# **DRAINAGE ANALYSIS**

3 Bay Garage, 3 Dover Road Tax Map 4 Lot 49 3 Dover Road Durham, NH 03824

Prepared for:

Tropic Star Development LLC 321D Lafayette Road Hampton, NH 03842



Prepared by:

Jones & Beach Engineers, Inc.
85 Portsmouth Avenue
P.O. Box 219
Stratham, NH 03885
(603) 772-4746
August 27, 2014
Revised: February 11, 2015

JBE Project No. 14011

#### TABLE OF CONTENTS

- 1. Executive Summary
- 2. Drainage Analysis
  - 2.1. Introduction
  - 2.2. Methodology
  - 2.3. Existing Conditions Analysis
  - 2.4. Proposed Conditions Analysis
  - 2.5. Conclusion
  - 2.6. Drainage Appendix I
    - 2.6.1. 1" 24 Hour Summary Analysis
    - 2.6.2. 2 Year 24 Hour Summary Analysis
    - 2.6.3. 10 Year 24 Hour Complete Analysis
    - 2.6.4. 25 Year 24 Hour Summary Analysis
    - 2.6.5. 50 Year 24 Hour Summary Analysis
    - 2.6.6. 100 Year-24 Hour Summary Analysis
  - 2.7. Drainage Appendix II Proposed Conditions Analysis
    - 2.7.1. 1" 24 Hour Summary Analysis
    - 2.7.2. 2 Year 24 Hour Summary Analysis
    - 2.7.3. 10 Year 24 Hour Complete Analysis
    - 2.7.4. 25 Year 24 Hour Summary Analysis
    - 2.7.5. 50 Year 24 Hour Summary Analysis
    - 2.7.6. 100 Year-24 Hour Summary Analysis
- 3. USGS Map Appendix III
- 4. Web Soil Survey Map Appendix IV
- 5. Aerial Photograph of Site Appendix V
- 6. Operations and Maintenance Manual Appendix VI
- 7. Plans Appendix VII
  - 7.1. Drainage Plan
    - 7.1.1. Pre-Development Drainage Plan
    - 7.1.2. Post-Development Drainage Plan

### 1. EXECUTIVE SUMMARY

The purpose of this project is to remove all existing structures, and replace them with a 3 bay garage with associated parking.

A drainage analysis of the development site and its offsite contributing watershed areas was conducted for the purpose of estimating the peak rate of stormwater runoff and to subsequently design adequate drainage structures. Two models were compiled, one for the area in its existing (pre-construction) condition, and a second for its proposed (post-construction) condition. The analysis was conducted using the USDA SCS TR-20 method within the HydroCAD Stormwater Modeling System environment. A summary of the existing and proposed conditions peak rates of runoff is as follows:

COMPONENT		PEAK DISCHARGE COMPARISON										
	1	"	2 Y	ear	10	Year	25 Y	/ear	50	Year	100	Year
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post
Analysis Point #1	0.24	0.12	1.23	1.16	1.98	1.92	2.56	2.51	3.10	3.03	3.75	3.66
Analysis Point #2	0	0	0.03	0.03	0.07	0.06	0.11	0.08	0.14	0.11	0.18	0.14
Total	0.24	0.12	1.27	1.19	2.05	1.97	2.67	2.59	3.24	3.13	3.94	3.78

The drainage design intent for this site is to maintain the post-development peak flow to the predevelopment peak flow conditions to the extent practicable. This has been accomplished through the use of an underground detention system to maintain the peak discharge.

#### 2.1 INTRODUCTION

The intent of this project is to construct a 2,450 s.f. 3 bay garage on Town of Auburn Tax Map 4 Lot 49.

#### 2.2 METHODOLOGY

The existing and proposed watersheds were modeled utilizing HydroCad stormwater software, version 9.10. The watersheds were analyzed utilizing the SCS TR-20 methodology for hydrograph development and the TR-55 methodology for Time of Concentration (Tc) determination. The Dynamic-Storage-Indicating method for reach and pond routing was utilized. Type III, 24-hour hydrographs were developed for the 1-Inch, 2-year, 10-year, 25-year, 50-year, and 100-year storm events, corresponding to rainfall events of 1", 3.14", 4.75", 6.02", 7.20", and 8.63" respectively.

Existing topography and site features were obtained through on-ground topography completed by Jones & Beach Engineers. Existing soil conditions were derived from soils information obtained from the NRCS Web Soil Survey.

#### 2.3 EXISTING CONDITIONS ANALYSIS

The study area consists of the subject property and upstream contributing area. The study area contains 0.463 acres including offsite contributing areas. The existing site is currently partially developed. The existing site currently contains a single building with parking and utilities.

Two (2) Analysis Points (AP's) were defined for this project.

Analysis Point #1 is defined as the existing south-eastern catch basin. Analysis Point #1 includes drainage from the majority of the building and site.

Analysis Point #2 is defined as the northern and eastern property lines. Analysis Point #2 includes the area of the parcel which drains north and east across the existing property line.

#### 2.4 PROPOSED CONDITIONS ANALYSIS

The proposed site includes a 2,450 sq.ft 3 bay garage. The subject parcel includes unique conditions limiting the storm water features that can be incorporated. The most limiting feature is the size of the site. Underground detention underneath the pavement was used because of this.

As the table in the Executive Summary demonstrates, the proposed peak rates of runoff have been maintained at the existing peak rates of runoff for the analyzed storms to the extent practicable.

#### 2.5 CONCLUSION

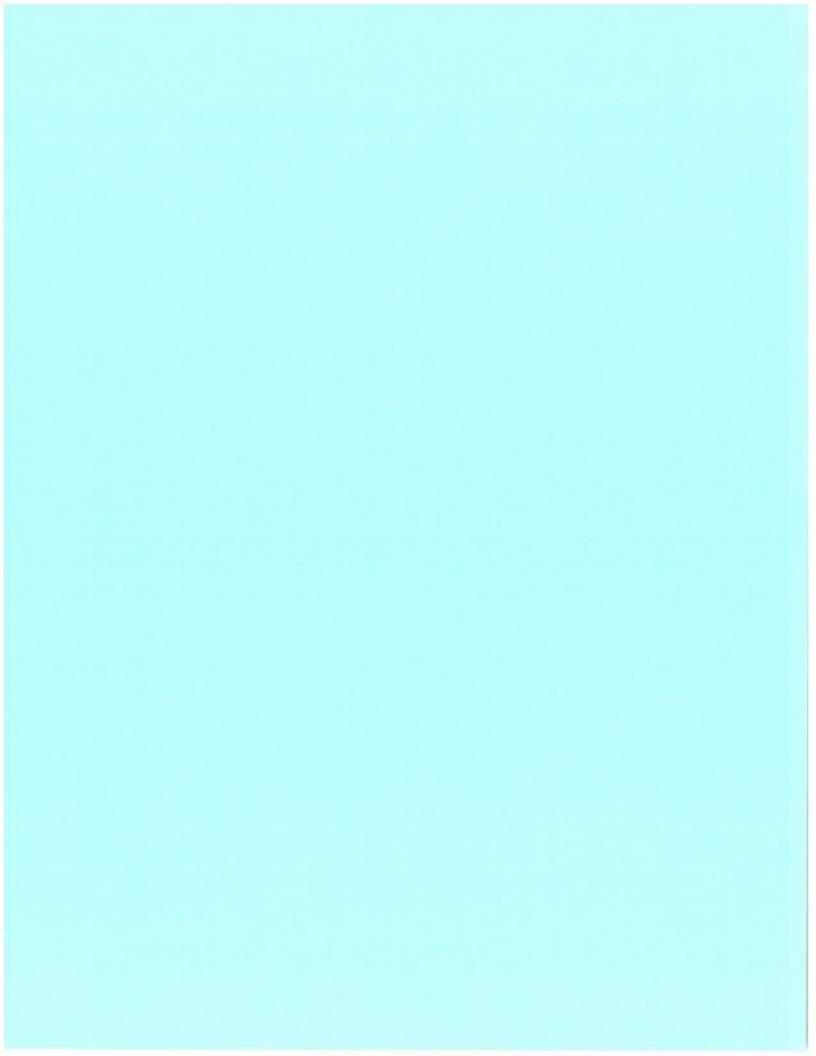
This proposed site development will have minimal effect on abutting infrastructures or properties by way of stormwater runoff or siltation. Peak runoff rate from the proposed site has been maintained to the existing conditions peak rate to the extent practicable.

The area of disturbance is approximately 11,000 square feet.

Respectfully Submitted,
JONES & BEACH ENGINEERS, INC.

Barry W. Gier, P.E.

