850		0.926	
Flt Protected	0.950			0.950				0.953			0.991	
Satd. Flow (prot)	1770	1863	1583	1770	1859	0	0	1775	1583	0	1709	0
Flt Permitted	0.574			0.217				0.721			0.939	
Satd. Flow (perm)	1069	1863	1583	404	1859	0	0	1343	1583	0	1620	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			234		1				443		6	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	3	544	234	426	301	4	154	2	443	2	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	544	234	426	305	0	0	156	443	0	11	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)	34.8	29.2	29.2	54.0	51.9			12.3	12.3		7.0	
Actuated g/C Ratio	0.44	0.37	0.37	0.69	0.66			0.16	0.16		0.09	
v/c Ratio	0.01	0.78	0.32	0.70	0.25			0.74	0.71		0.07	
Control Delay	8.0	34.3	4.5	15.0	6.7			55.3	10.8		25.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	8.0	34.3	4.5	15.0	6.7			55.3	10.8		25.7	
LOS	A	C	A	B	A			E	B		C	
Approach Delay		25.3			11.6			22.4			25.7	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)	1	232	0	81	48			70	0		2	
Queue Length 95th (ft)	3	#490	50	175	123			#186	90		17	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2019 Existing Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)	100			100					150			
Base Capacity (vph)	534	693	736	820	1231			223	632		274	
Starvation Cap Reductn	0	0	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.01	0.78	0.32	0.52	0.25			0.70	0.70		0.04	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 78.4

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.78

Intersection Signal Delay: 19.7

Intersection LOS: B

Intersection Capacity Utilization 80.1%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	5.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	64	175	364	90	164	200
Future Vol, veh/h	64	175	364	90	164	200
Conflicting Peds, #/hr	0	44	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	70	190	404	100	167	204
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	992	498	0	0	504	0
Stage 1	454	-	-	-	-	-
Stage 2	538	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	272	572	-	-	1061	-
Stage 1	640	-	-	-	-	-
Stage 2	585	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	224	548	-	-	1061	-
Mov Cap-2 Maneuver	224	-	-	-	-	-
Stage 1	640	-	-	-	-	-
Stage 2	481	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	18.5	0	4.1			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 224 548	1061	-		
HCM Lane V/C Ratio	-	- 0.311 0.347	0.158	-		
HCM Control Delay (s)	-	- 28.1 15	9	0		
HCM Lane LOS	-	- D C	A	A		
HCM 95th %tile Q(veh)	-	- 1.3 1.5	0.6	-		

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2019 Existing Condition
Timing Plan: Weekday PM Peak

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	603	364	0	0	0	539
Future Volume (Veh/h)	603	364	0	0	0	539
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	678	409	0	0	0	606
Pedestrians					91	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					9	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			769		769	430
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			769		769	430
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	0
cM capacity (veh/h)			768		308	524
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	339	339	409	303	303	
Volume Left	0	0	0	0	0	
Volume Right	0	0	409	303	303	
cSH	1700	1700	1700	524	524	
Volume to Capacity	0.20	0.20	0.24	0.58	0.58	
Queue Length 95th (ft)	0	0	0	91	91	
Control Delay (s)	0.0	0.0	0.0	20.9	20.9	
Lane LOS				C	C	
Approach Delay (s)	0.0			20.9		
Approach LOS				C		
Intersection Summary						
Average Delay			7.5			
Intersection Capacity Utilization			42.2%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	901	935	906	853	876	894
Vehs Exited	899	925	907	862	884	895
Starting Vehs	9	8	15	14	13	12
Ending Vehs	11	18	14	5	5	11
Travel Distance (mi)	313	323	315	298	305	311
Travel Time (hr)	14.1	15.0	14.3	13.4	13.7	14.1
Total Delay (hr)	2.3	2.8	2.4	2.1	2.1	2.4
Total Stops	391	465	438	382	391	414
Fuel Used (gal)	11.3	11.7	11.3	10.6	10.9	11.2

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	901	935	906	853	876	894
Vehs Exited	899	925	907	862	884	895
Starting Vehs	9	8	15	14	13	12
Ending Vehs	11	18	14	5	5	11
Travel Distance (mi)	313	323	315	298	305	311
Travel Time (hr)	14.1	15.0	14.3	13.4	13.7	14.1
Total Delay (hr)	2.3	2.8	2.4	2.1	2.1	2.4
Total Stops	391	465	438	382	391	414
Fuel Used (gal)	11.3	11.7	11.3	10.6	10.9	11.2

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.3	0.4	0.4
Total Del/Veh (s)	2.6	11.8	8.0

6: Main Street & Madbury Road
Queuing and Blocking Report

2019 Existing Condition
Weekday PM Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	123	240
Average Queue (ft)	30	118
95th Queue (ft)	83	197
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2019 Existing Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	462	201	373	312	2	178	0	312	4	4	0
Future Volume (vph)	0	462	201	373	312	2	178	0	312	4	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999				0.850			
Flt Protected				0.950				0.950			0.976	
Satd. Flow (prot)	1863	1863	1583	1770	1861	0	0	1770	1583	0	1818	0
Flt Permitted				0.255				0.752			0.841	
Satd. Flow (perm)	1863	1863	1583	475	1861	0	0	1401	1583	0	1567	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			209		1				325			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	481	209	389	325	2	185	0	325	4	4	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	481	209	389	327	0	0	185	325	0	8	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)		30.0	30.0	52.4	52.4			12.9	12.9		7.1	
Actuated g/C Ratio		0.39	0.39	0.68	0.68			0.17	0.17		0.09	
v/c Ratio		0.67	0.28	0.65	0.26			0.79	0.61		0.06	
Control Delay		27.4	4.3	11.6	5.4			57.8	9.4		31.9	
Queue Delay		0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay		27.4	4.3	11.6	5.4			57.8	9.4		31.9	
LOS		C	A	B	A			E	A		C	
Approach Delay		20.4			8.8			26.9			31.9	
Approach LOS		C			A			C			C	
Queue Length 50th (ft)		184	0	66	52			84	0		4	
Queue Length 95th (ft)		#390	46	125	83			#208	71		16	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2019 Existing Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)				100					150			
Base Capacity (vph)		723	742	842	1262			236	536		264	
Starvation Cap Reductn		0	0	0	0			0	0		0	
Spillback Cap Reductn		0	0	0	0			0	0		0	
Storage Cap Reductn		0	0	0	0			0	0		0	
Reduced v/c Ratio		0.67	0.28	0.46	0.26			0.78	0.61		0.03	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 77.3

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.79

Intersection Signal Delay: 17.8

Intersection LOS: B

Intersection Capacity Utilization 76.5%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	4.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	61	108	186	59	110	146
Future Vol, veh/h	61	108	186	59	110	146
Conflicting Peds, #/hr	0	44	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	66	117	207	66	112	149
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	613	284	0	0	273	0
Stage 1	240	-	-	-	-	-
Stage 2	373	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	456	755	-	-	1290	-
Stage 1	800	-	-	-	-	-
Stage 2	696	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	413	723	-	-	1290	-
Mov Cap-2 Maneuver	413	-	-	-	-	-
Stage 1	800	-	-	-	-	-
Stage 2	630	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	12.5	0	3.5			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 413 723 1290	-	-		
HCM Lane V/C Ratio	-	- 0.161 0.162 0.087	-	-		
HCM Control Delay (s)	-	- 15.4 10.9 8.1	0			
HCM Lane LOS	-	- C B A	A			
HCM 95th %tile Q(veh)	-	- 0.6 0.6 0.3	-			

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2019 Existing Condition
Timing Plan: SAT Midday Peak

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	516	264	0	0	0	296
Future Volume (Veh/h)	516	264	0	0	0	296
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	580	297	0	0	0	333
Pedestrians					91	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					9	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			671		671	381
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			671		671	381
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	41
cM capacity (veh/h)			836		356	563
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	290	290	297	166	166	
Volume Left	0	0	0	0	0	
Volume Right	0	0	297	166	166	
cSH	1700	1700	1700	563	563	
Volume to Capacity	0.17	0.17	0.17	0.30	0.30	
Queue Length 95th (ft)	0	0	0	31	31	
Control Delay (s)	0.0	0.0	0.0	14.0	14.0	
Lane LOS				B	B	
Approach Delay (s)	0.0			14.0		
Approach LOS				B		
Intersection Summary						
Average Delay			3.9			
Intersection Capacity Utilization			31.3%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	1036	1001	943	970	911	972
Vehs Exited	1038	996	940	970	917	972
Starting Vehs	12	11	15	17	13	13
Ending Vehs	10	16	18	17	7	14
Travel Distance (mi)	361	348	327	337	317	338
Travel Time (hr)	18.2	19.0	15.3	16.2	15.1	16.8
Total Delay (hr)	4.5	5.8	2.9	3.5	3.1	4.0
Total Stops	585	580	477	516	472	526
Fuel Used (gal)	13.4	13.3	11.8	12.2	11.6	12.5

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	1036	1001	943	970	911	972
Vehs Exited	1038	996	940	970	917	972
Starting Vehs	12	11	15	17	13	13
Ending Vehs	10	16	18	17	7	14
Travel Distance (mi)	361	348	327	337	317	338
Travel Time (hr)	18.2	19.0	15.3	16.2	15.1	16.8
Total Delay (hr)	4.5	5.8	2.9	3.5	3.1	4.0
Total Stops	585	580	477	516	472	526
Fuel Used (gal)	13.4	13.3	11.8	12.2	11.6	12.5

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.3	0.5	0.4
Total Del/Veh (s)	2.7	19.0	13.0