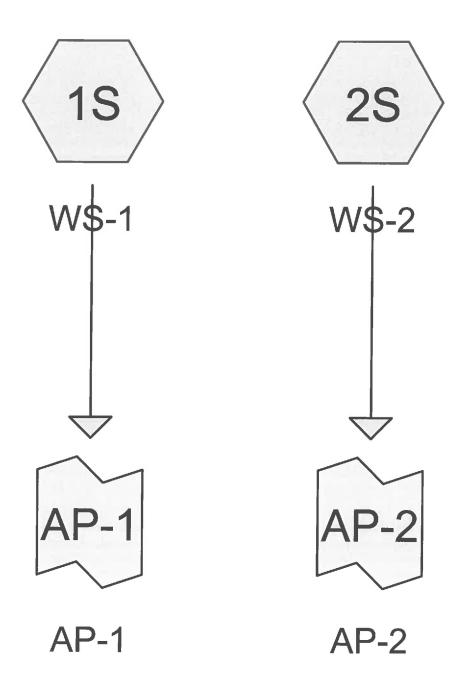
Senior Project Manager



# 2.6 APPENDIX I

# EXISTING CONDITIONS DRAINAGE ANALYSIS

2.6.1	1" 24=Hour Summary Analysis
2.6.2	2-Year 24-Hour Summary Analysis
2.6.3	10-Year 24-Hour Complete Analysis
2.6.4	25-Year 24-Hour Summary Analysis
2.6.5	50-Year 24-Hour Summary Analysis
2.6.6	100-Year 24-Hour Summary Analysis











Routing Diagram for 14011-EXISTING
Prepared by Jones & Beach Engineers, Printed 2/16/2015
HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

## **14011-EXISTING**

Prepared by Jones & Beach Engineers
HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015 Page 2

# Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.118	74	>75% Grass cover, Good, HSG C (1S, 2S)
0.346	98	Paved parking, HSG C (1S, 2S)
0.463	92	TOTAL AREA

## **14011-EXISTING**

Prepared by Jones & Beach Engineers

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015 Page 3

# Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.463	HSG C	1S, 2S
0.000	HSG D	
0.000	Other	
0.463		<b>TOTAL AREA</b>

Printed 2/16/2015

Page 4

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1

Runoff Area=19,106 sf 78.66% Impervious Runoff Depth>0.42" Flow Length=259' Tc=4.4 min CN=93 Runoff=0.24 cfs 0.015 af

Subcatchment 2S: WS-2

Runoff Area=1,081 sf 2.96% Impervious Runoff Depth>0.02" Flow Length=12' Slope=0.1900 '/' Tc=1.1 min CN=75 Runoff=0.00 cfs 0.000 af

Link AP-1: AP-1

Inflow=0.24 cfs 0.015 af Primary=0.24 cfs 0.015 af

Link AP-2: AP-2

Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.015 af Average Runoff Depth = 0.40" 25.40% Pervious = 0.118 ac 74.60% Impervious = 0.346 ac

Printed 2/16/2015

Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1

Runoff Area=19,106 sf 78.66% Impervious Runoff Depth>2.26" Flow Length=259' Tc=4.4 min CN=93 Runoff=1.23 cfs 0.082 af

Subcatchment 2S: WS-2

Runoff Area=1,081 sf 2.96% Impervious Runoff Depth>0.96"

Flow Length=12' Slope=0.1900 '/' Tc=1.1 min CN=75 Runoff=0.03 cfs 0.002 af

Link AP-1: AP-1

Inflow=1.23 cfs 0.082 af Primary=1.23 cfs 0.082 af

Link AP-2: AP-2

Inflow=0.03 cfs 0.002 af Primary=0.03 cfs 0.002 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.084 af Average Runoff Depth = 2.19" 25.40% Pervious = 0.118 ac 74.60% Impervious = 0.346 ac

Printed 2/16/2015

Page 1

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1

Runoff Area=19,106 sf 78.66% Impervious Runoff Depth>3.74" Flow Length=259' Tc=4.4 min CN=93 Runoff=1.98 cfs 0.137 af

Subcatchment 2S: WS-2

Runoff Area=1,081 sf 2.96% Impervious Runoff Depth>2.09" Flow Length=12' Slope=0.1900 '/' Tc=1.1 min CN=75 Runoff=0.07 cfs 0.004 af

Link AP-1: AP-1

Inflow=1.98 cfs 0.137 af Primary=1.98 cfs 0.137 af

Link AP-2: AP-2

Inflow=0.07 cfs 0.004 af Primary=0.07 cfs 0.004 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.141 af Average Runoff Depth = 3.65" 25.40% Pervious = 0.118 ac 74.60% Impervious = 0.346 ac

Printed 2/16/2015

Page 2

### Summary for Subcatchment 1S: WS-1

Runoff 1.98 cfs @ 12.06 hrs, Volume=

0.137 af. Depth> 3.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

	Area (sf)	CN I	Description		
	15,028	98	Paved park	ing, HSG C	
	4,078	74 >	>75% Gras	s cover, Go	ood, HSG C
	19,106	93 \	Neighted A	verage	
	4,078	2	21.34% Pei	vious Area	
	15,028	7	<sup>7</sup> 8.66% lmp	pervious Ar	ea
_					
Tc	0	Slope	Velocity	Capacity	Description
(min)	(feet)_	(ft/ft)	(ft/sec)	(cfs)	
2.9	22	0.0200	0.12		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.14"
0.6	53	0.0294	1.40		Sheet Flow,
					Smooth surfaces n= 0.011 P2= 3.14"
0.9	184	0.0310	3.57		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
4.4	259	Total			

## **Summary for Subcatchment 2S: WS-2**

0.07 cfs @ 12.02 hrs, Volume= Runoff

0.004 af, Depth> 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

_	А	rea (sf)	CN	Description		
		1,049	74	>75% Gras	s cover, Go	ood, HSG C
_		32	98	Paved park	ing, HSG C	
		1,081	75	Weighted A	verage	
		1,049		97.04% Per		1
		32		2.96% Impe	ervious Are	a
_	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	1.1	12	0.1900	0.19		Sheet Flow,

Grass: Dense n= 0.240 P2= 3.14"

# **Summary for Link AP-1: AP-1**

Inflow Area = 0.439 ac, 78.66% Impervious, Inflow Depth > 3.74" for 10 YR event

Inflow 1.98 cfs @ 12.06 hrs, Volume= 0.137 af

Primary 1.98 cfs @ 12.06 hrs, Volume= 0.137 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

#### **14011-EXISTING**

Type III 24-hr 10 YR Rainfall=4.75"

Prepared by Jones & Beach Engineers

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015

Page 3

## **Summary for Link AP-2: AP-2**

Inflow Area = 0.025 ac, 2.96% Impervious, Inflow Depth > 2.09" for 10 YR event

Inflow = 0.07 cfs @ 12.02 hrs, Volume= 0.004 af

Primary = 0.07 cfs @ 12.02 hrs, Volume= 0.004 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Printed 2/16/2015

Page 6

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1

Runoff Area=19,106 sf 78.66% Impervious Runoff Depth>4.91" Flow Length=259' Tc=4.4 min CN=93 Runoff=2.56 cfs 0.179 af

Subcatchment 2S: WS-2

Runoff Area=1,081 sf 2.96% Impervious Runoff Depth>3.08" Flow Length=12' Slope=0.1900 '/' Tc=1.1 min CN=75 Runoff=0.11 cfs 0.006 af

Inflow=2.56 cfs 0.179 af Primary=2.56 cfs 0.179 af

Link AP-2: AP-2

Link AP-1: AP-1

Inflow=0.11 cfs 0.006 af Primary=0.11 cfs 0.006 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.186 af Average Runoff Depth = 4.81" 25.40% Pervious = 0.118 ac 74.60% Impervious = 0.346 ac

Printed 2/16/2015

Page 7

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1

Runoff Area=19,106 sf 78.66% Impervious Runoff Depth>6.00" Flow Length=259' Tc=4.4 min CN=93 Runoff=3.10 cfs 0.219 af

Subcatchment 2S: WS-2

Runoff Area=1,081 sf 2.96% Impervious Runoff Depth>4.05"

Flow Length=12' Slope=0.1900 '/' Tc=1.1 min CN=75 Runoff=0.14 cfs 0.008 af

Link AP-1: AP-1

Inflow=3.10 cfs 0.219 af Primary=3.10 cfs 0.219 af

Link AP-2: AP-2

Inflow=0.14 cfs 0.008 af Primary=0.14 cfs 0.008 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.228 af Average Runoff Depth = 5.89" 25.40% Pervious = 0.118 ac 74.60% Impervious = 0.346 ac

#### 14011-EXISTING

Type III 24-hr 100 YR Rainfall=8.63"

Prepared by Jones & Beach Engineers

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015

Page 8

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1

Runoff Area=19,106 sf 78.66% Impervious Runoff Depth>7.32" Flow Length=259' Tc=4.4 min CN=93 Runoff=3.75 cfs 0.267 af

Subcatchment 2S: WS-2

Runoff Area=1,081 sf 2.96% Impervious Runoff Depth>5.28" Flow Length=12' Slope=0.1900 '/' Tc=1.1 min CN=75 Runoff=0.18 cfs 0.011 af