6: Main Street & Madbury Road
Queuing and Blocking Report

2019 Existing Condition
SAT Midday Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	102	374
Average Queue (ft)	28	174
95th Queue (ft)	76	334
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2021 No Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	527	227	413	292	4	149	2	429	2	3	6
Future Volume (vph)	3	527	227	413	292	4	149	2	429	2	3	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.998				0.850		0.926	
Flt Protected	0.950			0.950				0.953			0.991	
Satd. Flow (prot)	1770	1863	1583	1770	1859	0	0	1775	1583	0	1709	0
Flt Permitted	0.573			0.211				0.721			0.938	
Satd. Flow (perm)	1067	1863	1583	393	1859	0	0	1343	1583	0	1618	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			236		1				447		6	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	3	549	236	430	304	4	155	2	447	2	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	549	236	430	308	0	0	157	447	0	11	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)	34.7	29.1	29.1	54.1	52.0			12.3	12.3		7.0	
Actuated g/C Ratio	0.44	0.37	0.37	0.69	0.66			0.16	0.16		0.09	
v/c Ratio	0.01	0.80	0.32	0.71	0.25			0.75	0.72		0.07	
Control Delay	8.0	35.2	4.5	15.7	6.7			55.9	10.9		25.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	8.0	35.2	4.5	15.7	6.7			55.9	10.9		25.7	
LOS	A	D	A	B	A			E	B		C	
Approach Delay		25.9			12.0			22.6			25.7	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)	1	236	0	86	49			70	0		2	
Queue Length 95th (ft)	3	#498	50	181	124			#187	90		17	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road Lanes, Volumes, Timings

2021 No Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)	100			100					150			
Base Capacity (vph)	531	690	735	817	1232			223	636		273	
Starvation Cap Reductn	0	0	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.01	0.80	0.32	0.53	0.25			0.70	0.70		0.04	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 78.5

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 20.2

Intersection LOS: C

Intersection Capacity Utilization 80.6%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	5.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	64	175	368	90	164	202
Future Vol, veh/h	64	175	368	90	164	202
Conflicting Peds, #/hr	0	44	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	70	190	409	100	167	206
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	999	503	0	0	509	0
Stage 1	459	-	-	-	-	-
Stage 2	540	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	270	569	-	-	1056	-
Stage 1	636	-	-	-	-	-
Stage 2	584	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	222	545	-	-	1056	-
Mov Cap-2 Maneuver	222	-	-	-	-	-
Stage 1	636	-	-	-	-	-
Stage 2	479	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	18.7	0	4.1			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 222 545	1056	-		
HCM Lane V/C Ratio	-	- 0.313 0.349	0.158	-		
HCM Control Delay (s)	-	- 28.4 15.1	9.1	0		
HCM Lane LOS	-	- D C	A	A		
HCM 95th %tile Q(veh)	-	- 1.3 1.6	0.6	-		

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2021 No Build Condition
Timing Plan: Weekday PM Peak

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	609	368	0	0	0	544
Future Volume (Veh/h)	609	368	0	0	0	544
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	684	413	0	0	0	611
Pedestrians					91	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					9	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			775		775	433
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			775		775	433
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	0
cM capacity (veh/h)			764		306	521
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	342	342	413	306	306	
Volume Left	0	0	0	0	0	
Volume Right	0	0	413	306	306	
cSH	1700	1700	1700	521	521	
Volume to Capacity	0.20	0.20	0.24	0.59	0.59	
Queue Length 95th (ft)	0	0	0	93	93	
Control Delay (s)	0.0	0.0	0.0	21.2	21.2	
Lane LOS				C	C	
Approach Delay (s)	0.0			21.2		
Approach LOS				C		
Intersection Summary						
Average Delay			7.6			
Intersection Capacity Utilization			42.5%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	918	970	922	903	871	917
Vehs Exited	920	959	923	903	879	917
Starting Vehs	9	8	15	14	13	12
Ending Vehs	7	19	14	14	5	12
Travel Distance (mi)	319	335	320	313	303	318
Travel Time (hr)	14.4	16.6	14.6	14.2	13.5	14.7
Total Delay (hr)	2.3	4.0	2.5	2.3	2.0	2.6
Total Stops	416	509	431	389	375	424
Fuel Used (gal)	11.4	12.5	11.6	11.2	10.8	11.5

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	918	970	922	903	871	917
Vehs Exited	920	959	923	903	879	917
Starting Vehs	9	8	15	14	13	12
Ending Vehs	7	19	14	14	5	12
Travel Distance (mi)	319	335	320	313	303	318
Travel Time (hr)	14.4	16.6	14.6	14.2	13.5	14.7
Total Delay (hr)	2.3	4.0	2.5	2.3	2.0	2.6
Total Stops	416	509	431	389	375	424
Fuel Used (gal)	11.4	12.5	11.6	11.2	10.8	11.5

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.3	0.5	0.4
Total Del/Veh (s)	2.6	13.2	8.8

6: Main Street & Madbury Road
Queuing and Blocking Report

2021 No Build Condition
Weekday PM Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	104	288
Average Queue (ft)	28	129
95th Queue (ft)	75	243
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2021 No Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	467	203	377	322	2	180	0	315	4	4	0
Future Volume (vph)	0	467	203	377	322	2	180	0	315	4	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999				0.850			
Flt Protected				0.950				0.950			0.976	
Satd. Flow (prot)	1863	1863	1583	1770	1861	0	0	1770	1583	0	1818	0
Flt Permitted				0.247				0.752			0.841	
Satd. Flow (perm)	1863	1863	1583	460	1861	0	0	1401	1583	0	1567	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			211		1				328			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	486	211	393	335	2	188	0	328	4	4	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	486	211	393	337	0	0	188	328	0	8	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)		29.8	29.8	52.6	52.6			13.0	13.0		7.1	
Actuated g/C Ratio		0.38	0.38	0.68	0.68			0.17	0.17		0.09	
v/c Ratio		0.68	0.29	0.66	0.27			0.81	0.61		0.06	
Control Delay		28.2	4.4	12.0	5.5			59.0	9.4		32.2	
Queue Delay		0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay		28.2	4.4	12.0	5.5			59.0	9.4		32.2	
LOS		C	A	B	A			E	A		C	
Approach Delay		21.0			9.0			27.5			32.3	
Approach LOS		C			A			C			C	
Queue Length 50th (ft)		188	0	67	54			86	0		4	
Queue Length 95th (ft)		#403	47	132	86			#214	72		16	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road Lanes, Volumes, Timings

2021 No Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)				100					150			
Base Capacity (vph)		715	738	836	1262			234	538		263	
Starvation Cap Reductn		0	0	0	0			0	0		0	
Spillback Cap Reductn		0	0	0	0			0	0		0	
Storage Cap Reductn		0	0	0	0			0	0		0	
Reduced v/c Ratio		0.68	0.29	0.47	0.27			0.80	0.61		0.03	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 77.6

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 18.3

Intersection LOS: B

Intersection Capacity Utilization 77.1%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	4.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	62	109	188	60	111	147
Future Vol, veh/h	62	109	188	60	111	147
Conflicting Peds, #/hr	0	44	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	67	118	209	67	113	150
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	619	287	0	0	276	0
Stage 1	243	-	-	-	-	-
Stage 2	376	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	452	752	-	-	1287	-
Stage 1	797	-	-	-	-	-
Stage 2	694	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	409	720	-	-	1287	-
Mov Cap-2 Maneuver	409	-	-	-	-	-
Stage 1	797	-	-	-	-	-
Stage 2	627	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	12.6	0	3.5			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 409 720 1287	-	-		
HCM Lane V/C Ratio	-	- 0.165 0.165 0.088	-	-		
HCM Control Delay (s)	-	- 15.5 11 8.1	0			
HCM Lane LOS	-	- C B A A				
HCM 95th %tile Q(veh)	-	- 0.6 0.6 0.3	-			

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2021 No Build Condition
Timing Plan: SAT Midday Peak

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	521	267	0	0	0	299
Future Volume (Veh/h)	521	267	0	0	0	299
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	585	300	0	0	0	336
Pedestrians					91	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					9	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			676		676	384
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			676		676	384
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	40
cM capacity (veh/h)			832		353	561
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	292	292	300	168	168	
Volume Left	0	0	0	0	0	
Volume Right	0	0	300	168	168	
cSH	1700	1700	1700	561	561	
Volume to Capacity	0.17	0.17	0.18	0.30	0.30	
Queue Length 95th (ft)	0	0	0	31	31	
Control Delay (s)	0.0	0.0	0.0	14.1	14.1	
Lane LOS				B	B	
Approach Delay (s)	0.0			14.1		
Approach LOS				B		
Intersection Summary						
Average Delay			3.9			
Intersection Capacity Utilization			31.5%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	979	1042	1029	982	947	996
Vehs Exited	985	1035	1029	982	946	995
Starting Vehs	12	11	15	17	13	13
Ending Vehs	6	18	15	17	14	14
Travel Distance (mi)	341	362	358	341	328	346
Travel Time (hr)	17.2	19.9	18.2	15.9	15.9	17.4
Total Delay (hr)	4.2	6.2	4.6	3.0	3.5	4.3
Total Stops	541	623	576	519	490	550
Fuel Used (gal)	12.7	13.9	13.2	12.2	12.0	12.8

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	979	1042	1029	982	947	996
Vehs Exited	985	1035	1029	982	946	995
Starting Vehs	12	11	15	17	13	13
Ending Vehs	6	18	15	17	14	14
Travel Distance (mi)	341	362	358	341	328	346
Travel Time (hr)	17.2	19.9	18.2	15.9	15.9	17.4
Total Delay (hr)	4.2	6.2	4.6	3.0	3.5	4.3
Total Stops	541	623	576	519	490	550
Fuel Used (gal)	12.7	13.9	13.2	12.2	12.0	12.8

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.3	0.5	0.4
Total Del/Veh (s)	2.8	20.9	13.9

6: Main Street & Madbury Road
Queuing and Blocking Report

2021 No Build Condition
SAT Midday Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	106	410
Average Queue (ft)	30	186
95th Queue (ft)	78	360
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2021 Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	536	232	413	297	4	151	2	429	2	3	6
Future Volume (vph)	3	536	232	413	297	4	151	2	429	2	3	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.998				0.850		0.926	
Flt Protected	0.950			0.950				0.953			0.991	
Satd. Flow (prot)	1770	1863	1583	1770	1859	0	0	1775	1583	0	1709	0
Flt Permitted	0.570			0.203				0.721			0.938	
Satd. Flow (perm)	1062	1863	1583	378	1859	0	0	1343	1583	0	1618	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			242		1				447		6	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	3	558	242	430	309	4	157	2	447	2	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	558	242	430	313	0	0	159	447	0	11	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)	34.6	29.1	29.1	54.1	52.0			12.4	12.4		7.0	
Actuated g/C Ratio	0.44	0.37	0.37	0.69	0.66			0.16	0.16		0.09	
v/c Ratio	0.01	0.81	0.33	0.72	0.25			0.75	0.72		0.07	
Control Delay	8.0	36.2	4.5	16.5	6.8			56.6	10.8		25.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	8.0	36.2	4.5	16.5	6.8			56.6	10.8		25.7	
LOS	A	D	A	B	A			E	B		C	
Approach Delay		26.5			12.4			22.8			25.7	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)	1	242	0	90	50			71	0		2	
Queue Length 95th (ft)	3	#510	51	186	126			#190	90		17	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2021 Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)	100			100					150			
Base Capacity (vph)	528	689	738	812	1231			223	635		273	
Starvation Cap Reductn	0	0	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.01	0.81	0.33	0.53	0.25			0.71	0.70		0.04	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 78.5

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 20.6

Intersection LOS: C

Intersection Capacity Utilization 81.2%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	6.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	73	209	368	95	181	202
Future Vol, veh/h	73	209	368	95	181	202
Conflicting Peds, #/hr	0	60	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	60	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	79	227	409	106	185	206
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1038	522	0	0	515	0
Stage 1	462	-	-	-	-	-
Stage 2	576	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	256	555	-	-	1051	-
Stage 1	634	-	-	-	-	-
Stage 2	562	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	211	529	-	-	1051	-
Mov Cap-2 Maneuver	211	-	-	-	-	-
Stage 1	634	-	-	-	-	-
Stage 2	463	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	20.7	0	4.3			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 211 529	1051	-		
HCM Lane V/C Ratio	-	- 0.376 0.429	0.176	-		
HCM Control Delay (s)	-	- 32 16.8	9.2	-		
HCM Lane LOS	-	- D C	A	-		
HCM 95th %tile Q(veh)	-	- 1.6 2.1	0.6	-		

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2021 Build Condition
Timing Plan: Weekday PM Peak

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	609	385	0	0	0	578
Future Volume (Veh/h)	609	385	0	0	0	578
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	684	433	0	0	0	649
Pedestrians					123	
Lane Width (ft)					10.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					10	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			807		807	465
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			807		807	465
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	0
cM capacity (veh/h)			734		288	491
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	342	342	433	324	324	
Volume Left	0	0	0	0	0	
Volume Right	0	0	433	324	324	
cSH	1700	1700	1700	491	491	
Volume to Capacity	0.20	0.20	0.25	0.66	0.66	
Queue Length 95th (ft)	0	0	0	119	119	
Control Delay (s)	0.0	0.0	0.0	25.5	25.5	
Lane LOS				D	D	
Approach Delay (s)	0.0			25.5		
Approach LOS				D		
Intersection Summary						
Average Delay			9.4			
Intersection Capacity Utilization			43.7%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	923	950	962	932	919	937
Vehs Exited	922	954	959	931	924	938
Starting Vehs	9	14	15	14	13	13
Ending Vehs	10	10	18	15	8	12
Travel Distance (mi)	320	331	333	323	320	325
Travel Time (hr)	15.0	15.9	16.0	14.9	14.7	15.3
Total Delay (hr)	2.9	3.4	3.5	2.6	2.6	3.0
Total Stops	444	509	484	424	468	466
Fuel Used (gal)	11.7	12.1	12.2	11.7	11.5	11.8

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	923	950	962	932	919	937
Vehs Exited	922	954	959	931	924	938
Starting Vehs	9	14	15	14	13	13
Ending Vehs	10	10	18	15	8	12
Travel Distance (mi)	320	331	333	323	320	325
Travel Time (hr)	15.0	15.9	16.0	14.9	14.7	15.3
Total Delay (hr)	2.9	3.4	3.5	2.6	2.6	3.0
Total Stops	444	509	484	424	468	466
Fuel Used (gal)	11.7	12.1	12.2	11.7	11.5	11.8

6: Main Street EBL/Main Street WBR & Madbury Road Performance by movement

Movement	EBL	WBR	All
Denied Del/Veh (s)	0.3	0.4	0.4
Total Del/Veh (s)	2.9	15.1	10.0

6: Main Street & Madbury Road
Queuing and Blocking Report

2021 Build Condition
Weekday PM Peak

Intersection: 6: Main Street EBL/Main Street WBR & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	120	267
Average Queue (ft)	33	141
95th Queue (ft)	86	245
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2021 Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	474	207	377	322	2	184	0	315	4	4	0
Future Volume (vph)	0	474	207	377	322	2	184	0	315	4	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999				0.850			
Flt Protected				0.950				0.950			0.976	
Satd. Flow (prot)	1863	1863	1583	1770	1861	0	0	1770	1583	0	1818	0
Flt Permitted				0.236				0.752			0.839	
Satd. Flow (perm)	1863	1863	1583	440	1861	0	0	1401	1583	0	1563	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			216		1				328			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	494	216	393	335	2	192	0	328	4	4	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	494	216	393	337	0	0	192	328	0	8	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)		29.7	29.7	52.9	52.9			13.0	13.0		7.1	
Actuated g/C Ratio		0.38	0.38	0.68	0.68			0.17	0.17		0.09	
v/c Ratio		0.70	0.29	0.66	0.27			0.82	0.61		0.06	
Control Delay		29.1	4.4	12.6	5.5			61.3	9.4		32.4	
Queue Delay		0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay		29.1	4.4	12.6	5.5			61.3	9.4		32.4	
LOS		C	A	B	A			E	A		C	
Approach Delay		21.6			9.3			28.6			32.4	
Approach LOS		C			A			C			C	
Queue Length 50th (ft)		195	0	67	54			88	0		4	
Queue Length 95th (ft)		#415	48	138	85			#222	73		16	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road Lanes, Volumes, Timings

2021 Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)				100					150			
Base Capacity (vph)		709	736	828	1263			234	537		261	
Starvation Cap Reductn		0	0	0	0			0	0		0	
Spillback Cap Reductn		0	0	0	0			0	0		0	
Storage Cap Reductn		0	0	0	0			0	0		0	
Reduced v/c Ratio		0.70	0.29	0.47	0.27			0.82	0.61		0.03	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 77.9

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 18.9

Intersection LOS: B

Intersection Capacity Utilization 77.7%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	5.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	69	135	188	67	137	147
Future Vol, veh/h	69	135	188	67	137	147
Conflicting Peds, #/hr	0	60	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	60	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	75	147	209	74	140	150
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	676	306	0	0	283	0
Stage 1	246	-	-	-	-	-
Stage 2	430	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	419	734	-	-	1279	-
Stage 1	795	-	-	-	-	-
Stage 2	656	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	373	699	-	-	1279	-
Mov Cap-2 Maneuver	373	-	-	-	-	-
Stage 1	795	-	-	-	-	-
Stage 2	584	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	13.4	0	3.9			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 373 699 1279	-			
HCM Lane V/C Ratio	-	- 0.201 0.21 0.109	-			
HCM Control Delay (s)	-	- 17.1 11.5 8.2	-			
HCM Lane LOS	-	- C B A	-			
HCM 95th %tile Q(veh)	-	- 0.7 0.8 0.4	-			

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2021 Build Condition
Timing Plan: SAT Midday Peak

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	521	293	0	0	0	325
Future Volume (Veh/h)	521	293	0	0	0	325
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	585	329	0	0	0	365
Pedestrians					123	
Lane Width (ft)					10.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					10	
Right turn flare (veh)						
Median type	None		None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			708		708	416
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			708		708	416
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	31
cM capacity (veh/h)			800		333	529
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	292	292	329	182	182	
Volume Left	0	0	0	0	0	
Volume Right	0	0	329	182	182	
cSH	1700	1700	1700	529	529	
Volume to Capacity	0.17	0.17	0.19	0.35	0.35	
Queue Length 95th (ft)	0	0	0	38	38	
Control Delay (s)	0.0	0.0	0.0	15.4	15.4	
Lane LOS				C	C	
Approach Delay (s)	0.0			15.4		
Approach LOS				C		
Intersection Summary						
Average Delay			4.4			
Intersection Capacity Utilization			32.4%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	7	Avg
Start Time	4:27	4:27
End Time	5:30	5:30
Total Time (min)	63	63
Time Recorded (min)	60	60
# of Intervals	2	2
# of Recorded Intervals	1	1
Vehs Entered	986	986
Vehs Exited	997	997
Starting Vehs	25	25
Ending Vehs	14	14
Travel Distance (mi)	345	345
Travel Time (hr)	17.7	17.7
Total Delay (hr)	4.7	4.7
Total Stops	553	553
Fuel Used (gal)	13.0	13.0