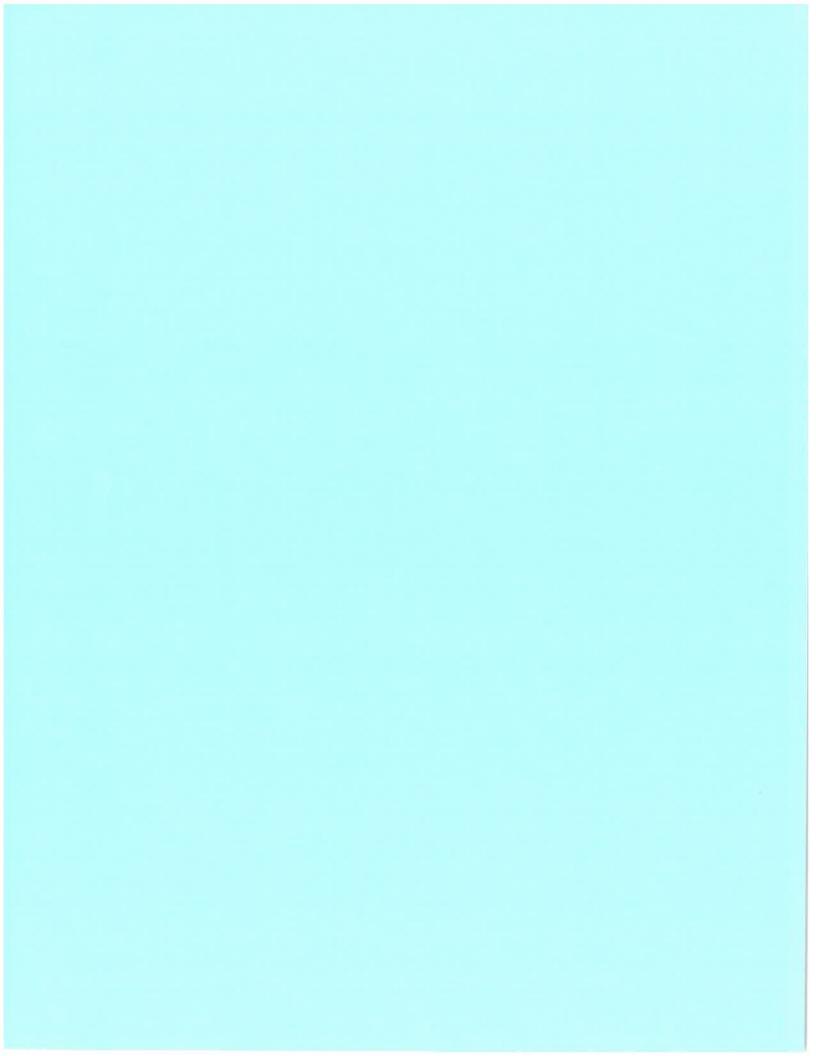
Link AP-1: AP-1

Inflow=3.75 cfs 0.267 af Primary=3.75 cfs 0.267 af

Link AP-2: AP-2

Inflow=0.18 cfs 0.011 af Primary=0.18 cfs 0.011 af

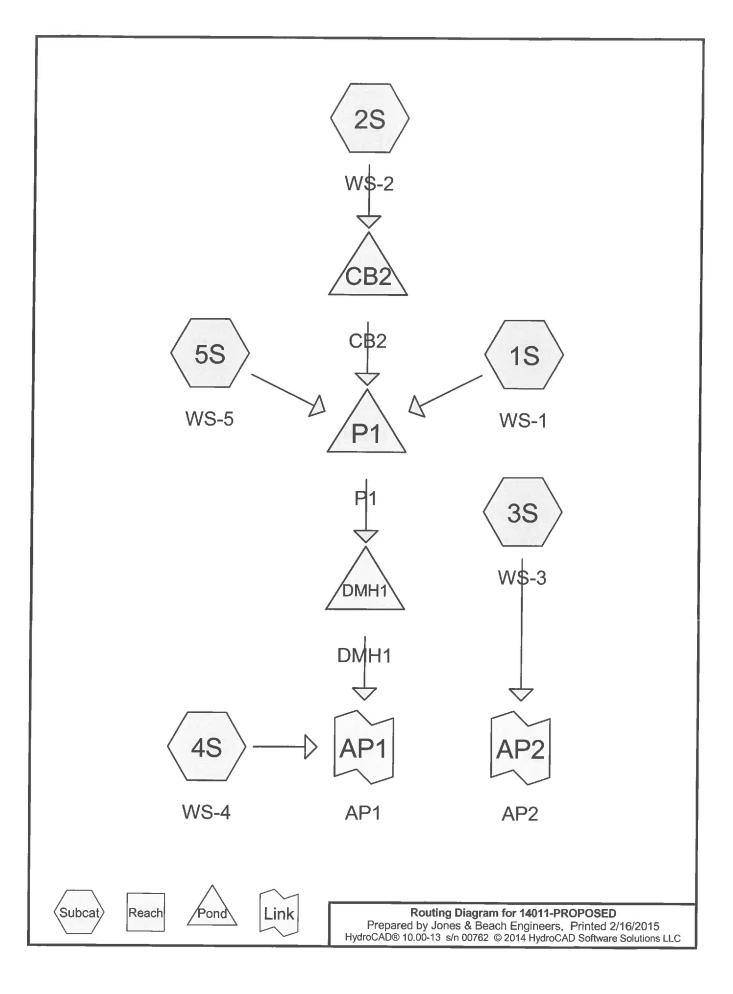
Total Runoff Area = 0.463 ac Runoff Volume = 0.278 af Average Runoff Depth = 7.21" 25.40% Pervious = 0.118 ac 74.60% Impervious = 0.346 ac



# 2.7 APPENDIX II

# PROPOSED CONDITIONS DRAINAGE ANALYSIS

2.7.1	1" 24-Hour Summary Analysis
2.7.2	2-Year 24-Hour Summary Analysis
2.7.3	10-Year 24-Hour Complete Analysis
2.7.4	25-Year 24-Hour Summary Analysis
2.7.5	50-Year 24-Hour Summary Analysis
2.7.6	100-Year 24-Hour Summary Analysis



Prepared by Jones & Beach Engineers
HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015 Page 2

# **Area Listing (all nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
0.103	74	>75% Grass cover, Good, HSG C (1S, 2S, 3S, 4S)
0.305	98	Paved parking, HSG C (1S, 2S, 3S, 4S)
0.056	98	Roofs, HSG C (5S)
0.463	93	TOTAL AREA

Prepared by Jones & Beach Engineers
HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015 Page 3

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
0.463	HSG C	1S, 2S, 3S, 4S, 5S
0.000	HSG D	
0.000	Other	
0.463		<b>TOTAL AREA</b>

Prepared by Jones & Beach Engineers HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC Printed 2/16/2015

Page 4

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1	Runoff Area=1,944 sf 86.78% Impervious Runoff Depth>0.53" Flow Length=61' Slope=0.0230 '/' Tc=0.8 min CN=95 Runoff=0.03 cfs 0.002 af
Subcatchment 2S: WS-2	Runoff Area=5,063 sf 84.40% Impervious Runoff Depth>0.47" Flow Length=142' Tc=2.5 min CN=94 Runoff=0.08 cfs 0.005 af
Subcatchment 3S: WS-3	Runoff Area=852 sf 3.64% Impervious Runoff Depth>0.02" Flow Length=12' Slope=0.1700 '/' Tc=1.1 min CN=75 Runoff=0.00 cfs 0.000 af
Subcatchment 4S: WS-4	Runoff Area=9,878 sf 73.70% Impervious Runoff Depth>0.37" Flow Length=212' Tc=2.8 min CN=92 Runoff=0.12 cfs 0.007 af
Subcatchment 5S: WS-5	Runoff Area=2,449 sf 100.00% Impervious Runoff Depth>0.75" Flow Length=15' Slope=0.1500 '/' Tc=0.1 min CN=98 Runoff=0.06 cfs 0.004 af
Pond CB2: CB2	Peak Elev=34.13' Storage=0.000 af Inflow=0.08 cfs 0.005 af 12.0" Round Culvert n=0.012 L=63.0' S=0.0103 '/' Outflow=0.07 cfs 0.005 af
Pond DMH1: DMH1	Peak Elev=33.30' Storage=0.000 af Inflow=0.04 cfs 0.006 af

12.0" Round Culvert n=0.012 L=9.0' S=0.0500 '/' Outflow=0.04 cfs 0.006 af

Pond P1: P1 Peak Elev=33.47' Storage=219 cf Inflow=0.16 cfs 0.010 af

Outflow=0.04 cfs 0.006 af

Link AP1: AP1 Inflow=0.12 cfs 0.013 af Primary=0.12 cfs 0.013 af

Link AP2: AP2 Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

> Total Runoff Area = 0.463 ac Runoff Volume = 0.017 af Average Runoff Depth = 0.44" 22.12% Pervious = 0.103 ac 77.88% Impervious = 0.361 ac

Printed 2/16/2015

Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1	Runoff Area=1,944 sf 86.78% Impervious Runoff Depth>2.45" Flow Length=61' Slope=0.0230 '/' Tc=0.8 min CN=95 Runoff=0.14 cfs 0.009 af
Subcatchment 2S: WS-2	Runoff Area=5,063 sf 84.40% Impervious Runoff Depth>2.35" Flow Length=142' Tc=2.5 min CN=94 Runoff=0.35 cfs 0.023 af
Subcatchment 3S: WS-3	Runoff Area=852 sf 3.64% Impervious Runoff Depth>0.96" Flow Length=12' Slope=0.1700 '/' Tc=1.1 min CN=75 Runoff=0.03 cfs 0.002 af
Subcatchment 4S: WS-4	Runoff Area=9,878 sf 73.70% Impervious Runoff Depth>2.17" Flow Length=212' Tc=2.8 min CN=92 Runoff=0.65 cfs 0.041 af
Subcatchment 5S: WS-5	Runoff Area=2,449 sf 100.00% Impervious Runoff Depth>2.72" Flow Length=15' Slope=0.1500 '/' Tc=0.1 min CN=98 Runoff=0.20 cfs 0.013 af
Pond CB2: CB2	Peak Elev=34.29' Storage=0.000 af Inflow=0.35 cfs 0.023 af 12.0" Round Culvert n=0.012 L=63.0' S=0.0103 '/' Outflow=0.35 cfs 0.023 af
Pond DMH1: DMH1	Peak Elev=33.36' Storage=0.000 af Inflow=0.52 cfs 0.040 af 12.0" Round Culvert n=0.012 L=9.0' S=0.0500 '/' Outflow=0.52 cfs 0.040 af
Pond P1: P1	Peak Elev=33.86' Storage=386 cf Inflow=0.66 cfs 0.045 af Outflow=0.52 cfs 0.040 af
Link AP1: AP1	Inflow=1.16 cfs 0.081 af Primary=1.16 cfs 0.081 af
Link AP2: AP2	Inflow=0.03 cfs 0.002 af Primary=0.03 cfs 0.002 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.087 af Average Runoff Depth = 2.26" 22.12% Pervious = 0.103 ac 77.88% Impervious = 0.361 ac