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	7	Avg
Vehs Entered	986	986
Vehs Exited	997	997
Starting Vehs	25	25
Ending Vehs	14	14
Travel Distance (mi)	345	345
Travel Time (hr)	17.7	17.7
Total Delay (hr)	4.7	4.7
Total Stops	553	553
Fuel Used (gal)	13.0	13.0

6: Main Street & Madbury Road Performance by movement

Movement	EBL	WBR	All
Denied Del/Veh (s)	0.4	0.6	0.5
Total Del/Veh (s)	3.3	23.0	15.2

6: Main Street & Madbury Road
Queuing and Blocking Report

2021 Build Condition
SAT Midday Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	119	499
Average Queue (ft)	34	206
95th Queue (ft)	91	412
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2031 No Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	549	237	430	304	4	156	2	447	2	3	6
Future Volume (vph)	3	549	237	430	304	4	156	2	447	2	3	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.998				0.850		0.926	
Flt Protected	0.950			0.950				0.953			0.991	
Satd. Flow (prot)	1770	1863	1583	1770	1859	0	0	1775	1583	0	1709	0
Flt Permitted	0.566			0.181				0.721			0.938	
Satd. Flow (perm)	1054	1863	1583	337	1859	0	0	1343	1583	0	1618	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			247		1				466		6	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	3	572	247	448	317	4	163	2	466	2	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	572	247	448	321	0	0	165	466	0	11	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)	34.1	28.5	28.5	54.5	52.4			12.5	12.5		7.1	
Actuated g/C Ratio	0.43	0.36	0.36	0.69	0.66			0.16	0.16		0.09	
v/c Ratio	0.01	0.85	0.34	0.75	0.26			0.78	0.73		0.07	
Control Delay	8.3	40.6	4.7	19.7	6.8			59.4	11.0		26.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	8.3	40.6	4.7	19.7	6.8			59.4	11.0		26.0	
LOS	A	D	A	B	A			E	B		C	
Approach Delay		29.7			14.3			23.7			26.0	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)	1	256	0	112	51			75	0		2	
Queue Length 95th (ft)	3	#542	53	211	130			#198	94		17	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road Lanes, Volumes, Timings

2031 No Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)	100			100					150			
Base Capacity (vph)	514	671	728	796	1232			221	650		271	
Starvation Cap Reductn	0	0	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.01	0.85	0.34	0.56	0.26			0.75	0.72		0.04	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 79.1

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 22.7

Intersection LOS: C

Intersection Capacity Utilization 83.1%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	5.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	64	175	383	90	164	210
Future Vol, veh/h	64	175	383	90	164	210
Conflicting Peds, #/hr	0	44	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	70	190	426	100	167	214
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1024	520	0	0	526	0
Stage 1	476	-	-	-	-	-
Stage 2	548	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	261	556	-	-	1041	-
Stage 1	625	-	-	-	-	-
Stage 2	579	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	213	533	-	-	1041	-
Mov Cap-2 Maneuver	213	-	-	-	-	-
Stage 1	625	-	-	-	-	-
Stage 2	474	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	19.4	0	4			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 213 533 1041		-		
HCM Lane V/C Ratio	-	- 0.327 0.357 0.161		-		
HCM Control Delay (s)	-	- 29.9 15.5 9.1		0		
HCM Lane LOS	-	- D C A		A		
HCM 95th %tile Q(veh)	-	- 1.4 1.6 0.6		-		

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2031 No Build Condition
Timing Plan: Weekday PM Peak

						
Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations	↑↑	↑				↑↑
Traffic Volume (veh/h)	634	383	0	0	0	567
Future Volume (Veh/h)	634	383	0	0	0	567
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	712	430	0	0	0	637
Pedestrians					91	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					9	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			803		803	447
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			803		803	447
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	0
cM capacity (veh/h)			746		293	511
Direction, Lane #	EB 1	EB 2	EB 3	NE 1	NE 2	
Volume Total	356	356	430	318	318	
Volume Left	0	0	0	0	0	
Volume Right	0	0	430	318	318	
cSH	1700	1700	1700	511	511	
Volume to Capacity	0.21	0.21	0.25	0.62	0.62	
Queue Length 95th (ft)	0	0	0	106	106	
Control Delay (s)	0.0	0.0	0.0	23.0	23.0	
Lane LOS				C	C	
Approach Delay (s)	0.0			23.0		
Approach LOS				C		
Intersection Summary						
Average Delay			8.2			
Intersection Capacity Utilization			44.0%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	951	975	905	976	893	940
Vehs Exited	957	964	906	977	895	940
Starting Vehs	17	9	15	16	13	14
Ending Vehs	11	20	14	15	11	14
Travel Distance (mi)	331	336	314	339	310	326
Travel Time (hr)	15.1	17.2	14.4	16.1	14.3	15.4
Total Delay (hr)	2.5	4.5	2.5	3.3	2.6	3.1
Total Stops	451	514	411	486	435	459
Fuel Used (gal)	12.0	12.6	11.3	12.4	11.2	11.9

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	951	975	905	976	893	940
Vehs Exited	957	964	906	977	895	940
Starting Vehs	17	9	15	16	13	14
Ending Vehs	11	20	14	15	11	14
Travel Distance (mi)	331	336	314	339	310	326
Travel Time (hr)	15.1	17.2	14.4	16.1	14.3	15.4
Total Delay (hr)	2.5	4.5	2.5	3.3	2.6	3.1
Total Stops	451	514	411	486	435	459
Fuel Used (gal)	12.0	12.6	11.3	12.4	11.2	11.9

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.3	0.4	0.4
Total Del/Veh (s)	3.0	15.4	10.2

6: Main Street & Madbury Road
Queuing and Blocking Report

2031 No Build Condition
Weekday PM Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	114	323
Average Queue (ft)	34	139
95th Queue (ft)	90	262
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road
Lanes, Volumes, Timings

2031 No Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	486	211	392	328	2	187	0	328	4	4	0
Future Volume (vph)	0	486	211	392	328	2	187	0	328	4	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999				0.850			
Flt Protected				0.950				0.950			0.976	
Satd. Flow (prot)	1863	1863	1583	1770	1861	0	0	1770	1583	0	1818	0
Flt Permitted				0.217				0.752			0.838	
Satd. Flow (perm)	1863	1863	1583	404	1861	0	0	1401	1583	0	1561	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			220		1				342			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	506	220	408	342	2	195	0	342	4	4	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	506	220	408	344	0	0	195	342	0	8	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)		29.2	29.2	53.4	53.4			13.0	13.0		7.1	
Actuated g/C Ratio		0.37	0.37	0.68	0.68			0.17	0.17		0.09	
v/c Ratio		0.73	0.30	0.69	0.27			0.84	0.62		0.06	
Control Delay		31.2	4.5	14.3	5.5			64.2	9.6		33.1	
Queue Delay		0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay		31.2	4.5	14.3	5.5			64.2	9.6		33.1	
LOS		C	A	B	A			E	A		C	
Approach Delay		23.1			10.3			29.4			33.1	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)		206	0	71	56			90	0		4	
Queue Length 95th (ft)		#443	49	159	87			#232	75		16	

101: Newmarket Road/Hotel Driveway & Main Street/Dover Road Lanes, Volumes, Timings

2031 No Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)				100					150			
Base Capacity (vph)		694	727	815	1267			232	548		259	
Starvation Cap Reductn		0	0	0	0			0	0		0	
Spillback Cap Reductn		0	0	0	0			0	0		0	
Storage Cap Reductn		0	0	0	0			0	0		0	
Reduced v/c Ratio		0.73	0.30	0.50	0.27			0.84	0.62		0.03	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 78.5

Natural Cycle: 60

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 20.1

Intersection LOS: C

Intersection Capacity Utilization 79.3%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street/Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	4.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	61	108	196	59	110	153
Future Vol, veh/h	61	108	196	59	110	153
Conflicting Peds, #/hr	0	44	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	66	117	218	66	112	156
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	631	295	0	0	284	0
Stage 1	251	-	-	-	-	-
Stage 2	380	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	445	744	-	-	1278	-
Stage 1	791	-	-	-	-	-
Stage 2	691	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	402	713	-	-	1278	-
Mov Cap-2 Maneuver	402	-	-	-	-	-
Stage 1	791	-	-	-	-	-
Stage 2	625	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	12.7	0	3.4			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 402 713 1278	-	-		
HCM Lane V/C Ratio	-	- 0.165 0.165 0.088	-	-		
HCM Control Delay (s)	-	- 15.7 11 8.1	0			
HCM Lane LOS	-	- C B A A				
HCM 95th %tile Q(veh)	-	- 0.6 0.6 0.3	-	-		

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2031 No Build Condition
Timing Plan: SAT Midday Peak

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	542	278	0	0	0	311
Future Volume (Veh/h)	542	278	0	0	0	311
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	609	312	0	0	0	349
Pedestrians				91		
Lane Width (ft)				0.0		
Walking Speed (ft/s)				3.5		
Percent Blockage				0		
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			609		609	396
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			609		609	396
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	42
cM capacity (veh/h)			966		427	604
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	304	304	312	174	174	
Volume Left	0	0	0	0	0	
Volume Right	0	0	312	174	174	
cSH	1700	1700	1700	604	604	
Volume to Capacity	0.18	0.18	0.18	0.29	0.29	
Queue Length 95th (ft)	0	0	0	30	30	
Control Delay (s)	0.0	0.0	0.0	13.4	13.4	
Lane LOS				B	B	
Approach Delay (s)	0.0			13.4		
Approach LOS				B		
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			38.2%	ICU Level of Service	A	
Analysis Period (min)			15			

Summary of All Intervals

Run Number	10	Avg
Start Time	4:27	4:27
End Time	5:30	5:30
Total Time (min)	63	63
Time Recorded (min)	60	60
# of Intervals	2	2
# of Recorded Intervals	1	1
Vehs Entered	1017	1017
Vehs Exited	1009	1009
Starting Vehs	13	13
Ending Vehs	21	21
Travel Distance (mi)	352	352
Travel Time (hr)	18.5	18.5
Total Delay (hr)	5.1	5.1
Total Stops	588	588
Fuel Used (gal)	13.1	13.1

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	10	Avg
Vehs Entered	1017	1017
Vehs Exited	1009	1009
Starting Vehs	13	13
Ending Vehs	21	21
Travel Distance (mi)	352	352
Travel Time (hr)	18.5	18.5
Total Delay (hr)	5.1	5.1
Total Stops	588	588
Fuel Used (gal)	13.1	13.1

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.4	0.5	0.5
Total Del/Veh (s)	3.8	25.1	16.5

6: Main Street & Madbury Road
Queuing and Blocking Report

2031 No Build Condition
SAT Midday Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	142	500
Average Queue (ft)	48	221
95th Queue (ft)	110	417
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street /Dover Road
Lanes, Volumes, Timings

2031 Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	3	558	242	430	309	4	158	2	447	2	3	6
Future Volume (vph)	3	558	242	430	309	4	158	2	447	2	3	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.998				0.850		0.926	
Flt Protected	0.950			0.950				0.953			0.991	
Satd. Flow (prot)	1770	1863	1583	1770	1859	0	0	1775	1583	0	1709	0
Flt Permitted	0.563			0.173				0.721			0.938	
Satd. Flow (perm)	1049	1863	1583	322	1859	0	0	1343	1583	0	1618	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			252		1				466		6	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	3	581	252	448	322	4	165	2	466	2	3	6
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	581	252	448	326	0	0	167	466	0	11	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)	34.0	28.5	28.5	54.5	52.4			12.5	12.5		7.1	
Actuated g/C Ratio	0.43	0.36	0.36	0.69	0.66			0.16	0.16		0.09	
v/c Ratio	0.01	0.87	0.34	0.76	0.26			0.79	0.73		0.07	
Control Delay	8.3	42.0	4.7	20.8	6.8			60.1	11.0		26.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay	8.3	42.0	4.7	20.8	6.8			60.1	11.0		26.0	
LOS	A	D	A	C	A			E	B		C	
Approach Delay		30.6			14.9			24.0			26.0	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)	1	262	0	117	52			76	0		2	
Queue Length 95th (ft)	3	#554	52	216	131			#202	94		17	

101: Newmarket Road/Hotel Driveway & Main Street /Dover Road
Lanes, Volumes, Timings

2031 Build Condition
Timing Plan: Weekday PM Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)	100			100					150			
Base Capacity (vph)	512	670	731	791	1231			221	650		271	
Starvation Cap Reductn	0	0	0	0	0			0	0		0	
Spillback Cap Reductn	0	0	0	0	0			0	0		0	
Storage Cap Reductn	0	0	0	0	0			0	0		0	
Reduced v/c Ratio	0.01	0.87	0.34	0.57	0.26			0.76	0.72		0.04	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 79.1

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.87

Intersection Signal Delay: 23.3

Intersection LOS: C

Intersection Capacity Utilization 83.7%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street /Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	6.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	73	209	383	95	181	210
Future Vol, veh/h	73	209	383	95	181	210
Conflicting Peds, #/hr	0	60	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	60	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	79	227	426	106	185	214
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	1063	539	0	0	532	0
Stage 1	479	-	-	-	-	-
Stage 2	584	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	247	542	-	-	1036	-
Stage 1	623	-	-	-	-	-
Stage 2	557	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	203	516	-	-	1036	-
Mov Cap-2 Maneuver	203	-	-	-	-	-
Stage 1	623	-	-	-	-	-
Stage 2	457	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	21.6	0	4.3			
HCM LOS	C					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 203 516 1036	-			
HCM Lane V/C Ratio	-	- 0.391 0.44 0.178	-			
HCM Control Delay (s)	-	- 33.7 17.4 9.2	-			
HCM Lane LOS	-	- D C A	-			
HCM 95th %tile Q(veh)	-	- 1.7 2.2 0.6	-			

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2031 Build Condition
Timing Plan: Weekday PM Peak

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	634	400	0	0	0	601
Future Volume (Veh/h)	634	400	0	0	0	601
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	712	449	0	0	0	675
Pedestrians					123	
Lane Width (ft)					10.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					10	
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			835		835	479
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			835		835	479
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	0
cM capacity (veh/h)			717		276	481
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	356	356	449	338	338	
Volume Left	0	0	0	0	0	
Volume Right	0	0	449	338	338	
cSH	1700	1700	1700	481	481	
Volume to Capacity	0.21	0.21	0.26	0.70	0.70	
Queue Length 95th (ft)	0	0	0	136	136	
Control Delay (s)	0.0	0.0	0.0	28.3	28.3	
Lane LOS				D	D	
Approach Delay (s)	0.0			28.3		
Approach LOS				D		
Intersection Summary						
Average Delay			10.4			
Intersection Capacity Utilization			45.2%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	1	2	3	4	5	Avg
Start Time	4:27	4:27	4:27	4:27	4:27	4:27
End Time	5:30	5:30	5:30	5:30	5:30	5:30
Total Time (min)	63	63	63	63	63	63
Time Recorded (min)	60	60	60	60	60	60
# of Intervals	2	2	2	2	2	2
# of Recorded Intervals	1	1	1	1	1	1
Vehs Entered	957	1024	967	942	952	968
Vehs Exited	963	1021	959	948	951	968
Starting Vehs	17	16	15	16	11	15
Ending Vehs	11	19	23	10	12	15
Travel Distance (mi)	334	355	334	328	330	336
Travel Time (hr)	15.4	19.1	15.8	15.1	15.5	16.2
Total Delay (hr)	2.8	5.7	3.1	2.7	3.0	3.5
Total Stops	452	549	467	458	483	482
Fuel Used (gal)	12.1	13.5	12.1	11.7	11.9	12.3

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	1	2	3	4	5	Avg
Vehs Entered	957	1024	967	942	952	968
Vehs Exited	963	1021	959	948	951	968
Starting Vehs	17	16	15	16	11	15
Ending Vehs	11	19	23	10	12	15
Travel Distance (mi)	334	355	334	328	330	336
Travel Time (hr)	15.4	19.1	15.8	15.1	15.5	16.2
Total Delay (hr)	2.8	5.7	3.1	2.7	3.0	3.5
Total Stops	452	549	467	458	483	482
Fuel Used (gal)	12.1	13.5	12.1	11.7	11.9	12.3

6: Main Street & Madbury Road Performance by approach

Approach	EB	WB	All
Denied Del/Veh (s)	0.4	0.4	0.4
Total Del/Veh (s)	3.0	17.2	11.3

6: Main Street & Madbury Road
Queuing and Blocking Report

2031 Build Condition
Weekday PM Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	106	368
Average Queue (ft)	33	152
95th Queue (ft)	85	302
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

101: Newmarket Road/Hotel Driveway & Main Street /Dover Road
Lanes, Volumes, Timings

2031 Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	493	215	392	335	2	191	2	328	4	4	0
Future Volume (vph)	0	493	215	392	335	2	191	2	328	4	4	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	100		0	0		150	0		0
Storage Lanes	1		1	1		0	0		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999				0.850			
Flt Protected				0.950				0.953			0.976	
Satd. Flow (prot)	1863	1863	1583	1770	1861	0	0	1775	1583	0	1818	0
Flt Permitted				0.209				0.723			0.836	
Satd. Flow (perm)	1863	1863	1583	389	1861	0	0	1347	1583	0	1557	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			224		1				342			
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		731			773			892			159	
Travel Time (s)		16.6			17.6			20.3			3.6	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	514	224	408	349	2	199	2	342	4	4	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	514	224	408	351	0	0	201	342	0	8	0
Turn Type	pm+pt	NA	Prot	pm+pt	NA		Perm	NA	Prot	Perm	NA	
Protected Phases	1	6	6	5	2			4	4		8	
Permitted Phases	6			2			4			8		
Detector Phase	1	6	6	5	2		4	4	4	8	8	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	11.0	24.0	24.0	11.0	24.0		11.0	11.0	11.0	11.0	11.0	
Total Split (s)	12.0	32.0	32.0	37.0	57.0		19.0	19.0	19.0	19.0	19.0	
Total Split (%)	13.6%	36.4%	36.4%	42.0%	64.8%		21.6%	21.6%	21.6%	21.6%	21.6%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0	6.0	6.0			6.0	6.0		6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Recall Mode	None	Max	Max	None	Max		None	None	None	None	None	
Act Effct Green (s)		29.2	29.2	53.4	53.4			13.0	13.0		7.1	
Actuated g/C Ratio		0.37	0.37	0.68	0.68			0.17	0.17		0.09	
v/c Ratio		0.74	0.31	0.70	0.28			0.90	0.62		0.06	
Control Delay		31.7	4.5	15.1	5.5			75.1	9.6		33.1	
Queue Delay		0.0	0.0	0.0	0.0			0.0	0.0		0.0	
Total Delay		31.7	4.5	15.1	5.5			75.1	9.6		33.1	
LOS		C	A	B	A			E	A		C	
Approach Delay		23.5			10.7			33.8			33.1	
Approach LOS		C			B			C			C	
Queue Length 50th (ft)		210	0	76	57			94	0		4	
Queue Length 95th (ft)		#453	49	164	89			#245	75		16	

101: Newmarket Road/Hotel Driveway & Main Street /Dover Road
Lanes, Volumes, Timings