Type III 24-hr 10 YR Rainfall=4.75"

Prepared by Jones & Beach Engineers HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC Printed 2/16/2015

Page 1

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1	Runoff Area=1,944 sf 86.78% Impervious Runoff Depth>3.93" Flow Length=61' Slope=0.0230 '/' Tc=0.8 min CN=95 Runoff=0.23 cfs 0.015 af
Subcatchment 2S: WS-2	Runoff Area=5,063 sf 84.40% Impervious Runoff Depth>3.84" Flow Length=142' Tc=2.5 min CN=94 Runoff=0.56 cfs 0.037 af
Subcatchment 3S: WS-3	Runoff Area=852 sf 3.64% Impervious Runoff Depth>2.09" Flow Length=12' Slope=0.1700 '/' Tc=1.1 min CN=75 Runoff=0.06 cfs 0.003 af
Subcatchment 4S: WS-4	Runoff Area=9,878 sf 73.70% Impervious Runoff Depth>3.64" Flow Length=212' Tc=2.8 min CN=92 Runoff=1.06 cfs 0.069 af
Subcatchment 5S: WS-5	Runoff Area=2,449 sf 100.00% Impervious Runoff Depth>4.19" Flow Length=15' Slope=0.1500 '/' Tc=0.1 min CN=98 Runoff=0.30 cfs 0.020 af
Pond CB2: CB2	Peak Elev=34.38' Storage=0.000 af Inflow=0.56 cfs 0.037 af 12.0" Round Culvert n=0.012 L=63.0' S=0.0103 '/' Outflow=0.56 cfs 0.037 af
Pond DMH1: DMH1	Peak Elev=33.48' Storage=0.000 af Inflow=0.88 cfs 0.067 af 12.0" Round Culvert n=0.012 L=9.0' S=0.0500 '/' Outflow=0.88 cfs 0.067 af
Pond P1: P1	Peak Elev=34.06' Storage=463 cf Inflow=1.04 cfs 0.071 af Outflow=0.88 cfs 0.067 af

Link AP1: AP1 Inflow=1.92 cfs 0.136 af Primary=1.92 cfs 0.136 af

Link AP2: AP2 Inflow=0.06 cfs 0.003 af Primary=0.06 cfs 0.003 af

> Total Runoff Area = 0.463 ac Runoff Volume = 0.144 af Average Runoff Depth = 3.72" 22.12% Pervious = 0.103 ac 77.88% Impervious = 0.361 ac

Page 2

## **Summary for Subcatchment 1S: WS-1**

Runoff

0.23 cfs @ 12.01 hrs, Volume=

0.015 af, Depth> 3.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

_	A	rea (sf)	CN	Description						
		1,687	98	Paved park	ing, HSG C	<del></del>				
_		257	74	>75% Grass cover, Good, HSG C						
		1,944 257 1,687		Weighted A 13.22% Pei 86.78% Imp	vious Area					
_	Tc (min)	Length (feet)	Slope (ft/ft)	-	Capacity (cfs)	Description				
	0.8	61	0.0230	1.31		Sheet Flow, Smooth surfaces	n= 0 011	P2= 3 14"		

### Summary for Subcatchment 2S: WS-2

Runoff

0.56 cfs @ 12.04 hrs, Volume=

0.037 af, Depth> 3.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

_	A	rea (sf)	CN	Description					
		790							
_		4,273							
		5,063		94 Weighted Average					
		790		15.60% Pe					
		4,273 84.40% Impervious Area							
	Тс	Longth	Slope	Volocity	Conneity	Description			
		Length (feet)	Slope (ft/ft)		Capacity	Description			
_	(min)				(cfs)				
	1.7	14	0.0300	0.13		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.14"			
	0.4	36	0.0500	1.60		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 3.14"			
	0.4	92	0.0300	3.52		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	2.5	142	Total		·				

# Summary for Subcatchment 3S: WS-3

Runoff

0.06 cfs @ 12.02 hrs, Volume=

0.003 af, Depth> 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

Type III 24-hr 10 YR Rainfall=4.75"

Prepared by Jones & Beach Engineers

Printed 2/16/2015

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Page 3

_	A	rea (sf)	CN	Description	escription					
		31	98	Paved park	ing, HSG C	;	<u> </u>		-	_
		821	74		75% Grass cover, Good, HSG C					
	852 75 Weighted Average								_	
	821 96.36% Pervious Area									
	31 3.64% Impervious Area					а				
	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description				
	1.1	12	0.1700	0.18		Sheet Flow, Grass: Dense	n= 0 240	P2= 3 14"		_

## **Summary for Subcatchment 4S: WS-4**

Runoff

1.06 cfs @ 12.05 hrs, Volume=

0.069 af, Depth> 3.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

	A	rea (sf)	CN	Description					
		2,598	74	>75% Gras	s cover. Go	ood, HSG C			
		7,280			ing, HSG C	·			
		9,878	92						
		2,598			rvious Area				
		7,280		73.70% Imp	pervious Ar	ea			
	_		01						
	Tc	Length	Slope		Capacity	Description			
	<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	1.4	14	0.0480	0.16		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.14"			
	0.4	36	0.0500	1.60		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 3.14"			
	1.0	162	0.0190	2.80		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	2.8	212	Total						

# Summary for Subcatchment 5S: WS-5

Runoff

0.30 cfs @ 12.00 hrs, Volume=

0.020 af, Depth> 4.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YR Rainfall=4.75"

Area (st	f) CN	Description	
2,44	9 98	Roofs, HSG C	
2,44	9	100.00% Impervious Area	

Type III 24-hr 10 YR Rainfall=4.75"

Prepared by Jones & Beach Engineers HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC Printed 2/16/2015

Page 4

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	0.1	15	0.1500	2.09		Sheet Flow, Smooth surfaces	n= 0.011	P2= 3.14"	

## Summary for Pond CB2: CB2

Inflow Area = 0.116 ac, 84.40% Impervious, Inflow Depth > 3.84" for 10 YR event

Inflow 0.56 cfs @ 12.04 hrs, Volume= 0.037 af

Outflow = 0.56 cfs @ 12.04 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.1 min

Primary 0.56 cfs @ 12.04 hrs, Volume= 0.037 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 34.38' @ 12.04 hrs Surf.Area= 0.000 ac Storage= 0.000 af Flood Elev= 38.40' Surf.Area= 0.000 ac Storage= 0.001 af

Plug-Flow detention time= 0.5 min calculated for 0.037 af (100% of inflow) Center-of-Mass det. time= 0.3 min (746.8 - 746.5)

<u>Volume</u>	Invert	Avail.Storage	Storage Description
#1	34.00'	0.001 af	4.00'D x 4.40'H Vertical Cone/Cylinder
Device	Routing	Invert Ou	ıtlet Devices
#1	Primary	Inle	.0" Round Culvert L= 63.0' Ke= 0.500 et / Outlet Invert= 34.00' / 33.35' S= 0.0103 '/' Cc= 0.900 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.04 hrs HW=34.37' (Free Discharge) 1=Culvert (Inlet Controls 0.55 cfs @ 2.07 fps)

# **Summary for Pond DMH1: DMH1**

Inflow Area = 0.217 ac, 88.93% Impervious, Inflow Depth > 3.70" for 10 YR event

Inflow 0.067 af

0.88 cfs @ 12.07 hrs, Volume= 0.88 cfs @ 12.07 hrs, Volume= Outflow = 0.067 af, Atten= 0%, Lag= 0.0 min

Primary 0.88 cfs @ 12.07 hrs, Volume= 0.067 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 33.48' @ 12.07 hrs Surf.Area= 0.000 ac Storage= 0.000 af

Flood Elev= 37.30' Surf.Area= 0.000 ac Storage= 0.001 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 0.0 min (768.9 - 768.9)

Volume	Invert	Avail.Storage	Storage Description
#1	33.30'	0.001 af	4.00'D x 4.00'H Vertical Cone/Cylinder
Device	Routing	Invert Ou	itlet Devices
#1	Primary	Inle	.0" Round Culvert L= 9.0' Ke= 0.500 et / Outlet Invert= 33.00' / 32.55' S= 0.0500 '/' Cc= 0.900 0.012, Flow Area= 0.79 sf

Type III 24-hr 10 YR Rainfall=4.75"

Prepared by Jones & Beach Engineers HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC Printed 2/16/2015

Page 5

Primary OutFlow Max=0.85 cfs @ 12.07 hrs HW=33.47' (Free Discharge) 1=Culvert (Inlet Controls 0.85 cfs @ 2.34 fps)

## **Summary for Pond P1: P1**

Inflow Area =

0.217 ac, 88.93% Impervious, Inflow Depth > 3.95" for 10 YR event

Inflow =

1.04 cfs @ 12.02 hrs, Volume=

0.071 af

Outflow = 0.88 cfs @ 12.07 hrs, Volume= 0.88 cfs @ 12.07 hrs, Volume=

0.067 af, Atten= 15%, Lag= 2.8 min

Primary =

0.067 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 34.06' @ 12.07 hrs Surf.Area= 591 sf Storage= 463 cf

Flood Elev= 35.08' Surf.Area= 591 sf Storage= 740 cf

Plug-Flow detention time= 50.7 min calculated for 0.067 af (93% of inflow)

Center-of-Mass det. time= 27.4 min ( 768.9 - 741.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	32.75'	425 cf	24.83'W x 23.80'L x 2.33'H Field A
			1,379 cf Overall - 316 cf Embedded = 1,063 cf x 40.0% Voids
#2A	33.25'	316 cf	StormTech SC-310 x 21 Inside #1
			Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf
			Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap
			Row Length Adjustment= +0.44' x 2.07 sf x 7 rows
			Now Length Adjustment To.44 X 2.07 St X 7 Tows

741 cf Total Available Storage

Storage Group A created with Chamber Wizard

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Device 4	33.35'	6.0" Vert. Orifice/Grate C= 0.600
#2	Device 4	33.75'	6.0" Vert. Orifice/Grate C= 0.600
#3	Device 4	34.55'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			1.0' Crest Height
#4	Primary	33.00'	<b>12.0" Round Culvert</b> L= 14.0' Ke= 0.500
			Inlet / Outlet Invert= 33.00' / 32.85' S= 0.0107 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.85 cfs @ 12.07 hrs HW=34.05' (Free Discharge)

**4=Culvert** (Passes 0.85 cfs of 2.54 cfs potential flow)

-1=Orifice/Grate (Orifice Controls 0.63 cfs @ 3.21 fps)

-2=Orifice/Grate (Orifice Controls 0.22 cfs @ 1.85 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Link AP1: AP1

Inflow Area =

0.444 ac, 81.15% Impervious, Inflow Depth > 3.67" for 10 YR event

Inflow

1.92 cfs @ 12.05 hrs, Volume=

0.136 af

Primary

1.92 cfs @ 12.05 hrs, Volume=

0.136 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Type III 24-hr 10 YR Rainfall=4.75"

Prepared by Jones & Beach Engineers

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Page 6

Printed 2/16/2015

## **Summary for Link AP2: AP2**

Inflow Area = 0.020 ac, 3.64% Impervious, Inflow Depth > 2.09" for 10 YR event

Inflow = 0.06 cfs @ 12.02 hrs, Volume= 0.003 af

Primary = 0.06 cfs @ 12.02 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Prepared by Jones & Beach Engineers
HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Printed 2/16/2015

Page 6

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1	Runoff Area=1,944 sf 86.78% Impervious Runoff Depth>5.10" Flow Length=61' Slope=0.0230 '/' Tc=0.8 min CN=95 Runoff=0.29 cfs 0.019 af
Subcatchment 2S: WS-2	Runoff Area=5,063 sf 84.40% Impervious Runoff Depth>5.01" Flow Length=142' Tc=2.5 min CN=94 Runoff=0.72 cfs 0.049 af
Subcatchment 3S: WS-3	Runoff Area=852 sf 3.64% Impervious Runoff Depth>3.08" Flow Length=12' Slope=0.1700 '/' Tc=1.1 min CN=75 Runoff=0.08 cfs 0.005 af
Subcatchment 4S: WS-4	Runoff Area=9,878 sf 73.70% Impervious Runoff Depth>4.81" Flow Length=212' Tc=2.8 min CN=92 Runoff=1.38 cfs 0.091 af
Subcatchment 5S: WS-5	Runoff Area=2,449 sf 100.00% Impervious Runoff Depth>5.35" Flow Length=15' Slope=0.1500 '/' Tc=0.1 min CN=98 Runoff=0.38 cfs 0.025 af
Pond CB2: CB2	Peak Elev=34.43' Storage=0.000 af Inflow=0.72 cfs 0.049 af 12.0" Round Culvert n=0.012 L=63.0' S=0.0103 '/' Outflow=0.72 cfs 0.048 af
Pond DMH1: DMH1	Peak Elev=33.56' Storage=0.000 af Inflow=1.16 cfs 0.088 af 12.0" Round Culvert n=0.012 L=9.0' S=0.0500 '/' Outflow=1.16 cfs 0.088 af
Pond P1: P1	Peak Elev=34.20' Storage=514 cf Inflow=1.33 cfs 0.093 af Outflow=1.16 cfs 0.088 af
Link AP1: AP1	Inflow=2.51 cfs 0.179 af Primary=2.51 cfs 0.179 af
Link AP2: AP2	Inflow=0.08 cfs 0.005 af Primary=0.08 cfs 0.005 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.188 af Average Runoff Depth = 4.88" 22.12% Pervious = 0.103 ac 77.88% Impervious = 0.361 ac Prepared by Jones & Beach Engineers

Printed 2/16/2015

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Page 7

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1 Runoff Area=1,944 sf 86.78% Impervious Runoff Depth>6.19"

Flow Length=61' Slope=0.0230 '/' Tc=0.8 min CN=95 Runoff=0.35 cfs 0.023 af

Subcatchment 2S: WS-2 Runoff Area=5,063 sf 84.40% Impervious Runoff Depth>6.10"

Flow Length=142' Tc=2.5 min CN=94 Runoff=0.87 cfs 0.059 af

Subcatchment 3S: WS-3 Runoff Area=852 sf 3.64% Impervious Runoff Depth>4.05"

Flow Length=12' Slope=0.1700 '/' Tc=1.1 min CN=75 Runoff=0.11 cfs 0.007 af

Subcatchment 4S: WS-4 Runoff Area=9,878 sf 73.70% Impervious Runoff Depth>5.90"

Flow Length=212' Tc=2.8 min CN=92 Runoff=1.67 cfs 0.111 af

Subcatchment 5S: WS-5 Runoff Area=2,449 sf 100.00% Impervious Runoff Depth>6.42"

Flow Length=15' Slope=0.1500 '/' Tc=0.1 min CN=98 Runoff=0.45 cfs 0.030 af

Pond CB2: CB2 Peak Elev=34.48' Storage=0.000 af Inflow=0.87 cfs 0.059 af

12.0" Round Culvert n=0.012 L=63.0' S=0.0103 '/' Outflow=0.88 cfs 0.059 af

Pond DMH1: DMH1 Peak Elev=33.62' Storage=0.000 af Inflow=1.38 cfs 0.108 af

12.0" Round Culvert n=0.012 L=9.0' S=0.0500 '/' Outflow=1.38 cfs 0.108 af

Pond P1: P1 Peak Elev=34.35' Storage=561 cf Inflow=1.60 cfs 0.112 af

Outflow=1.38 cfs 0.108 af

Link AP1: AP1 Inflow=3.03 cfs 0.219 af

Primary=3.03 cfs 0.219 af

Link AP2: AP2 Inflow=0.11 cfs 0.007 af

Primary=0.11 cfs 0.007 af

Total Runoff Area = 0.463 ac Runoff Volume = 0.230 af Average Runoff Depth = 5.96" 22.12% Pervious = 0.103 ac 77.88% Impervious = 0.361 ac

Type III 24-hr 100 YR Rainfall=8.63"

Prepared by Jones & Beach Engineers

Printed 2/16/2015

HydroCAD® 10.00-13 s/n 00762 © 2014 HydroCAD Software Solutions LLC

Page 8

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: WS-1	Runoff Area=1.944 sf	26.7
Subcatchinent 15. W5-1	Rulloli Alea- 1.944 Si	00.74

Runoff Area=1,944 sf 86.78% Impervious Runoff Depth>7.50"

Flow Length=61' Slope=0.0230 '/' Tc=0.8 min CN=95 Runoff=0.42 cfs 0.028 af

Subcatchment 2S: WS-2

Runoff Area=5,063 sf 84.40% Impervious Runoff Depth>7.41"

Flow Length=142' Tc=2.5 min CN=94 Runoff=1.05 cfs 0.072 af

Subcatchment 3S: WS-3

Runoff Area=852 sf 3.64% Impervious Runoff Depth>5.28"

Flow Length=12' Slope=0.1700 '/' Tc=1.1 min CN=75 Runoff=0.14 cfs 0.009 af

Subcatchment 4S: WS-4

Runoff Area=9,878 sf 73.70% Impervious Runoff Depth>7.22"

Flow Length=212' Tc=2.8 min CN=92 Runoff=2.03 cfs 0.136 af

Subcatchment 5S: WS-5

Runoff Area=2,449 sf 100.00% Impervious Runoff Depth>7.72"