2031 Build Condition
Timing Plan: SAT Midday Peak

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		651			693			812			79	
Turn Bay Length (ft)				100					150			
Base Capacity (vph)		694	730	811	1267			223	548		258	
Starvation Cap Reductn		0	0	0	0			0	0		0	
Spillback Cap Reductn		0	0	0	0			0	0		0	
Storage Cap Reductn		0	0	0	0			0	0		0	
Reduced v/c Ratio		0.74	0.31	0.50	0.28			0.90	0.62		0.03	

Intersection Summary

Area Type: Other

Cycle Length: 88

Actuated Cycle Length: 78.5

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.90

Intersection Signal Delay: 21.5

Intersection LOS: C

Intersection Capacity Utilization 80.0%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 101: Newmarket Road/Hotel Driveway & Main Street /Dover Road

 Ø1	 Ø2	 Ø4
12 s	57 s	19 s
 Ø5	 Ø6	 Ø8
37 s	32 s	19 s

Intersection						
Int Delay, s/veh	5.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	68	134	196	66	136	153
Future Vol, veh/h	68	134	196	66	136	153
Conflicting Peds, #/hr	0	60	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	60	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	90	90	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	74	146	218	73	139	156
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	689	315	0	0	291	0
Stage 1	255	-	-	-	-	-
Stage 2	434	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	412	725	-	-	1271	-
Stage 1	788	-	-	-	-	-
Stage 2	653	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	367	690	-	-	1271	-
Mov Cap-2 Maneuver	367	-	-	-	-	-
Stage 1	788	-	-	-	-	-
Stage 2	582	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	13.5	0	3.8			
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT		
Capacity (veh/h)	-	- 367 690	1271	-		
HCM Lane V/C Ratio	-	- 0.201 0.211	0.109	-		
HCM Control Delay (s)	-	- 17.3 11.6	8.2	-		
HCM Lane LOS	-	- C B	A	-		
HCM 95th %tile Q(veh)	-	- 0.7 0.8	0.4	-		

202: Mill Road & Main Street
HCM Unsignalized Intersection Capacity Analysis

2031 Build Condition
Timing Plan: SAT Midday Peak

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗				↗↗
Traffic Volume (veh/h)	542	304	0	0	0	337
Future Volume (Veh/h)	542	304	0	0	0	337
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.89	0.89	0.92	0.92	0.89	0.89
Hourly flow rate (vph)	609	342	0	0	0	379
Pedestrians					123	
Lane Width (ft)					10.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					10	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			732		732	428
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			732		732	428
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	27
cM capacity (veh/h)			784		322	519
Direction, Lane #	EB 1	EB 2	EB 3	NB 1	NB 2	
Volume Total	304	304	342	190	190	
Volume Left	0	0	0	0	0	
Volume Right	0	0	342	190	190	
cSH	1700	1700	1700	519	519	
Volume to Capacity	0.18	0.18	0.20	0.36	0.36	
Queue Length 95th (ft)	0	0	0	41	41	
Control Delay (s)	0.0	0.0	0.0	15.9	15.9	
Lane LOS				C	C	
Approach Delay (s)	0.0			15.9		
Approach LOS				C		
Intersection Summary						
Average Delay			4.5			
Intersection Capacity Utilization			33.4%	ICU Level of Service		A
Analysis Period (min)			15			

Summary of All Intervals

Run Number	8	Avg
Start Time	4:27	4:27
End Time	5:30	5:30
Total Time (min)	63	63
Time Recorded (min)	60	60
# of Intervals	2	2
# of Recorded Intervals	1	1
Vehs Entered	1046	1046
Vehs Exited	1056	1056
Starting Vehs	17	17
Ending Vehs	7	7
Travel Distance (mi)	365	365
Travel Time (hr)	20.2	20.2
Total Delay (hr)	6.4	6.4
Total Stops	631	631
Fuel Used (gal)	13.9	13.9

Interval #0 Information Seeding

Start Time	4:27
End Time	4:30
Total Time (min)	3
Volumes adjusted by PHF.	
No data recorded this interval.	

Interval #1 Information Recording

Start Time	4:30
End Time	5:30
Total Time (min)	60
Volumes adjusted by PHF.	

Run Number	8	Avg
Vehs Entered	1046	1046
Vehs Exited	1056	1056
Starting Vehs	17	17
Ending Vehs	7	7
Travel Distance (mi)	365	365
Travel Time (hr)	20.2	20.2
Total Delay (hr)	6.4	6.4
Total Stops	631	631
Fuel Used (gal)	13.9	13.9

6: Main Street & Madbury Road Performance by movement

Movement	EBL	WBR	All
Denied Del/Veh (s)	0.3	0.6	0.5
Total Del/Veh (s)	3.6	31.4	20.1

6: Main Street & Madbury Road
Queuing and Blocking Report

2031 Build Condition
SAT Midday Peak

Intersection: 6: Main Street & Madbury Road

Movement	EB	WB
Directions Served	L	R
Maximum Queue (ft)	119	562
Average Queue (ft)	48	243
95th Queue (ft)	105	457
Link Distance (ft)	997	1070
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Mill Plaza			Organization:	Tighe & Bond
Project Location:	Durham NH			Performed By:	EM
Scenario Description:	PM Peak Hour			Date:	10/25/2019
Analysis Year:	2019			Checked By:	
Analysis Period:	PM Street Peak Hour			Date:	

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office				27	4	23
Retail				467	256	211
Restaurant				0		
Cinema/Entertainment				0		
Residential				0		
Hotel				0		
All Other Land Uses ²				41	20	21
				535	280	255

Table 2-P: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses ²						

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-P: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		5	0	0	0	0
Retail	1		0	0	0	0
Restaurant	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	0	0		0
Hotel	0	0	0	0	0	

Table 5-P: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	535	280	255
Internal Capture Percentage	2%	2%	2%
External Vehicle-Trips ⁵	523	274	249
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-P: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	25%	22%
Retail	2%	0%
Restaurant	N/A	N/A
Cinema/Entertainment	N/A	N/A
Residential	N/A	N/A
Hotel	N/A	N/A

¹ Land Use Codes (LUCs) from <i>Trip Generation Manual</i> , published by the Institute of Transportation Engineers.
² Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.
³ Enter trips assuming no transit or non-motorized trips (as assumed in ITE <i>Trip Generation Manual</i>).
⁴ Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be
⁵ Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.
⁶ Person-Trips
*Indicates computation that has been rounded to the nearest whole number.
Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Project Name:	Mill Plaza
Analysis Period:	PM Street Peak Hour

Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-P (D): Entering Trips			Table 7-P (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	4	4	1.00	23	23
Retail	1.00	256	256	1.00	211	211
Restaurant	1.00	0	0	1.00	0	0
Cinema/Entertainment	1.00	0	0	1.00	0	0
Residential	1.00	0	0	1.00	0	0
Hotel	1.00	0	0	1.00	0	0

Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		5	1	0	0	0
Retail	4		61	8	55	11
Restaurant	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	0	0		0
Hotel	0	0	0	0	0	

Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		20	0	0	0	0
Retail	1		0	0	0	0
Restaurant	1	128		0	0	0
Cinema/Entertainment	0	10	0		0	0
Residential	2	26	0	0		0
Hotel	0	5	0	0	0	

Table 9-P (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	1	3	4	3	0	0
Retail	5	251	256	251	0	0
Restaurant	0	0	0	0	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	0	0	0	0	0	0
Hotel	0	0	0	0	0	0
All Other Land Uses ³	0	20	20	20	0	0

Table 9-P (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles ¹	Transit ²	Non-Motorized ²
Office	5	18	23	18	0	0
Retail	1	210	211	210	0	0
Restaurant	0	0	0	0	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	0	0	0	0	0	0
Hotel	0	0	0	0	0	0
All Other Land Uses ³	0	21	21	21	0	0

¹Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

²Person-Trips

³Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator

*Indicates computation that has been rounded to the nearest whole number.

Mill Road Plaza Trip Generation								
			PM Peak Hour			Saturday Peak		
			Enter	Exit	Total	Enter	Exit	Total
Existing Trips (Based on ITE)								
	Land Use	Size						
	Retail (820)	55,400 SF	168	213	381	217	201	418
	Total		168	213	381	217	201	418
Existing Trips (Based on Driveway Counts)								
	Mill Road Plaza		254	239	493	169	169	338
	Total		254	239	493	169	169	338
Existing Non Pass-By Trips								
	Total		254	239	493	169	169	338
Generated Retail Trips ¹ (Proportional)								
	Retail (820)	55,865	256	241	497	170	170	341
Generated Trips (Based on ITE)								
	Off-Campus Student Housing (225)	258 Dwelling Units	32	33	65	32	33	65
	General Office (710)	22,385 SF	4	23	27	6	6	12
	Drive-in Bank (912)	2,000 SF	20	21	41	27	26	53
	Total		56	77	133	65	65	130
Internal Capture ³								
	<u>Internal Capture</u>							
	Residential (225)		15	15	30	15	15	30
	Office (710)		2	5	7	3	1	4
	Retail (820)		19	16	35	15	17	32
	Drive-in Bank (912)				0			0
	Total Internal Capture		36	36	72	33	33	66
	<u>External Trips</u>							
	Residential (225)		17	18	35	17	18	35
	Office (710)		2	18	20	3	5	8
	Retail (820)		237	225	462	155	153	308
	Drive-in Bank (912)		20	21	41	27	26	53
	Total External Capture		256	261	517	175	176	351
Total New Site Generated Trips								
	Residential (225)		17	18	35	17	18	35
	Office (710)		2	18	20	3	5	8
	Retail (820)		237	225	462	155	153	308
	Drive-in Bank (912)		20	21	41	27	26	53
	Total		276	282	558	202	202	404
Net Change due to Development			22	43	65	33	33	66

LUC 936

¹

²

³

⁴

Restaurant - The Works Café
Proportional Expansion of Retail Trip Generation
Trips Per 1000 SF.
Internal Capture Values are based on NCHRP 684 Report
Based on ITE Methodology