Flow Length=15' Slope=0.1500 '/' Tc=0.1 min CN=98 Runoff=0.55 cfs 0.036 af

Pond CB2: CB2

Peak Elev=34.53' Storage=0.000 af Inflow=1.05 cfs 0.072 af

12.0" Round Culvert n=0.012 L=63.0' S=0.0103 '/' Outflow=1.06 cfs 0.072 af

Pond DMH1: DMH1

Peak Elev=33.70' Storage=0.000 af Inflow=1.66 cfs 0.131 af

12.0" Round Culvert n=0.012 L=9.0' S=0.0500 '/' Outflow=1.66 cfs 0.131 af

Pond P1: P1

Peak Elev=34.57' Storage=620 cf Inflow=1.93 cfs 0.136 af

Outflow=1.66 cfs 0.131 af

Link AP1: AP1

Inflow=3.66 cfs 0.268 af

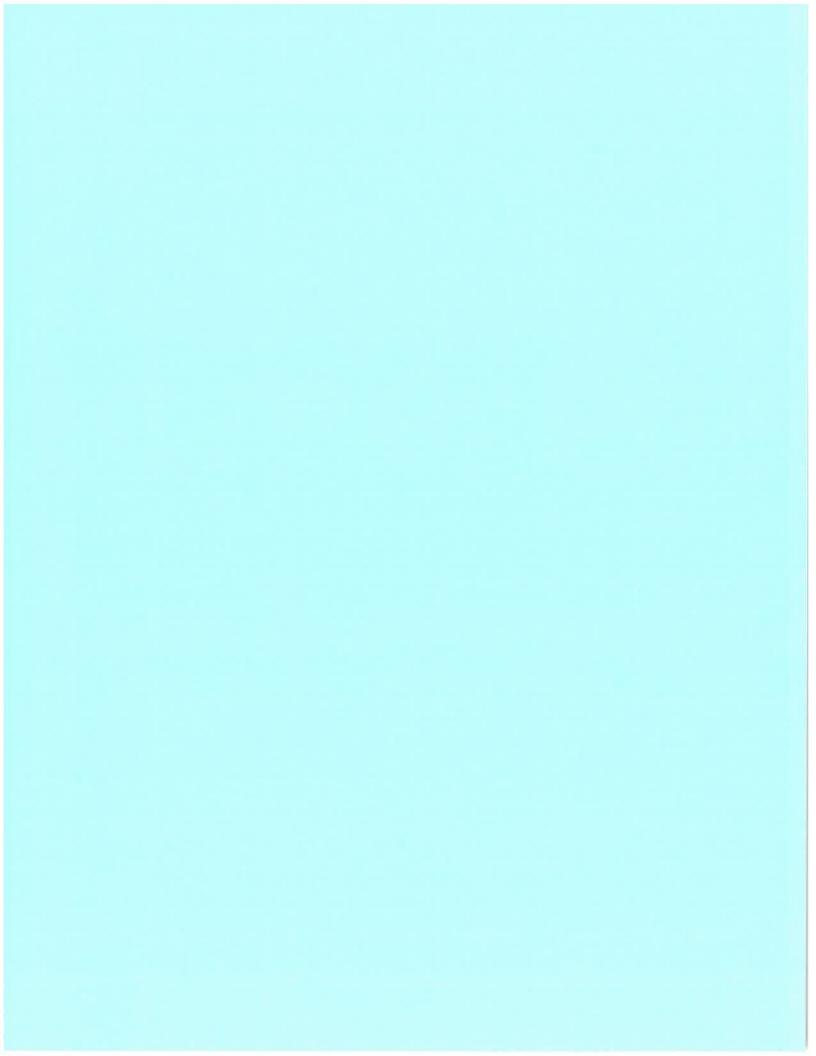
Primary=3.66 cfs 0.268 af

Link AP2: AP2

Inflow=0.14 cfs 0.009 af

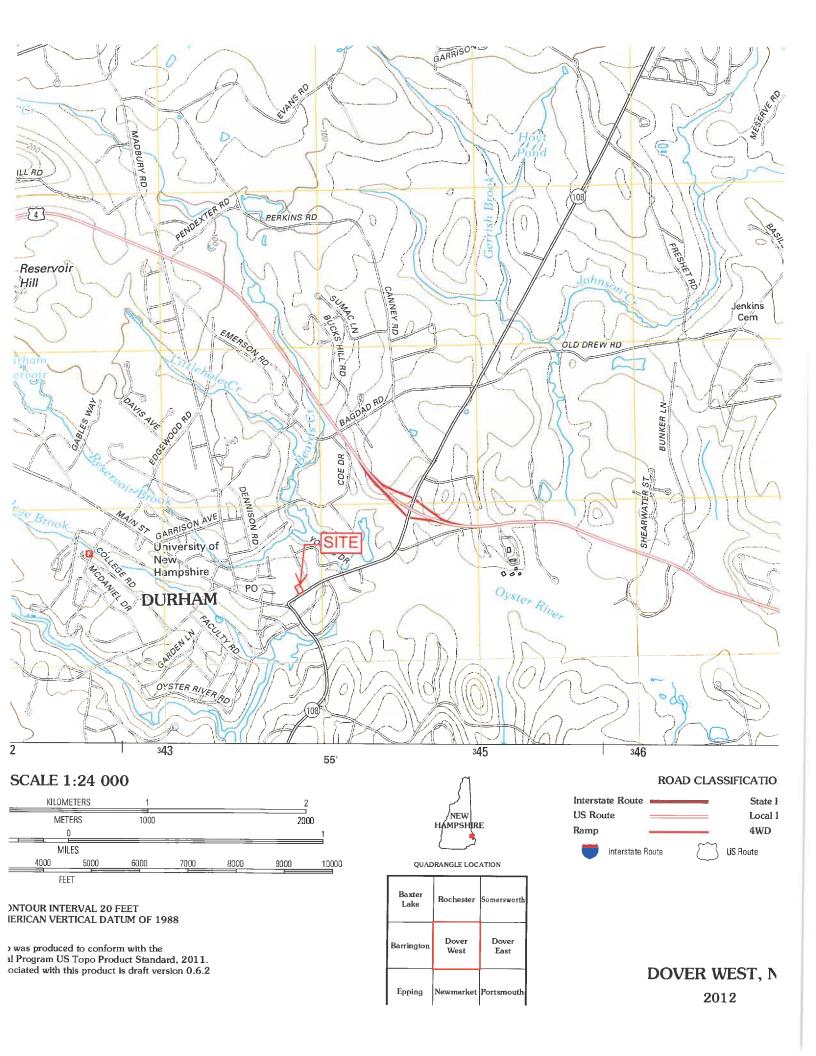
Primary=0.14 cfs 0.009 af

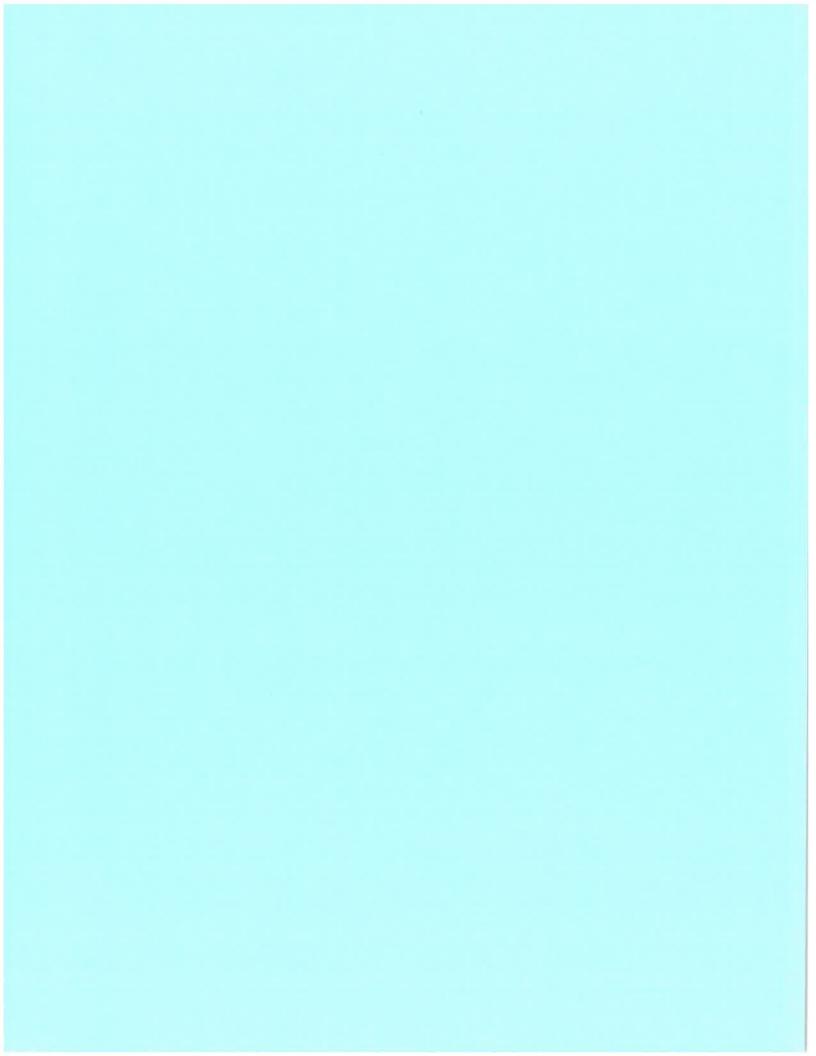
Total Runoff Area = 0.463 ac Runoff Volume = 0.281 af Average Runoff Depth = 7.27" 22.12% Pervious = 0.103 ac 77.88% Impervious = 0.361 ac



# APPENDIX III

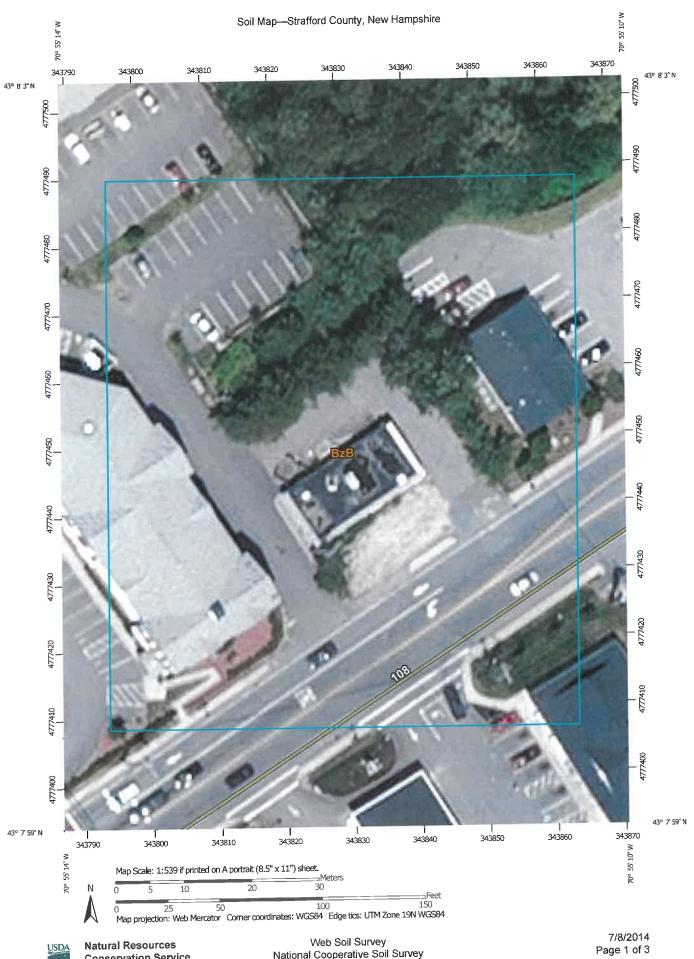
USGS Map





## APPENDIX IV

Web Soil Survey



## **MAP LEGEND**

Area of Ir	Area of Interest (AOI)	Œ	Spoil Area
	Area of Interest (AOI)	0	Stony Spot
Soils	Mood Hall and A	8	Very Stony Spot
	Soil Map Unit Lings	\$20	Wet Spot
	COII Map CITIC LITTES	<	Other
	Soil Map Unit Points		Special Line Fear
Specia	Special Point Features		

misunderstanding of the detail of mapping and accuracy of soil line

Enlargement of maps beyond the scale of mapping can cause

Warning: Soil Map may not be valid at this scale.

placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

The soil surveys that comprise your AOI were mapped at 1:20,000.

MAP INFORMATION



## Water Features

Streams and Canals

## Rails Transportation ‡

Closed Depression

Borrow Pit

Blowout

Clay Spot

Interstate Highways US Routes



Gravelly Spot

Landfill

**Gravel Pit** 



Marsh or swamp

102 de. 0

Lava Flow

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

## Aerial Photography Background

## This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Albers equal-area conic projection, should be used if more accurate

calculations of distance or area are required.

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator

projection, which preserves direction and shape but distorts

Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov Coordinate System: Web Mercator (EPSG:3857)

Source of Map: Natural Resources Conservation Service

Please rely on the bar scale on each map sheet for map

measurements.

Soil Survey Area: Strafford County, New Hampshire Survey Area Data: Version 13, Dec 31, 2013

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Jun 20, 2010—May 1,

imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background

Severely Eroded Spot

Slide or Slip

Sinkhole

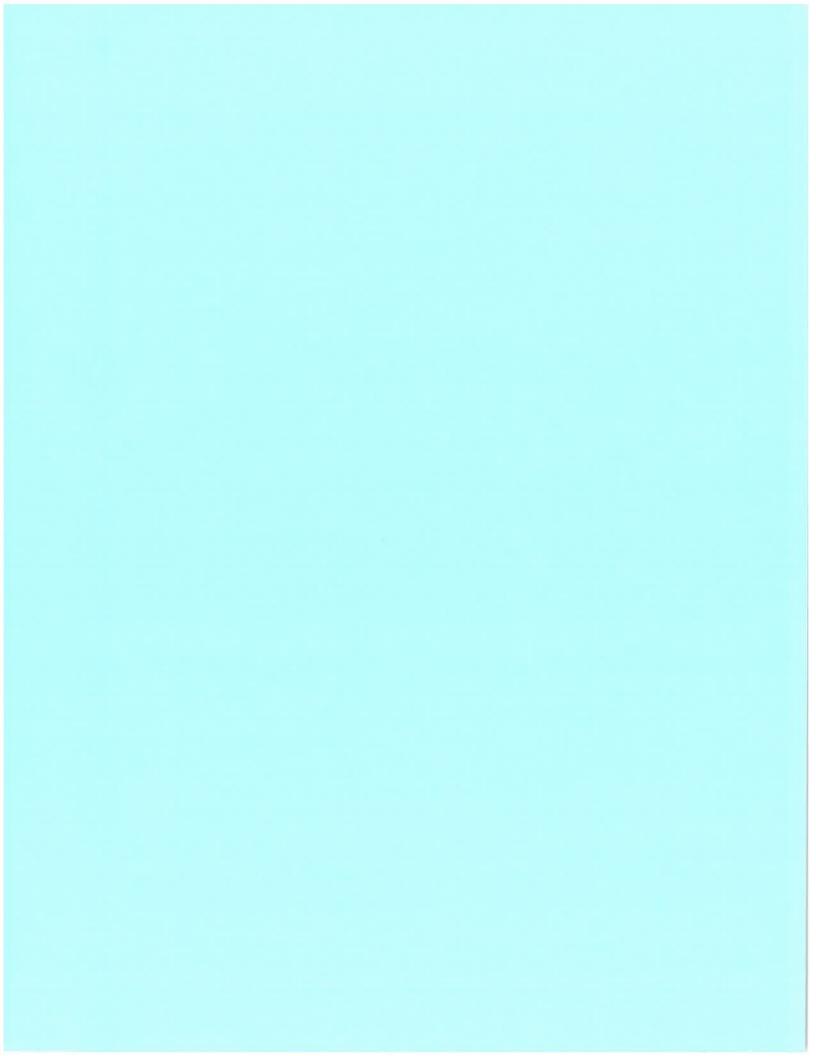
Sodic Spot

Sandy Spot

Saline Spot

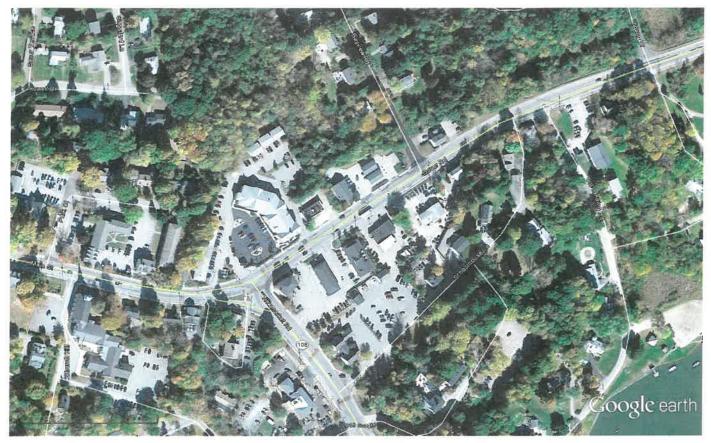
## **Map Unit Legend**

Strafford County, New Hampshire (NH017)									
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI						
BzB	Buxton silt loam, 3 to 8 percent slopes	1.4	100.0%						
Totals for Area of Interest		1.4	100.0%						