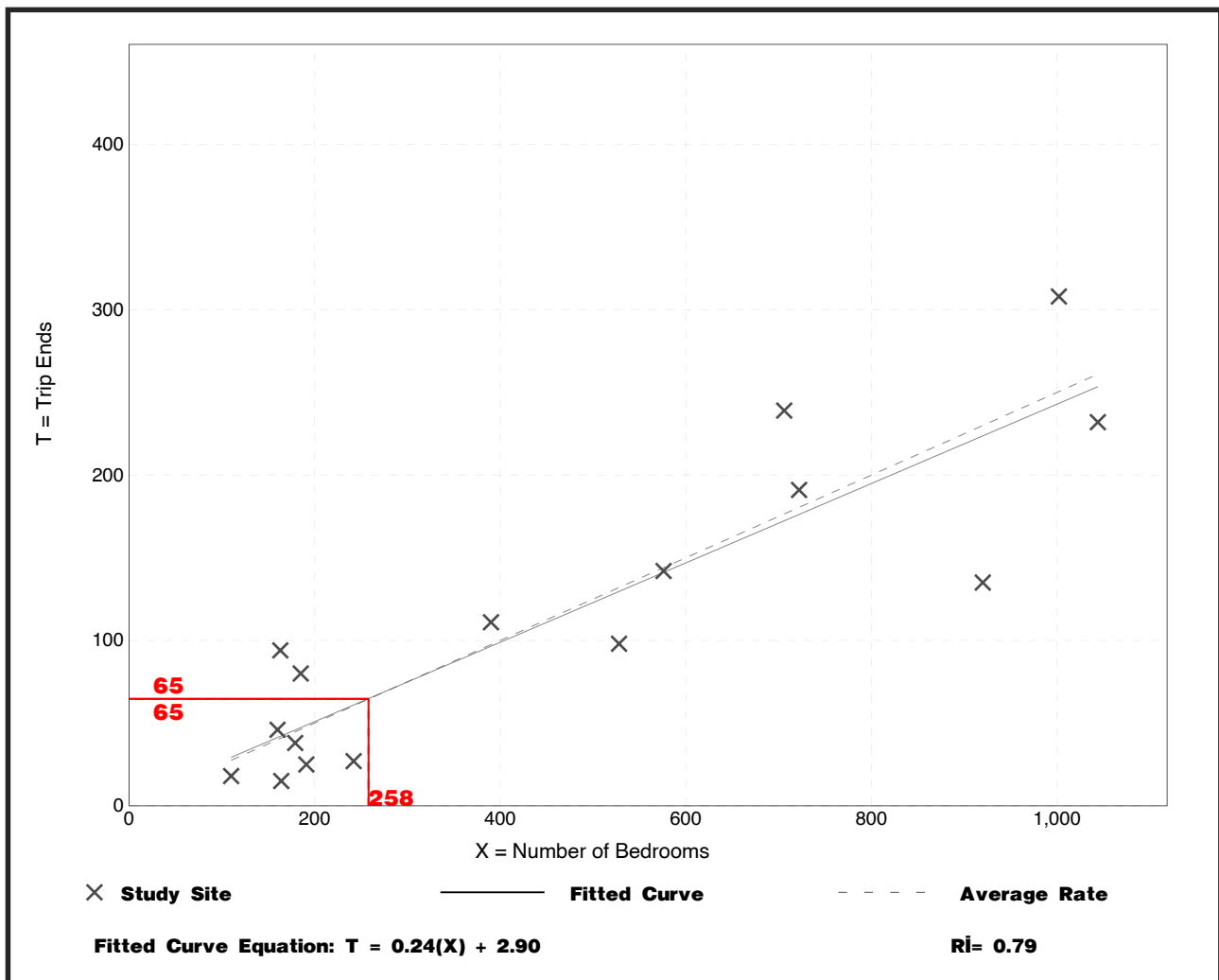
Off-Campus Student Apartment (225)

Vehicle Trip Ends vs: Bedrooms
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: Adjacent to Campus
Number of Studies: 16
Avg. Num. of Bedrooms: 455
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Bedroom

Average Rate	Range of Rates	Standard Deviation
0.25	0.09 - 0.58	0.09

Data Plot and Equation



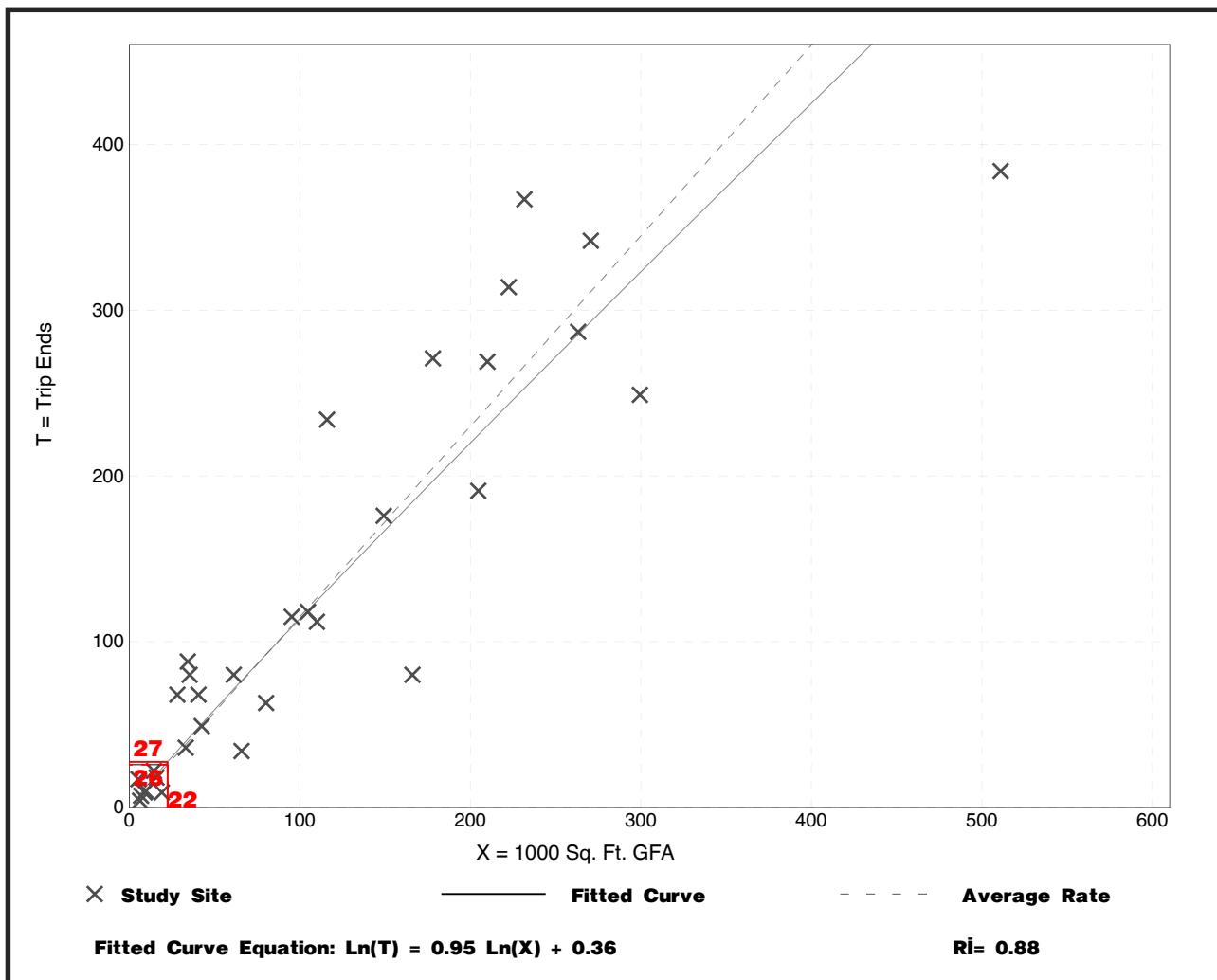
General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 32
 Avg. 1000 Sq. Ft. GFA: 114
 Directional Distribution: 16% entering, 84% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.15	0.47 - 3.23	0.42

Data Plot and Equation



Drive-in Bank (912)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 21
Avg. 1000 Sq. Ft. GFA: 7
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
100.03	32.67 - 408.42	61.61