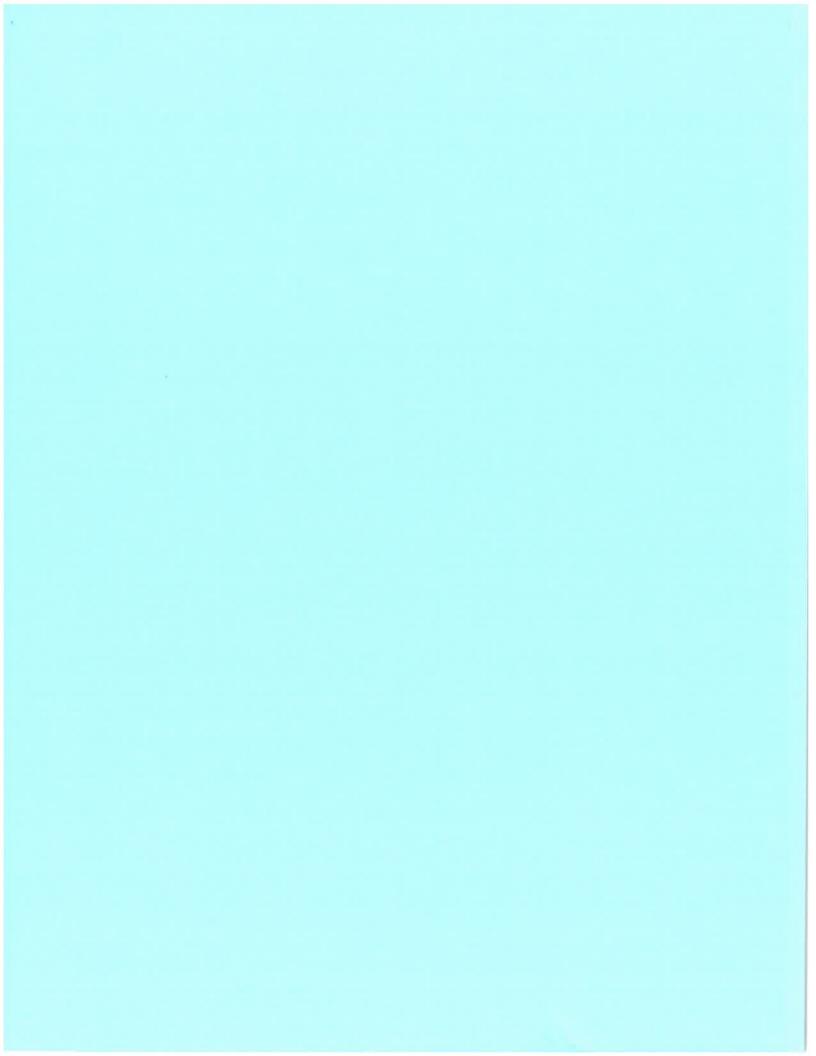
## APPENDIX V

**Aerial Photograph of Site** 



Google earth

feet \_\_\_\_\_\_1000 meters 300



## APPENDIX VI

**Operations and Maintenance Manual** 

## STORMWATER MANAGEMENT OPERATION AND MAINTENANCE MANUAL

Proposed Commercial Development
4 Bay Car Service
Tax Map 4 Lot 49
3 Dover Road
Durham, NH

Prepared for:

Tropic Star Development, LLC 321D Lafayette Road Hampton, NH 03842

Prepared by:
Jones & Beach Engineers, Inc.
85 Portsmouth Avenue
P.O. Box 219
Stratham, NH 03885
(603) 772-4746
Revision: February 11, 2015
August 27, 2014
JBE Project No. 14011

## Inspection and Maintenance of Facilities and Property

## A. Maintenance of Common Facilities or Property

1. The Property Owner (Owner) is responsible for maintenance of all stormwater infrastructure associated with this site. This includes all temporary and permanent stormwater and erosion control facilities both during and after construction.

## B. General Inspection and Maintenance Requirements

- 1. The Owner shall perform all inspections and maintenance with greater than annual frequency as required by this report.
- 2. Permanent stormwater and sediment and erosion control facilities to be maintained on the site include, but are not limited to, the following:
  - a. Catch basins and drain manholes
  - b. Culverts
  - c. Underground Detention basin
  - d. Vegetation and landscaping

- 2. Maintenance of permanent measures shall follow the following schedule:
  - a. **Culverts: Inspection** of culvert inlets and outlets at least **once per month** during the rainy season (March to November). Any debris is to be removed and disposed of properly.
  - b. **Erosion: Annual inspection** of the site for erosion, destabilization, settling, and sloughing. Any needed repairs are to be conducted immediately.
  - c. Vegetation and Landscaping: Annual inspection of site's vegetation and landscaping. Any areas that are bare shall be reseeded and mulched with hay or, if the case is extreme, loamed and seeded or sodded to ensure adequate vegetative cover. Landscape specimens shall be replaced in kind, if they are found to be dead or dying.
  - d. Catch basins and Drain Manholes: Annual inspection of catch basins and drain manholes to determine if they need to be cleaned. Catch basins are to be cleaned if the depth of deposits is greater than one-third the depth from the basin bottom to the invert of the lowest pipe or opening into or out of the basin. If a catch basin significantly exceeds the one-third depth standard during the inspection, then it should be cleaned more frequently. If woody debris or trash accumulates in a catch basin, then it should be cleaned on a weekly basis. Manholes should be cleaned of any material upon inspection. Catch basins and manholes can be cleaned either manually or by specially designed equipment including, but not limited to, bucket loaders and vacuum pumps. Before any materials can be disposed, it is necessary to perform a detailed chemical analysis to determine if the materials meet the EPA criteria for hazardous waste. This will help determine how the materials should be stored, treated, and disposed.
  - e. **Underground Detention Basin**: Detention Basin should be inspected twice annually and after every rainfall event of 2.5" or greater within a 24-hour period at a minimum. The detention basin areas designed to collect and detain stormwater will need only minimal maintenance. Traffic over the basin areas should be kept to a minimum prior to construction to prevent compaction of the soil reducing infiltration.

Basins shall be inspected for effectiveness at a minimum of twice annually. Sediment build up within the isolator rows shall be removed when sediment exceeds 3" in depth.

See attached sample forms as a guideline.

Any inquiries in regards to the design, function, and/or maintenance of any one of the above mentioned facilities or tasks shall be directed to the project engineer:

Jones & Beach Engineers, Inc. 85 Portsmouth Avenue P.O. Box 219 Stratham, NH 03885

T#: (603) 772-4746 F#: (603) 772-0227

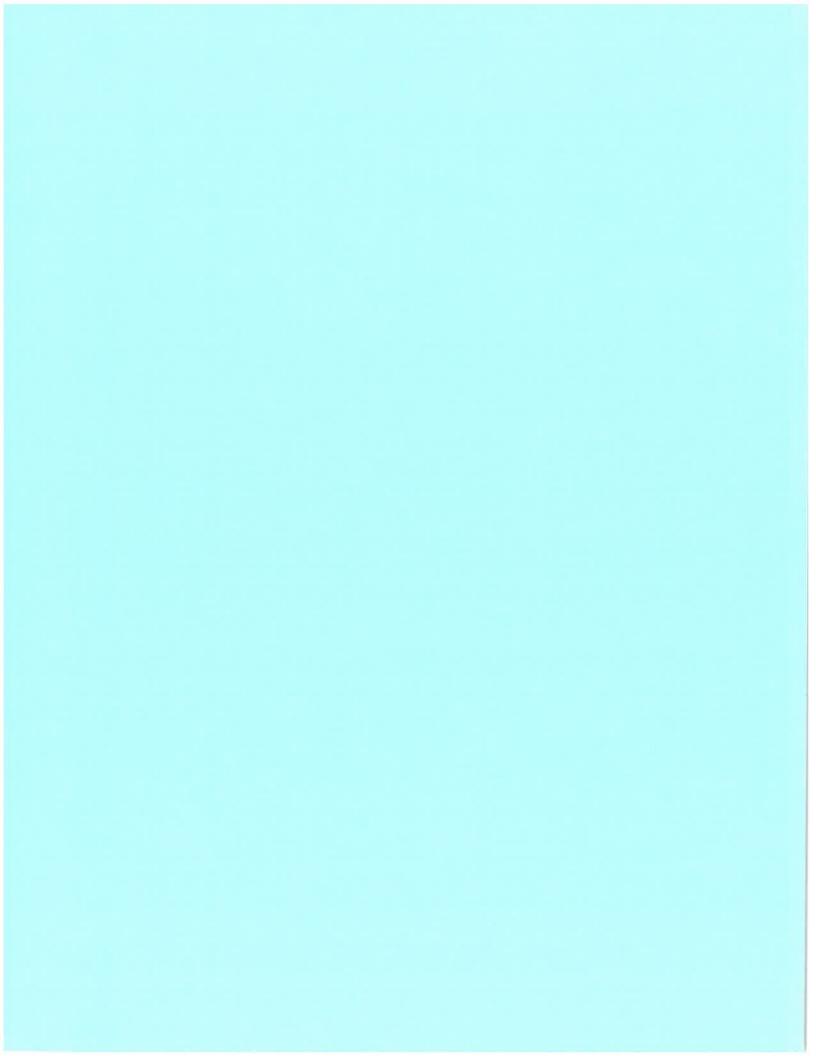
## STORM WATER POLLUTION PREVENTION PLAN INSPECTION PERIOD AND CRITERIA Tax Map 4 Lot 49 3 Dover Road Durham, NH

Stormwater Component	Inspection Period	Inspection Criteria/Methods
Culverts	Once per month	Inspect inlet/outlet. Remove debris.
Erosion	Annually	Repair site erosion.
Vegetation	Annually	Repair bare unvegetated areas.
Catch Basins and	Annually	Remove trash and debris. Inspect for sediment. Remove if sediment greater
Drain Manholes	(or more as required)	than 1/3 sump depth.
Detention Basins	Bi-annually	Inspect for standing water, sediment/debris collection, see item e.

# STORM WATER OPERATIONS AND MAINTENANCE PLAN INSPECTION REPORT

Tax Map 4 Lot 49 3 Dover Road Durham, NH

Yearly Inspection Form	Issue Detected / Action Taken											
	Inspector											
	Date of Inspection											
	Inspected Component	270	Erosion	Vegetation	Catch Basins	and Drain	Manholes		Detention Basins			



## APPENDIX VII

Drainage Plans: Pre-Development Drainage Plan Post-Development Drainage Plan