Data Plot and Equation

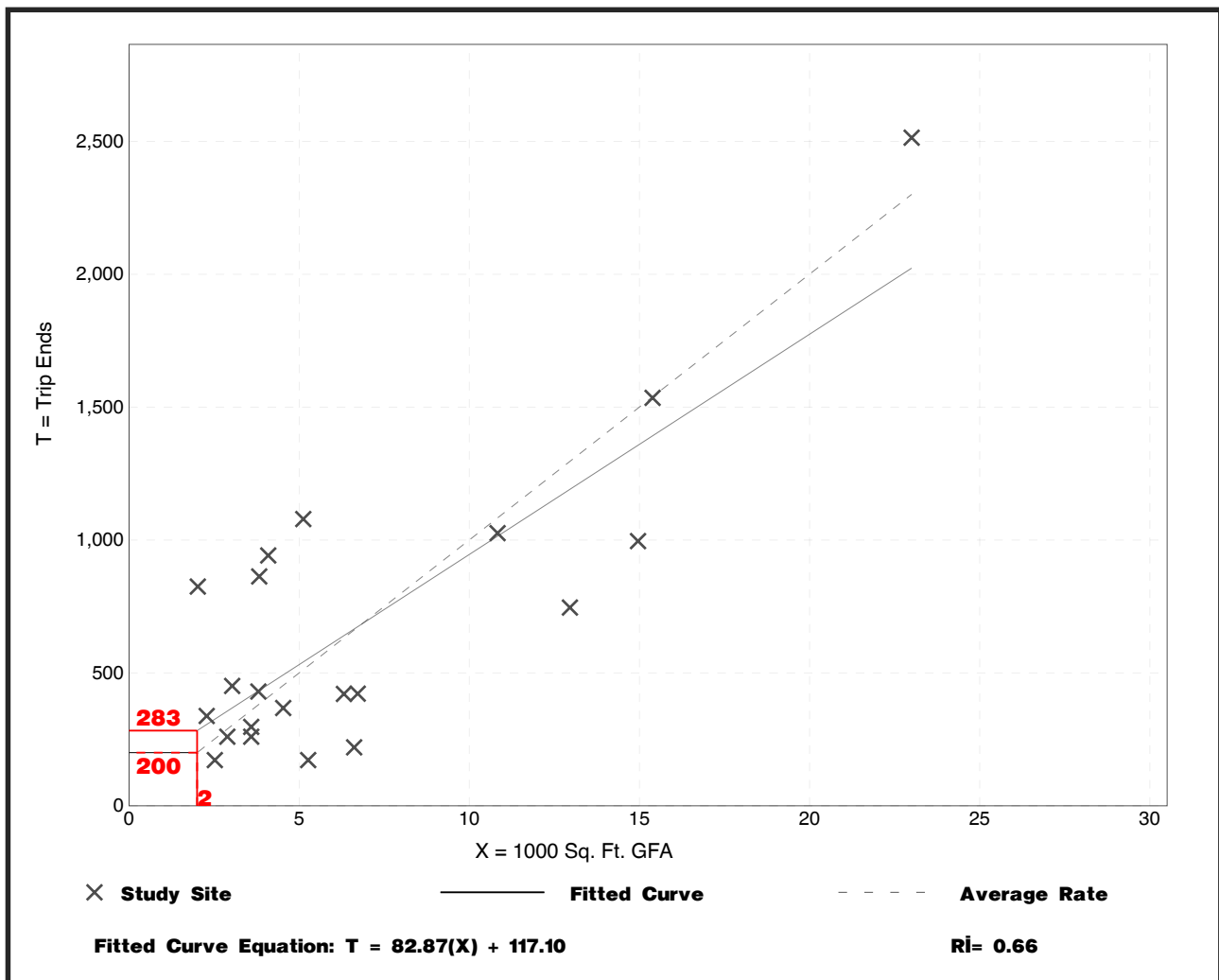


Table 9-7. Design Intersection Sight Distance—Case B1, Left Turn from Stop

U.S. Customary				Metric			
Design Speed (mph)	Stopping Sight Distance (ft)	Intersection Sight Distance for Passenger Cars		Design Speed (km/h)	Stopping Sight Distance (m)	Intersection Sight Distance for Passenger Cars	
		Calculated (ft)	Design (ft)			Calculated (m)	Design (m)
15	80	165.4	170	20	20	41.7	45
20	115	220.5	225	30	35	62.6	65
25	155	275.6	280	40	50	83.4	85
30	200	330.8	335	50	65	104.3	105
35	250	385.9	390	60	85	125.1	130
40	305	441.0	445	70	105	146.0	150
45	360	496.1	500	80	130	166.8	170
50	425	551.3	555	90	160	187.7	190
55	495	606.4	610	100	185	208.5	210
60	570	661.5	665	110	220	229.4	230
65	645	716.6	720	120	250	250.2	255
70	730	771.8	775	130	285	271.1	275
75	820	826.9	830				
80	910	882.0	885				

Note: Intersection sight distance shown is for a stopped passenger car to turn left onto a two-lane highway with no median and grades 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

Sight distance design for left turns at intersections on divided roads or streets should consider multiple design vehicles and median width. If the design vehicle used to determine sight distance for an intersection on a divided road or street is larger than a passenger car, then sight distance for left turns should be checked for that selected design vehicle and for a passenger car as well. If the median on a divided road or street is wide enough to store the design vehicle with a clearance to the through lanes of approximately 3 ft [1 m] at both ends of the vehicle, no separate analysis for the departure sight triangle for left turns is needed on the minor-road approach for the near roadway to the left. In most cases, the departure sight triangle for right turns (Case B2) will provide sufficient sight distance for a passenger car to cross the near roadway to reach the median. Possible exceptions are addressed in the discussion of Case B3.

If the design vehicle can be stored in the median with adequate clearance to the through lanes, a departure sight triangle to the right for left turns should be provided for that design vehicle turning left from the median roadway. Where the median is not wide enough to store the design vehicle, a departure sight triangle should be provided for that design vehicle to turn left from the minor-road approach.

Table 9-9. Design Intersection Sight Distance—Case B2, Right Turn from Stop

U.S. Customary				Metric			
Design Speed (mph)	Stopping Sight Distance (ft)	Intersection Sight Distance for Passenger Cars		Design Speed (km/h)	Stopping Sight Distance (m)	Intersection Sight Distance for Passenger Cars	
		Calculated (ft)	Design (ft)			Calculated (m)	Design (m)
15	80	143.3	145	20	20	36.1	40
20	115	191.1	195	30	35	54.2	55
25	155	238.9	240	40	50	72.3	75
30	200	286.7	290	50	65	90.4	95
35	250	334.4	335	60	85	108.4	110
40	305	382.2	385	70	105	126.5	130
45	360	430.0	430	80	130	144.6	145
50	425	477.8	480	90	160	162.6	165
55	495	525.5	530	100	185	180.7	185
60	570	573.3	575	110	220	198.8	200
65	645	621.1	625	120	250	216.8	220
70	730	668.9	670	130	285	234.9	235
75	820	716.6	720				
80	910	764.4	765				

Note: Intersection sight distance shown is for a stopped passenger car to turn right onto or to cross a two-lane roadway with no median and with grades of 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

9.5.3.2.3 Case B3—Crossing Maneuver from the Minor Road

In most cases, the departure sight triangles for left and right turns onto the major road, as described for Cases B1 and B2, will also provide adequate sight distance for minor-road vehicles to cross the major road. However, in the following situations, it is advisable to check the availability of sight distance for crossing maneuvers:

- where left or right turns or both are not permitted from a particular approach and the crossing maneuver is the only legal maneuver;
- where the crossing vehicle would cross the equivalent width of more than six lanes; or
- where substantial volumes of heavy vehicles cross the roadway and steep grades that might slow the vehicle while its back portion is still in the intersection are present on the departure roadway on the far side of the intersection.

The equation for intersection sight distance in Case B1 (see [Equation 9-1](#)) is used again for the crossing maneuver except that time gaps (t_g) are the same as those for the Right Turn from Stop maneuver, which presents time gaps and appropriate adjustment factors to determine the intersection sight distance along the major road to accommodate crossing maneuvers. At divid-

U.S. Customary	Metric
$d_B = 1.075 \frac{V^2}{a}$ where: d_B = braking distance, ft V = design speed, mph a = deceleration rate, ft/s²	$d_B = 0.039 \frac{V^2}{a}$ where: d_B = braking distance, m V = design speed, km/h a = deceleration rate, m/s²

(3-1)

Studies documented in the literature (19) show that most drivers decelerate at a rate greater than 14.8 ft/s² [4.5 m/s²] when confronted with the need to stop for an unexpected object in the roadway. Approximately 90 percent of all drivers decelerate at rates greater than 11.2 ft/s² [3.4 m/s²]. Such decelerations are within the driver's capability to stay within his or her lane and maintain steering control during the braking maneuver on wet surfaces. Therefore, 11.2 ft/s² [3.4 m/s²] (a comfortable deceleration for most drivers) is recommended as the deceleration threshold for determining stopping sight distance. Implicit in the choice of this deceleration threshold is the assessment that most vehicle braking systems and the tire-pavement friction levels of most roadways are capable of providing a deceleration rate of at least 11.2 ft/s² [3.4 m/s²]. The friction available on most wet pavement surfaces and the capabilities of most vehicle braking systems can provide braking friction that exceeds this deceleration rate.

Table 3-1. Stopping Sight Distance on Level Roadways

U.S. Customary					Metric				
Design Speed (mph)	Brake Reaction Distance (ft)	Braking Distance on Level (ft)	Stopping Sight Distance		Design Speed (km/h)	Brake Reaction Distance (m)	Braking Distance on Level (m)	Stopping Sight Distance	
			Calculated (ft)	Design (ft)				Calculated (m)	Design (m)
15	55.1	21.6	76.7	80	20	13.9	4.6	18.5	20
20	73.5	38.4	111.9	115	30	20.9	10.3	31.2	35
25	91.9	60.0	151.9	155	40	27.8	18.4	46.2	50
30	110.3	86.4	196.7	200	50	34.8	28.7	63.5	65
35	128.6	117.6	246.2	250	60	41.7	41.3	83.0	85
40	147.0	153.6	300.6	305	70	48.7	56.2	104.9	105
45	165.4	194.4	359.8	360	80	55.6	73.4	129.0	130
50	183.8	240.0	423.8	425	90	62.6	92.9	155.5	160
55	202.1	290.3	492.4	495	100	69.5	114.7	184.2	185
60	220.5	345.5	566.0	570	110	76.5	138.8	215.3	220
65	238.9	405.5	644.4	645	120	83.4	165.2	248.6	250
70	257.3	470.3	727.6	730	130	90.4	193.8	284.2	285
75	275.6	539.9	815.5	820	140	97.3	224.8	322.1	325
80	294.0	614.3	908.3	910					
85	313.5	693.5	1007.0	1010					

Note: Brake reaction distance predicated on a time of 2.5 s; deceleration rate of 11.2 ft/s² [3.4 m/s²] used to determine calculated sight distance.

